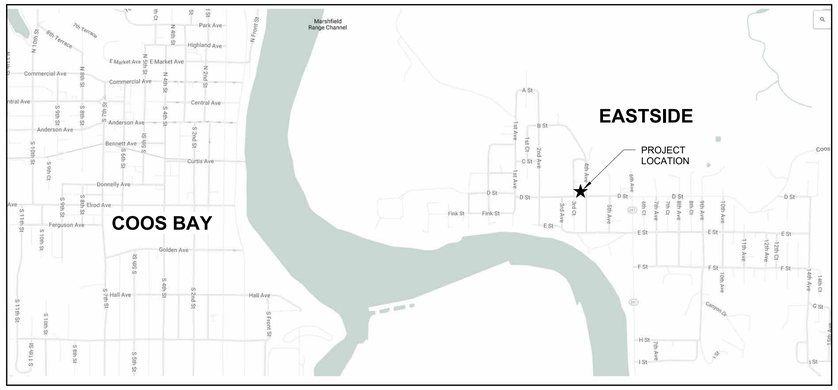
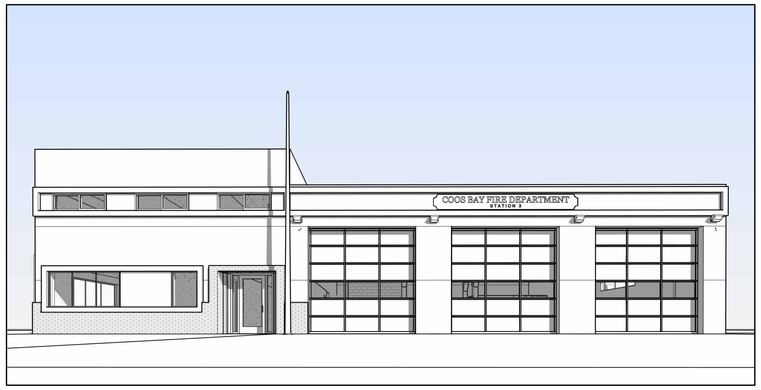
EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE

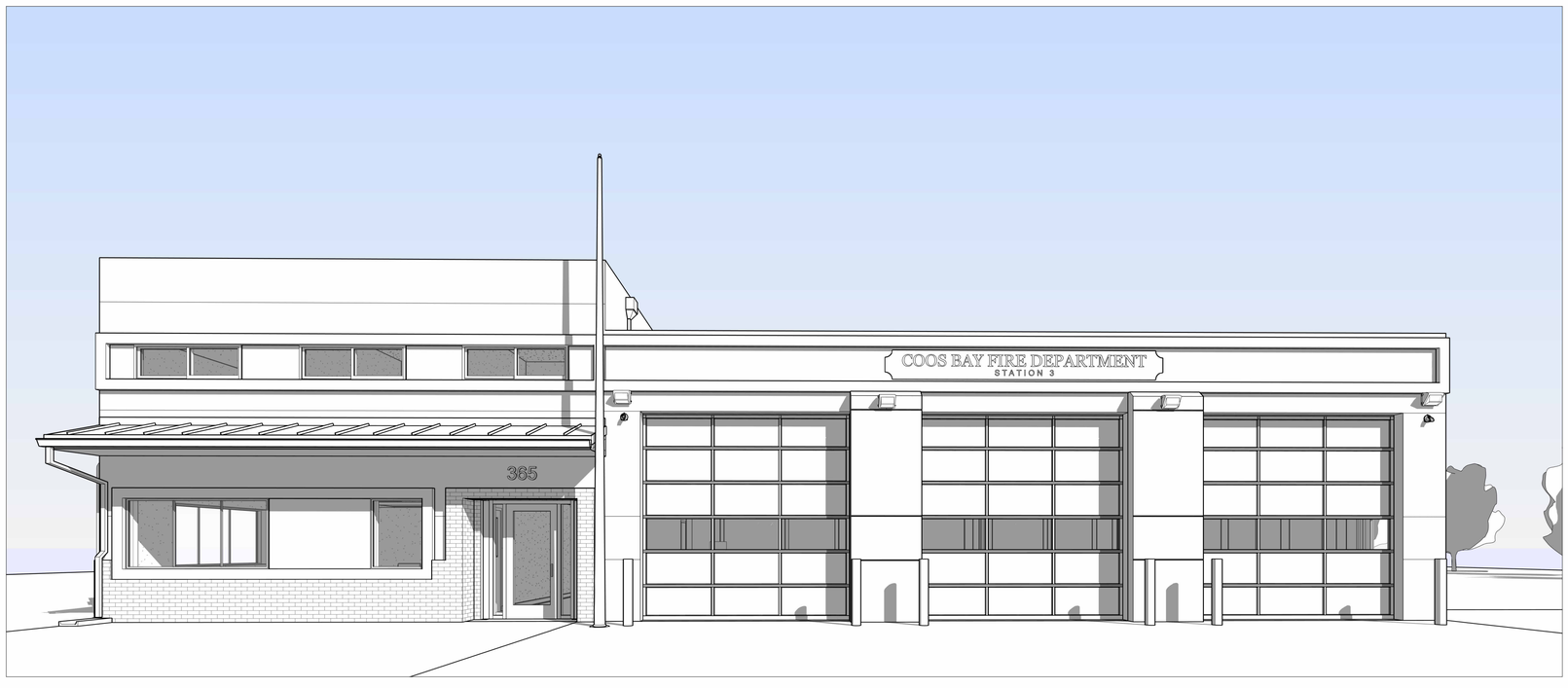
COOS BAY FIRE DEPARTMENT



3 **VICINITY MAP**
N.T.S.



2 **PERSPECTIVE - EXISTING**



1 **PERSPECTIVE**
N.T.S.

PROJECT TEAM

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SHEET INDEX

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REGISTERED ARCHITECT
2840
JOSEPH A. SLACK
COOS BAY, OREGON
STATE OF OREGON

PROJECT NO.: **22.22.2**

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
COVER SHEET

G0.0

CODE SUMMARY

APPLICABLE CODES:
2022 OREGON STRUCTURAL SPECIALTY CODE (OSSC)
COOS BAY MUNICIPAL CODE (CBMC): CHAPTER 17.235 INDUSTRIAL-COMMERCIAL DISTRICT

CONSTRUCTION TYPE (TABLE 601):
TYPE III B, NON-SPRINKLERED (NON COMBUSTIBLE EXTERIOR WALLS)

BUILDING AREA (GROSS SQUARE FEET):
EXISTING:
1ST FLOOR 3,840 SF
2ND FLOOR 1,425 SF
TOTAL: 5,265 SF

PROPOSED:
NO INCREASE OF BUILDING AREA

OCCUPANCY CLASSIFICATIONS (CHAPTER 3):
B BUSINESS (OFFICES & ASSOCIATED SPACES)
S-1 STORAGE (APPARATUS BAY)
R RESIDENTIAL (DORMS, DAYROOM)

OCCUPANCY SEPARATIONS (TABLE 508.4):
B / S-1: NO SEPARATION REQUIRED
B / R: 2-HR SEPARATION REQUIRED (NON-SPRINKLERED)
S-1 / R: 2-HR SEPARATION REQUIRED (NON-SPRINKLERED)

NOTE: NO CHANGE IN OCCUPANCY. EXISTING ASSEMBLIES BETWEEN OCCUPANCIES B / R ARE NOT FIRE-RATED. PER THE REQUIREMENTS FOR THE ALTERATION OF AN EXISTING BUILDING USING THE PRESCRIPTIVE PATH (OSSC SECTION 3405.3), IT IS NOT REQUIRED TO MAKE THESE ASSEMBLIES COMPLIANT WITH CURRENT CODE.

ALLOWABLE BUILDING HEIGHTS & AREAS:
TYPE III B, NON-SPRINKLERED, MOST STRINGENT OCCUPANCY TYPE
HEIGHT **(TABLE 504.3)**
ALLOWABLE: 55 FT
(ALLOWABLE HEIGHT PER CITY OF COOS BAY, ZONE I-C: 35 FT)
REMODEL: 24' - 9"; OK

STORIES **(TABLE 504.4)**
ALLOWABLE: 2
ACTUAL: 2; OK (NO CHANGE)

AREA **(TABLE 506.2)**
ALLOWABLE: 19,000 SF
ACTUAL: 5,265 SF; OK

TRAVEL DISTANCE MAXIMUM (TABLE 1017.2):
MOST STRINGENT OCCUPANCY TYPE: 200 FT; OK

PLUMBING FIXTURES (TABLE 2902.1):
BUILDING CLASSIFICATION: BUSINESS
TOILETS: 1 PER 25 FOR THE FIRST 50, THEN 1 PER 50
LAVATORIES: 1 PER 40 FOR THE FIRST 80, THEN 1 PER 80
DRINKING FOUNTAINS: NONE

TOTAL OCCUPANTS: 46
REQUIRED: 2 TOILETS, 2 LAVS
ACTUAL: 3, TOILETS, 3 LAVS, 1 SHOWER; OK

INSULATION VALUES:
ROOF: R-30 MINIMUM RIGID INSULATION + TAPERED RIGID INSULATION
WALLS: EXISTING UNINSULATED CONCRETE; NO CHANGE
FLOOR: EXISTING UNINSULATED SLAB ON GRADE; NO CHANGE

ROOF VENTING

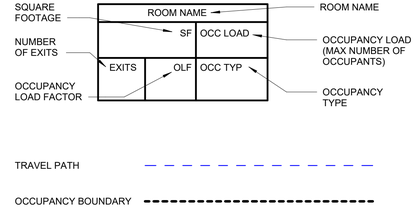
BUILDING AREA: 3,850 SQ. FT.
REQUIRED VENTING @ 1/150 = 25.66 SQ. FT.
REQUIRED VENTING @ 1/300 = 12.83 SQ. FT.
6.42 SQ. FT. UPPER; 6.42 SQ. FT. LOWER

LOWER VENTING REQ:
- 50'-0" LINEAR VENT
- 6.42 SQ. FT. REQ'D VENTING AREA
- REQUIRED VENT WIDTH: 0.13 SQ. FT. / LINEAR FT. = 19 SQ. IN. / LINEAR FT.
- ACTUAL: 20 SQ. IN. / LINEAR FT. (6"-WIDE VENT)

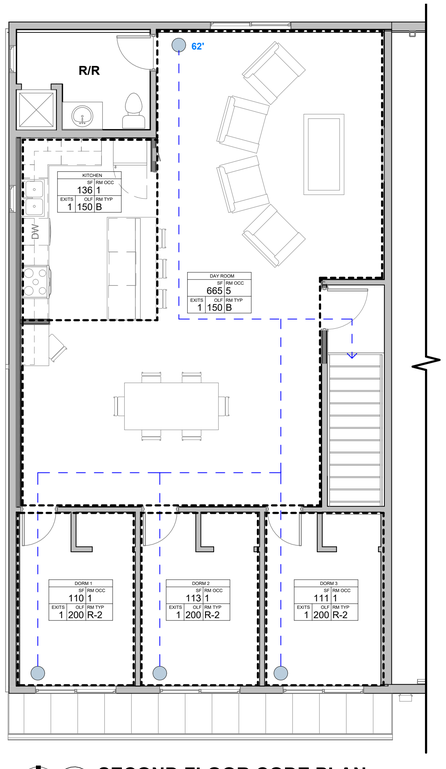
UPPER VENTING REQ:
- 50'-0" LINEAR VENT
- 6.42 SQ. FT. REQ'D VENTING AREA
- REQUIRED VENT WIDTH: 0.13 SQ. FT. / LINEAR FT. = 19 SQ. IN. / LINEAR FT.
- ACTUAL: 20 SQ. IN. / LINEAR FT. (6"-WIDE VENT

(RESULTING IN 50% UPPER VENTING & 50% LOWER VENTING)

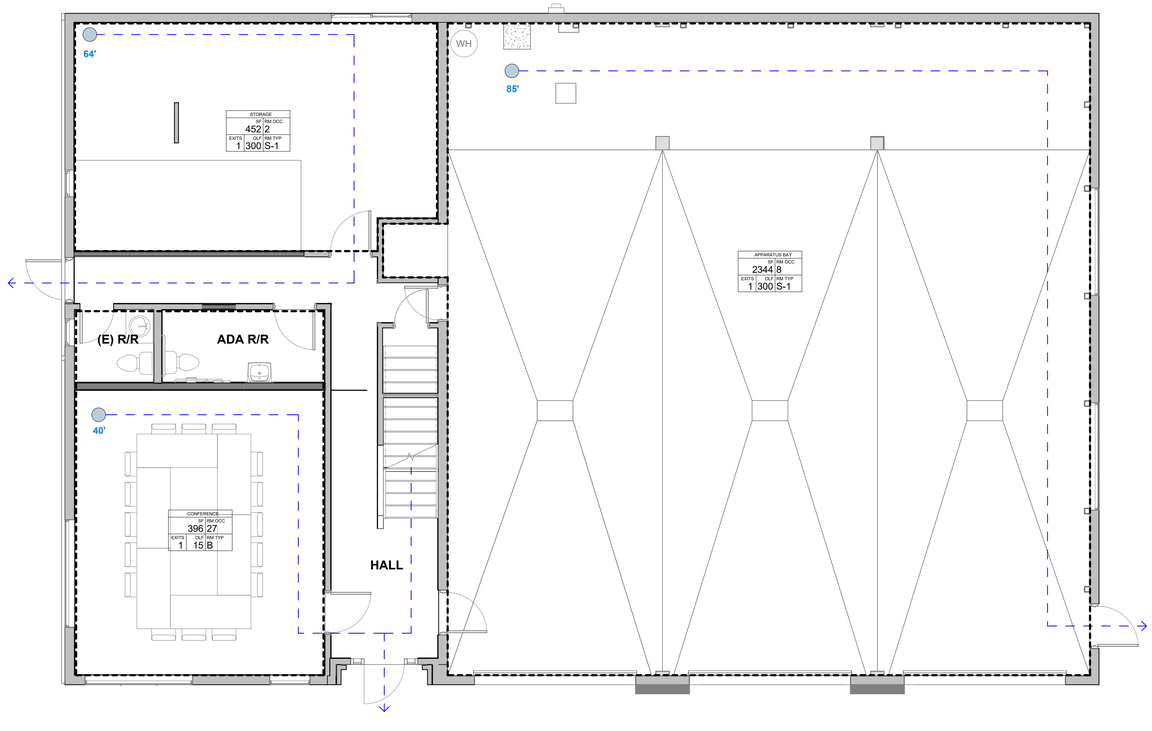
ROOM CODE LABEL



OCCUPANT LOAD SCHEDULE					
ROOM NUMBER	ROOM NAME	AREA	FUNCTION OF SPACE	OCCUPANT LOAD FACTOR	OCCUPANTS PER ROOM
3	CONFERENCE	396 SF	B	15	27
22	DAY ROOM	665 SF	B	150	5
23	KITCHEN	136 SF	B	150	1
B: 3		1197 SF			33
25	DORM 1	110 SF	R-2	200	1
26	DORM 2	113 SF	R-2	200	1
27	DORM 3	111 SF	R-2	200	1
R-2: 3		334 SF			3
6	STORAGE	452 SF	S-1	300	2
7	APPARATUS BAY	2344 SF	S-1	300	8
S-1: 2		2795 SF			10
GRAND TOTAL		4327 SF			46



2 **SECOND FLOOR CODE PLAN**
3/16" = 1'-0"



1 **FIRST FLOOR CODE PLAN**
3/16" = 1'-0"

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G0.1

LEGEND - PLAN SYMBOLS

SIM
1 / A-101 — BUILDING SECTION SYMBOL

1 / A-101 — DETAIL SYMBOL

1 / A-101 — ENLARGED PLAN SYMBOL

A-201 1 — EXTERIOR ELEVATION SYMBOL

A-211 1 — INTERIOR ELEVATION SYMBOL

4.XXX — DOOR TAG IDENTIFICATION

ROOM NAME
000 — ROOM NAME AND NUMBER

A — WALL TAG

A — WINDOW TAG

F.E. — FIRE EXTINGUISHER - SURFACE MOUNT

F.E.C. — FIRE EXTINGUISHER CABINET AND FIRE EXTINGUISHER - SEMI-RECESSED

FD — FLOOR DRAIN

GRID LINES

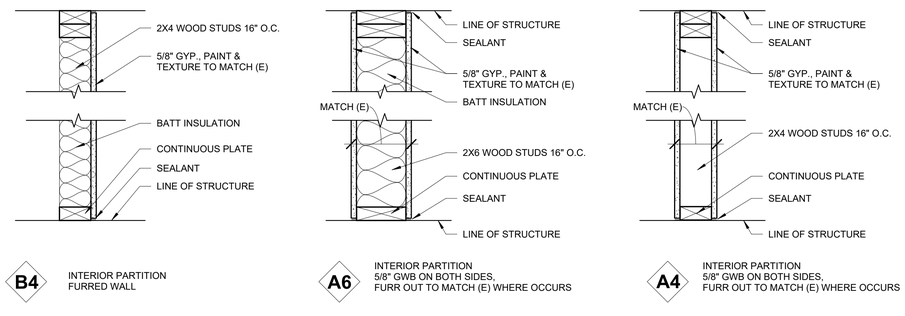
XXX
X' - X" — LEVEL OR SPOT ELEVATIONS

10'-10"
ACP-X — CEILING HEIGHT & FINISH

ABBREVIATIONS

	ANGLE
@	AT
AB	ANCHOR BOLT
AC	ACOUSTIC
ACC	ACCESS
ACT	ACOUSTIC CEILING TILE
ACP	ACOUSTIC CEILING PANEL
AD	AREA DRAIN
ADD	ADDITIONAL
ADJ	ADJUSTABLE
AFF	ABOVE FINISH FLOOR
AHU	AIR HANDLING UNIT
AL	ALUMINUM
ALT	ALTERNATE
AP	ACCESS PANEL
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL
ASPH	ASPHALT
BB	BOND BEAM
BD	BOARD
BF	BOTH FACES
BFC	BELOW FINISH CEILING
BG	BUMPER GUARD
BIT	BITUMINOUS
BLDG	BUILDING
BLKG	BLOCKING
BLKT	BLANKET
BM	BEAM/BENCH MARK
BLK	BLOCK
BOT	BOTTOM
BRG	BEARING
BRKR	BREAKER
BRK	BRICK
BRKT	BRACKET
BS	BACK SPLASH
BSMT	BASEMENT
BTWN	BETWEEN
CAB	CABINET
CER	CERAMIC
CFCI	CONTRACTOR FURNISHED, CONTRACTOR INSTALLED
CG	CORNER GUARD
CH	COAT HOOK
CIP	CAST IN PLACE
CJ	CONTROL JOINT/CONSTRUCTION JOINT
CLG	CEILING
CLO	CLOSET/CLOSURE
CLR	CLEAR
COL	COLUMN
COMB	COMBINATION
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CONF	CONFERENCE
CONN	CONNECTION/CONNECT
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
CORR	CORRIDOR
CPT	CARPET
CR	COAT RACK/CURTAIN ROD
CSG	CASING
CT	CERAMIC TILE
CTR	CENTER/COUNTER
CTSK	COUNTERSUNK
CUH	CABINET UNIT HEATER
CW	COLD WATER
D	DEPTH
DBL	DOUBLE
DET	DETAIL
DF	DRINKING FOUNTAIN
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIR	DIRECTION
DIV	DIVISION
DM	DE-MOUNTABLE PARTITION
DN	DOWN
DO	DITTO
DR	DOOR
DRWR	DRAWER
DS	DOWNSPOUT
DWG	DRAWING
DWL	DOWEL
DWS	DEFORMED WELDED STUD
EA	EACH
EC	ELECTRICAL CONTRACTOR
EF	EACH FACE
EH	ELECTRICAL HEATER/EXHAUST HOOD
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR/ELEVATION
EMBED	EMBEDDED
EMER	EMERGENCY
ENT	ENTRANCE
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EMERGENCY SHOWER
ESR	ELASTOMERIC SHEET ROOFING
ETR	EXISTING TO REMAIN
EVC	ELASTIC VINYL COATING
EW	EACH WAY
EWC	ELECTRIC WATER COOLER
EXC	EXCAVATE
EXP	EXPANSION
EXPD	EXPOSED
EXPF	EXPLOSION PROOF
EXT	EXTERIOR
FA	FIELD ADJUSTABLE
FV	FIELD VERIFY
FD	FLOOR DRAIN
FDN	FOUNDATION
FE	FIRE EXTINGUISHER
FEC	FIRE EXTINGUISHER CABINET
FHC	FIRE HOSE CABINET
FIN	FINISH
FIX	FIXTURE
FLEX	FLEXIBLE
FLR	FLOOR
FLRG	FLOORING
FOS	FACE OF STUD
FP	FIREPROOF/FIRE PROTECTION
FR	FIRE RETARDANT
FS	FULL SIZE/FULL SCALE
FT	FEET
FTG	FOOTING
FURG	FURRING
GA	GAUGE
GAL	GALLON
GALV	GALVANIZED
GB	GRAB BAR
GC	GENERAL CONTRACTOR
GEN	GENERAL
GFCI	GOVERNMENT FURNISHED, CONTRACTOR INSTALLED
GFGI	GOVERNMENT FURNISHED, GOVERNMENT INSTALLED
GFRC	GLASS FIBER REINFORCED CONCRETE
GFRG	GLASS FIBER REINFORCED GYPSUM
GL	GLASS
GLB	GLUE LAM BEAM
GMU	GLAZED MASONRY UNIT
GWB	GYPSUM WALL BOARD
GYP	GYPSUM
H	HEIGHT
HDBD	HARDBOARD
HDCP	HANDICAPPED
HDG	HOT DIPPED GALVANIZED
HDWD	HARDWOOD
HDWE	HARDWARE
HK	HOOK
HM	HOLLOW METAL
HP	HIGH POINT
HR	HANDRAIL
HT	HEIGHT
HVAC	HEATING VENTILATION AND AIR CONDITIONING
HWS	HEAD WELDED STUDS
ID	INSIDE DIAMETER
IMP	INSULATED METAL PANEL
IN	INCHES
INFO	INFORMATION
INSUL	INSULATION
INT	INTERIOR
IPW	INSULATED PLENUM WALL
IRF	INSULATED ROOF FILL
JAN	JANITOR
JS	JANITOR SINK
JST	JOIST
JT	JOINT
KD	KNOCKED DOWN
KO	KNOCK-OUT / KNEE OPENING
L	LENGTH
LAB	LABORATORY
LAM	LAMINATED
LB	POUND
LBS	POUNDS
LD	LINEAR DIFFUSER
LDG	LANDING
LF	LINEAR FOOT
LG	LONG
LGT	LIGHT
LKR	LOCKER
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LP	LOW POINT
LSH	LONG SLOTTED HOLE
LSL	LAMINATED STRAND LUMBER
LTG	LIGHTING
LVL	LAMINATED VENEER LUMBER
LVR	LOUVER
LWC	LIGHTWEIGHT CONCRETE
MACH	MACHINE
MAN	MANUAL
MAR	MARBLE
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MBW	MASONRY BEARING WALL
MC	MECHANICAL CONTRACTOR
MDO	MEDIUM DENSITY OVERLAY
MECH	MECHANICAL
MEMB	MEMBRANE
MET	METAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MIN	MINIMUM
MIR	MIRROR
MISC	MISCELLANEOUS
MK	MARK
ML	METAL LATH
MLDG	MOLDING
MO	MASONRY OPENING
MP	METAL PARTITION
MS	MACHINE SCREW
MTD	MOUNTED
MTG	MOUNTING
NA	NOT APPLICABLE
NIC	NOT IN CONTRACT
NO	NUMBER
NOM	NOMINAL
NS	NON-SHRINK
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
OA	OVERALL
OC	ON CENTER
OD	OUTSIDE DIAMETER/OVERFLOW DRAIN
OFF	OFFICE
OPNG	OPENING
OPP	OPPOSITE
OZ	OUNCE
PART	PARTITION
PC	PIECE
PCC	PRECAST CONCRETE
PCPL	PORTLAND CEMENT PLASTER
PDWR	PAPER TOWEL DISP. & WASTE RECEPTACLE
PH	PHILLIPS HEAD/PHASE
PL	PLATE/PROPERTY LINE
PLAM	PLASTIC LAMINATE
PLAS	PLASTER
PLBG	PLUMBING
PLYWD	PLYWOOD
PM	PROTECTED METAL
PNL	PANEL
PNLG	PANELING
POL	POLISHED
PR	PAIR
PRE FAB	PREFABRICATED
PRE FIN	PRE-FINISHED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT/PAINT
PTD	PAINTED/PAINT
PTM	PAINT TO MATCH
PVC	POLYVINYL CHLORIDE
QT	QUARRY TILE
QTY	QUANTITY
RAD	RADIUS
RAH	ROOFTOP AIR HANDLING UNIT
RB	RUBBER BASE
RC	REINFORCED CONCRETE
RCP	RADIANT CEILING PANEL / REFLECTED CEILING PLAN
RD	ROOF DRAIN
REC	RECESSED
REF	REFERENCE
REINF	REINFORCING
REL	RELOCATE
REM	REMAINDER
REQD	REQUIRED
RES	RESILIENT
RET	RETURN
RI	ROUGH IN
RM	ROOM
RO	ROUGH OPENING
RT	RUBBER TILE
RUB	RUBBER
SAMF	SELF ADHESIVE MEMBRANE FLASHING
SAT	STANDARD AGGREGATE TOPPING
SAWRB	SELF ADHESIVE WEATHER RESTISTANT BARRIER
SB	SOIL BEARING
SC	SEAMLESS COATING
SCF	SPECIAL CONCRETE FINISH
SCHD	SCHEDULE
SD	SOAP DISPENSER
SE	SHELF EDGE
SECT	SECTION
SF	SAND FLOAT
SG	SUPPLY AIR GRILLE
SGL	SINGLE
SH	SHELF
SHD	SHOWER DOOR
SHT	SHEET
SIM	SIMILAR
SJ	STEEL JOIST
SLV	SHORT LEG VERTICAL
SM	SMOOTH
SND	SANITARY NAPKIN DISPENSER
SNV	SANITARY NAPKIN VENDER
SOG	SLAB ON GRADE
SPEC	SPECIFICATION
SPR	SPRINKLER
SQ	SQUARE
SR	SHOWER ROD
SS	STAINLESS STEEL
ST	STREET
STD	STANDARD
STL	STEEL
STO	STORAGE
STRU	STRUCTURAL/STRUCTURE
SUSP	SUSPENDED
SV	SHEET VINYL
SYM	SYMMETRICAL
T & B	TOP AND BOTTOM
TB	TACKBOARD/TOWEL BAR
TBR	TO BE REMOVED
TCP	THIN COAT PLASTER
TD	TOWEL DISPENSER
TDW	TOWEL DISPENSER AND WASTE
TEMP	TEMPERATURE/TEMPERED
TER	TERRAZZO
TEX	TEXTURE
TFC	TROWELED FLOOR COVERING
T & G	TONGUE AND GROOVE
THK	THICK
TJI	TRUSS JOIST I-JOIST
TOB	TOP OF BEAM
TOC	TOP OF CURB/TOP OF CONCRETE
TOD	TOP OF DECK/TOP OF DUCT ELEVATION
TOF	TOP OF FOOTING
TOG	TOP OF GRATE
TOJ	TOP OF JOIST
TOP	TOP OF PIPE ELEVATION
TOS	TOP OF SLAB/TOP OF STEEL
TOW	TOP OF WALL
TPG	TOPPING
TPH	TOILET PAPER HOLDER
TRAN	TRANSOM
TRANS	TRANSVERSE
TS	TUBE STEEL
TWS	THREADED WELDED STUD
TYP	TYPICAL
UG	UNDERGROUND
UNO	UNLESS NOTED OTHERWISE
UR	URINAL
V	VINYL
VB	VINYL BASE
VCT	VINYL COMPOSITION TILE
VERT	VERTICAL
VEST	VESTIBULE
VIF	VERIFY IN FIELD
VOL	VOLUME
VWC	VINYL WALL COVERING
W	WIDE FLANGE STEEL BEAM
W/	WITH
WAF	WELDED ANGLE FRAME
WC	WATER CLOSET
WD	WOOD
WDW	WINDOW
WF	WIDE FLANGE
WG	WIRE GLASS
W/O	WITHOUT
WP	WEATHERPROOF
WPFG	WATERPROOFING
WR	WASTE RECEPTACLE
WRB	WEATHER RESTISTANT BARRIER
WSCT	WAINSCOT
WSTP	WEATHERSTRIP
WTR	WATER
WWF	WELDED WIRE FABRIC
X	EXISTING

ABBREVIATIONS ABOVE ARE FOR ARCHITECTURAL SHEETS ONLY.



1 WALL TYPES

1 1/2" = 1'-0"

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STATE OF OREGON

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

PROJECT NO.: **22.22.2**

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

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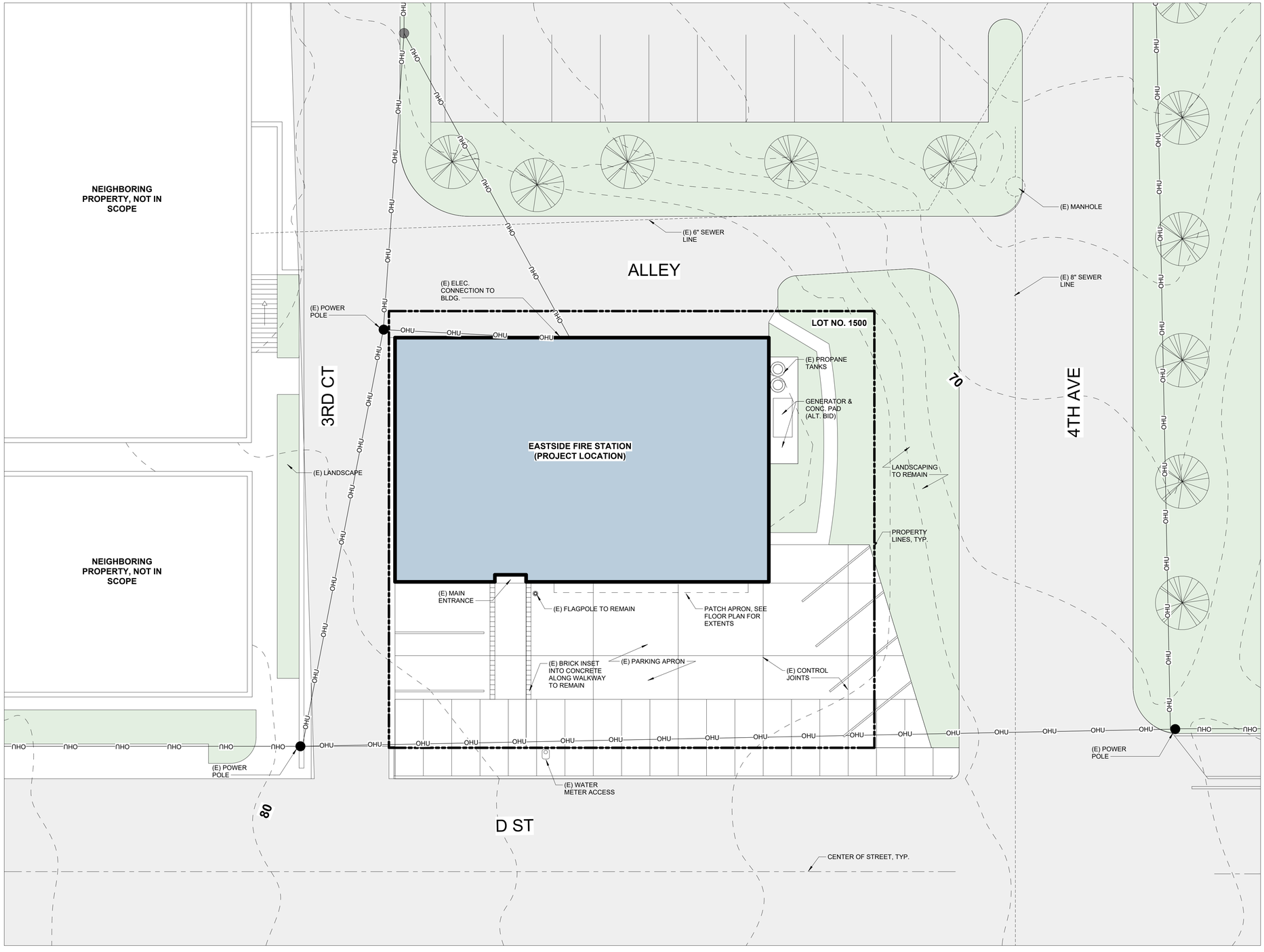
GENERAL INFORMATION

G0.2



SITE PLAN LEGEND

- (E) BUILDING FOOTPRINT
- (E) ASPHALT
- PLANTER / LANDSCAPING
- POLE MOUNTED SITE LIGHTING
- 2'-0" TOPO LINES
- SEWER LINE

1 **SITE PLAN**
1" = 10'-0"

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SITE PLAN

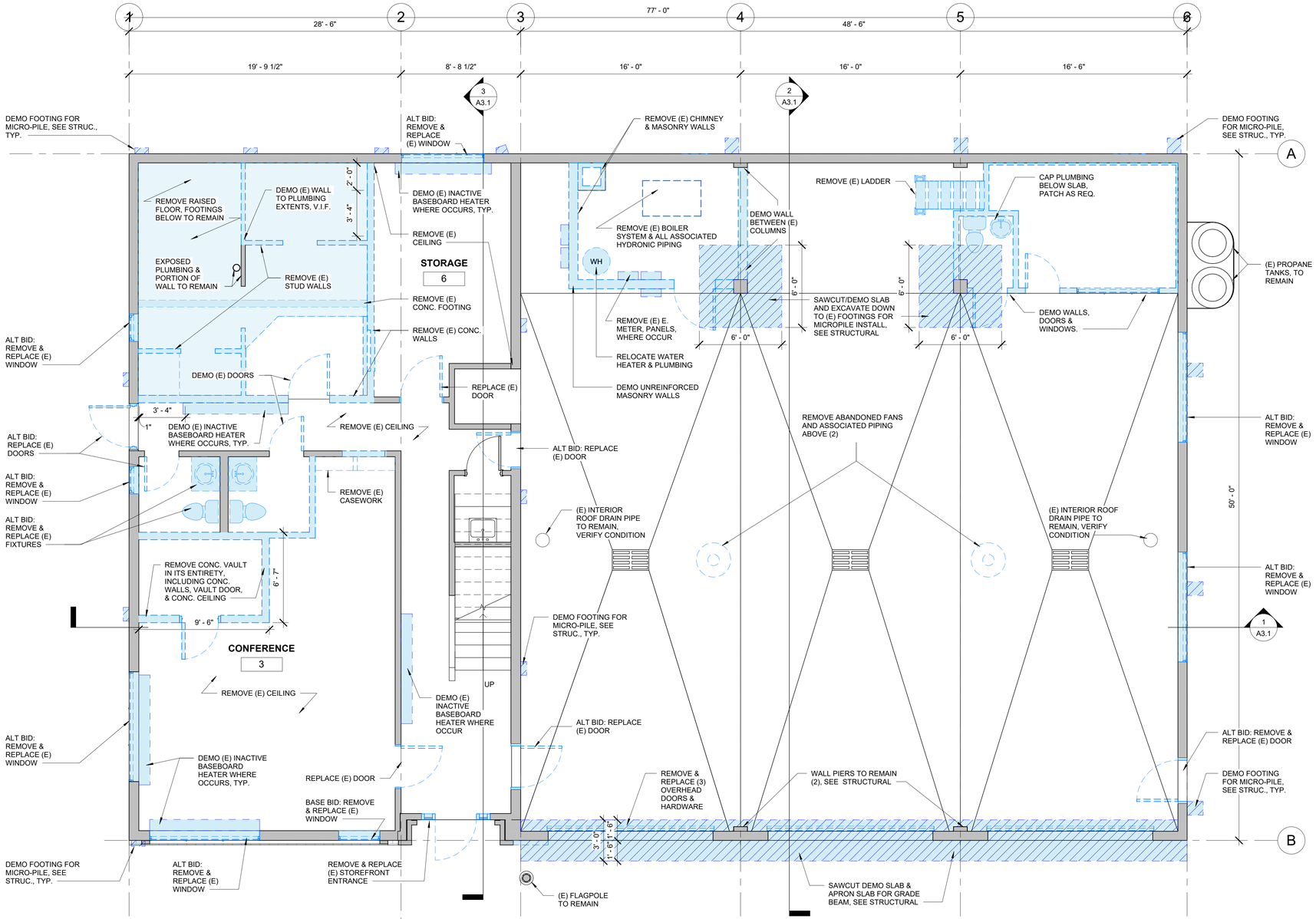
A1.1

DEMO PLAN LEGEND

- DEMO WALL / DOOR / WINDOW, EQUIPMENT
- (E) WALLS TO REMAIN
- DEMO SLAB

DEMO PLAN GENERAL NOTES

1. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS. SITE GRADES, ETC., IN FIELD PRIOR TO CONSTRUCTION AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES IN THE DRAWINGS AND ACTUAL FIELD CONDITIONS PRIOR TO START OF WORK.
2. ANY FIELD CONDITIONS NOT CALLED OUT ON THE ARCHITECTURAL PLANS ARE TO BE COORDINATED WITH THE CONTRACTOR, ARCHITECT, OWNER, AND OTHERS AS NECESSARY IN THE FIELD.
3. CONTRACTOR TO PROTECT AREAS AND SURFACES ADJACENT TO THE CONSTRUCTION AREA FROM DAMAGE AND DEBRIS. ALL AREAS ARE TO BE CLEAN AND SERVICEABLE AT THE COMPLETION OF DEMOLITION, PRIOR TO COMMENCEMENT OF NEW CONSTRUCTION
4. CUT ALL EXISTING & ABANDONED EXPOSED BOLT HEADS IN CONCRETE WALLS THROUGHOUT BUILDING. PATCH & PAINT.



1 FIRST FLOOR DEMO PLAN

1/4" = 1'-0"

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1ST FLOOR DEMO PLAN

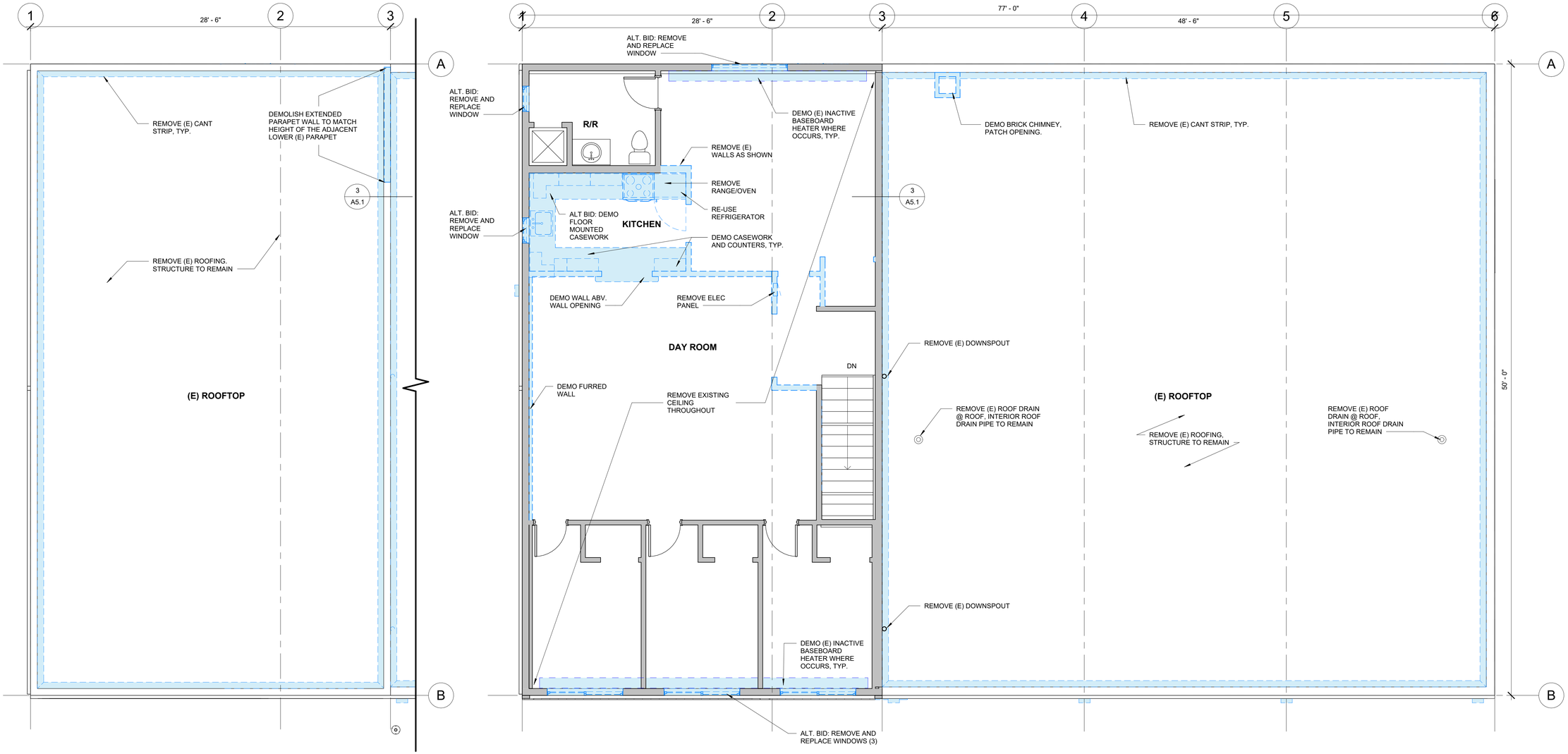
AD2.1

DEMO PLAN LEGEND

- DEMO WALL / DOOR / WINDOW, EQUIPMENT
- (E) WALLS TO REMAIN
- DEMO SLAB

DEMO PLAN GENERAL NOTES

1. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS. SITE GRADES, ETC., IN FIELD PRIOR TO CONSTRUCTION AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES IN THE DRAWINGS AND ACTUAL FIELD CONDITIONS PRIOR TO START OF WORK.
2. ANY FIELD CONDITIONS NOT CALLED OUT ON THE ARCHITECTURAL PLANS ARE TO BE COORDINATED WITH THE CONTRACTOR, ARCHITECT, OWNER, AND OTHERS AS NECESSARY IN THE FIELD.
3. CONTRACTOR TO PROTECT AREAS AND SURFACES ADJACENT TO THE CONSTRUCTION AREA FROM DAMAGE AND DEBRIS. ALL AREAS ARE TO BE CLEAN AND SERVICEABLE AT THE COMPLETION OF DEMOLITION, PRIOR TO COMMENCEMENT OF NEW CONSTRUCTION
4. CUT ALL EXISTING & ABANDONED EXPOSED BOLT HEADS IN CONCRETE WALLS THROUGHOUT BUILDING. PATCH & PAINT.



2 **ROOF DEMO PLAN**
1/4" = 1'-0"

1 **SECOND FLOOR DEMO PLAN**
1/4" = 1'-0"

HGE ARCHITECTS
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REGISTERED ARCHITECT
2840
JOSEPH A. SLACK
COOS BAY, OREGON
STATE OF OREGON

PROJECT NO.: **22.22.2**

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

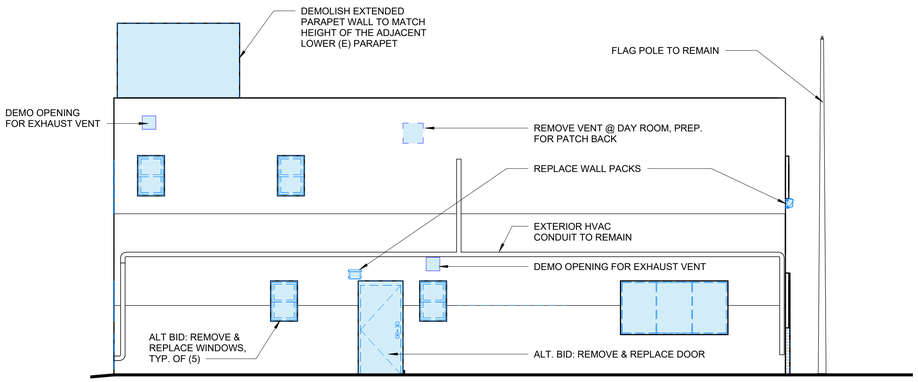
DATE: SEPTEMBER 2025

SHEET TITLE:
2ND FLOOR AND ROOF DEMO PLANS

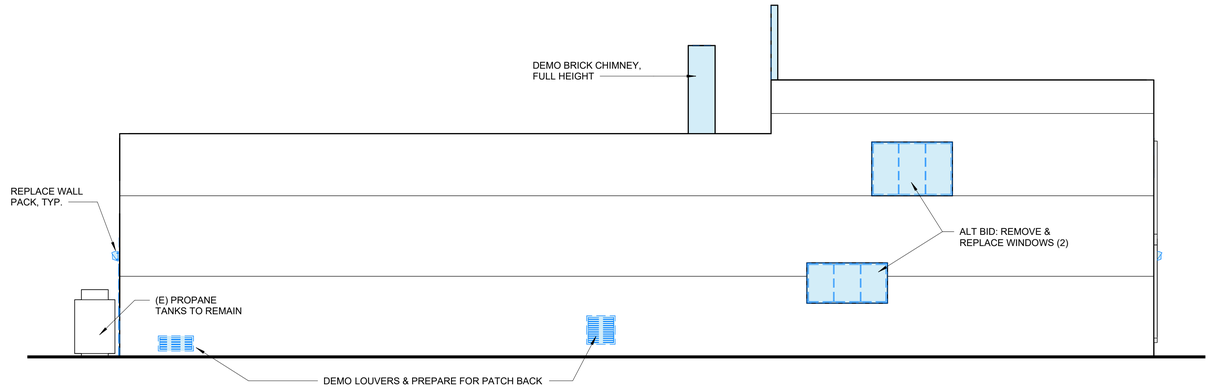
AD2.2

DEMO PLAN GENERAL NOTES

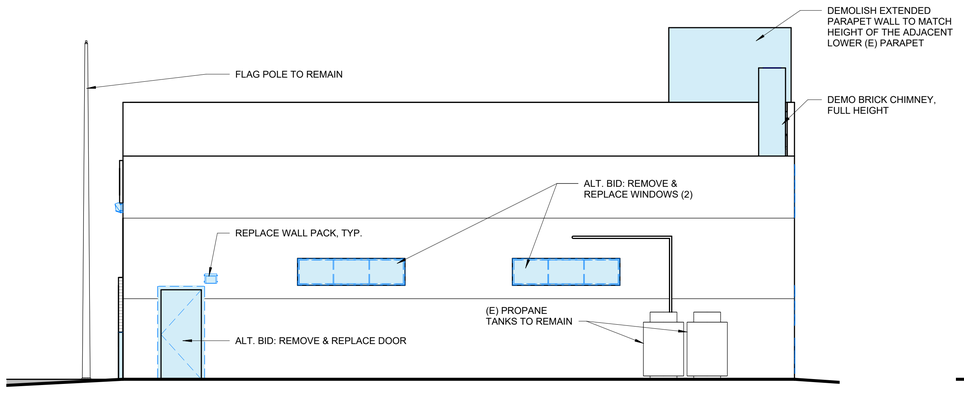
1. REPLACING (E) WALL PACKS TO BE BASE BID



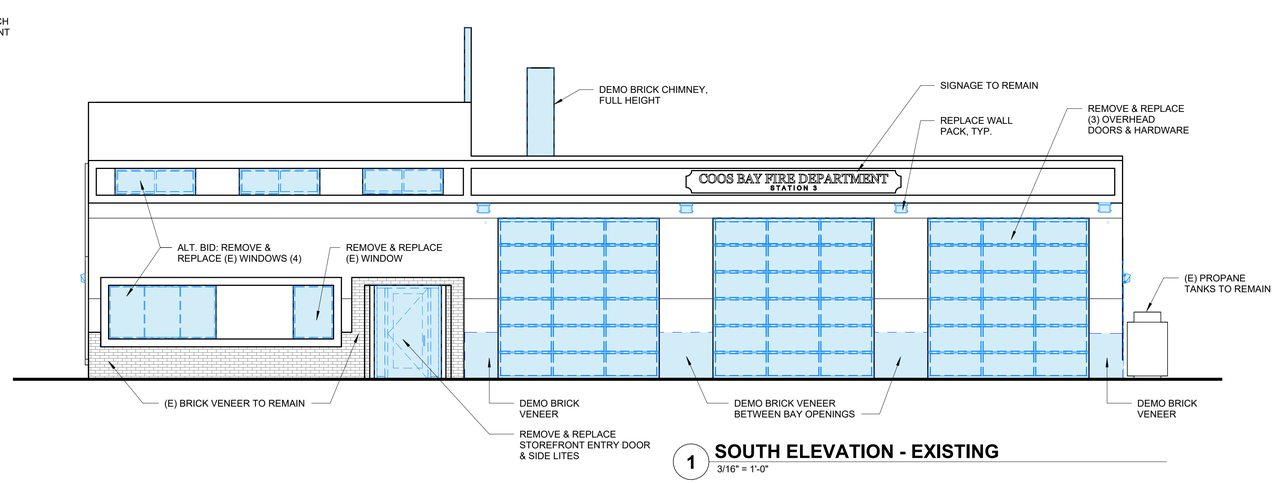
4 WEST ELEVATION - EXISTING
3/16" = 1'-0"



3 NORTH ELEVATION - EXISTING
3/16" = 1'-0"



2 EAST ELEVATION - EXISTING
3/16" = 1'-0"



1 SOUTH ELEVATION - EXISTING
3/16" = 1'-0"

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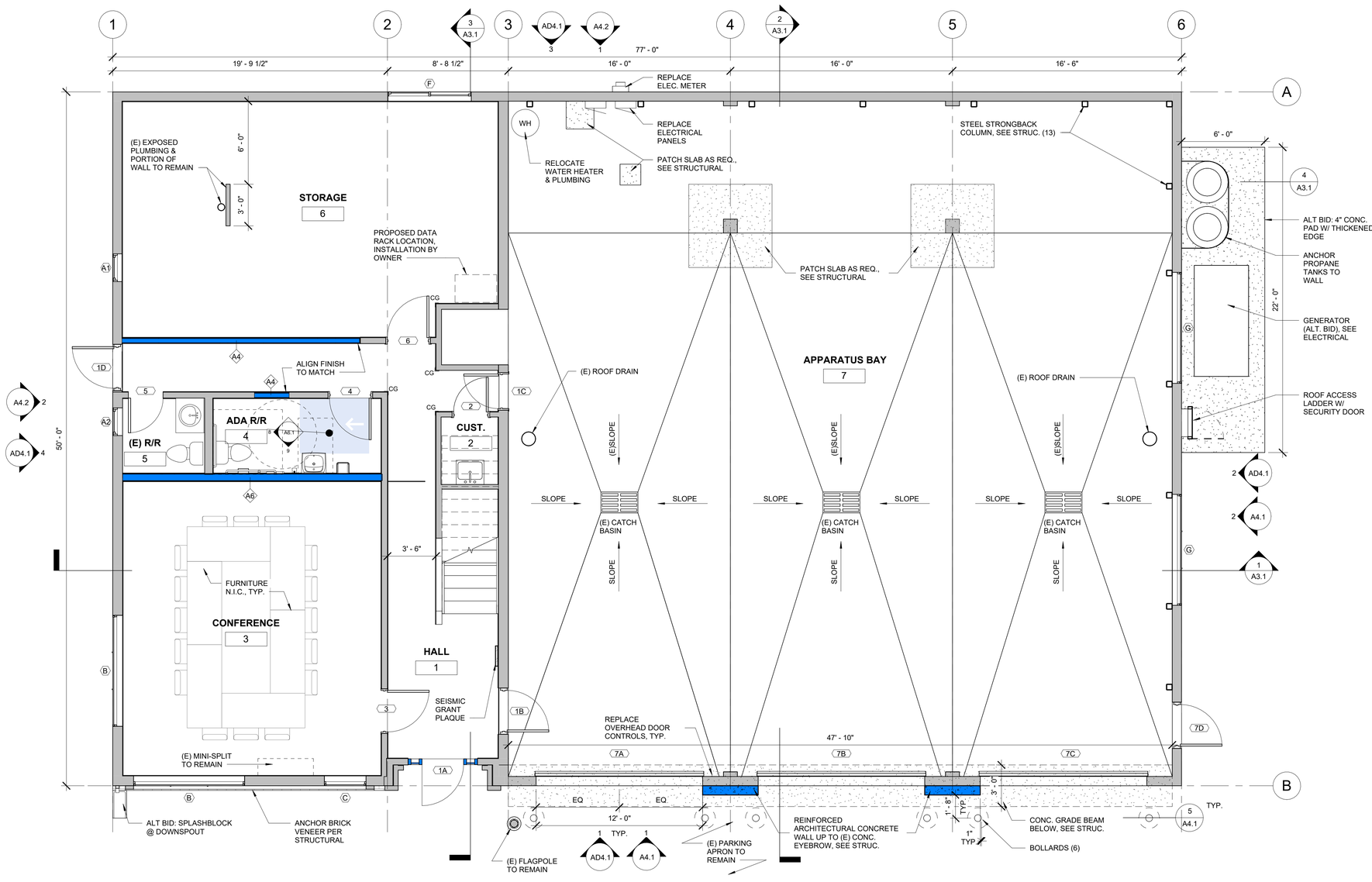
SHEET TITLE:
BUILDING ELEVATIONS DEMO

AD4.1



1 **FIRST FLOOR PLAN**
1/4" = 1'-0"

FLOOR PLAN GENERAL NOTES:

1. DIMENSIONS SHOWN ARE TO FACE OF STUD UNLESS NOTED OTHERWISE. CONTACT THE ARCHITECT FOR ANY ADDITIONAL DIMENSIONS REQUIRED TO LAY OUT THE WORK. WRITTEN DIMENSIONS GOVERN; DO NOT SCALE DRAWINGS.
2. ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE COUNTY AND STATE CODES AND STANDARDS.
3. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL UTILITY COMPANIES AND PERFORMING ALL WORK REQUIRED FOR UTILITIES.
4. ANY FIELD CONDITIONS NOT CALLED OUT ON THE ARCHITECTURAL PLANS ARE TO BE COORDINATED WITH THE CONTRACTOR, ARCHITECT, OWNERS, AND OTHERS AS NECESSARY IN THE FIELD.
5. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES IN THE DRAWINGS AND ACTUAL FIELD CONDITIONS PRIOR TO START OF WORK.
6. THE ARCHITECT WILL NOT BE RESPONSIBLE FOR, NOR HAVE CONTROL OF CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, NOR THE SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK AND WILL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
7. THE ARCHITECT WILL NOT BE RESPONSIBLE, NOR HAVE CONTROL OF, NOR BE IN CHARGE OF THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OF THEIR AGENTS OR EMPLOYEES, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK.

FLOOR PLAN LEGEND

- EXISTING WALLS TO REMAIN
- WALLS
- 1/2 WALLS
- SHEAR WALLS, SEE STRUCTURAL
- X — WALL TYPE
- X — WINDOW TYPE
- XX — DOOR NUMBER
- NEW SLAB

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#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
1ST FLR PLAN

A2.1

FLOOR PLAN GENERAL NOTES:

1. DIMENSIONS SHOWN ARE TO FACE OF STUD UNLESS NOTED OTHERWISE. CONTACT THE ARCHITECT FOR ANY ADDITIONAL DIMENSIONS REQUIRED TO LAY OUT THE WORK. WRITTEN DIMENSIONS GOVERN; DO NOT SCALE DRAWINGS.
2. ALL CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE COUNTY AND STATE CODES AND STANDARDS.
3. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL UTILITY COMPANIES AND PERFORMING ALL WORK REQUIRED FOR UTILITIES.
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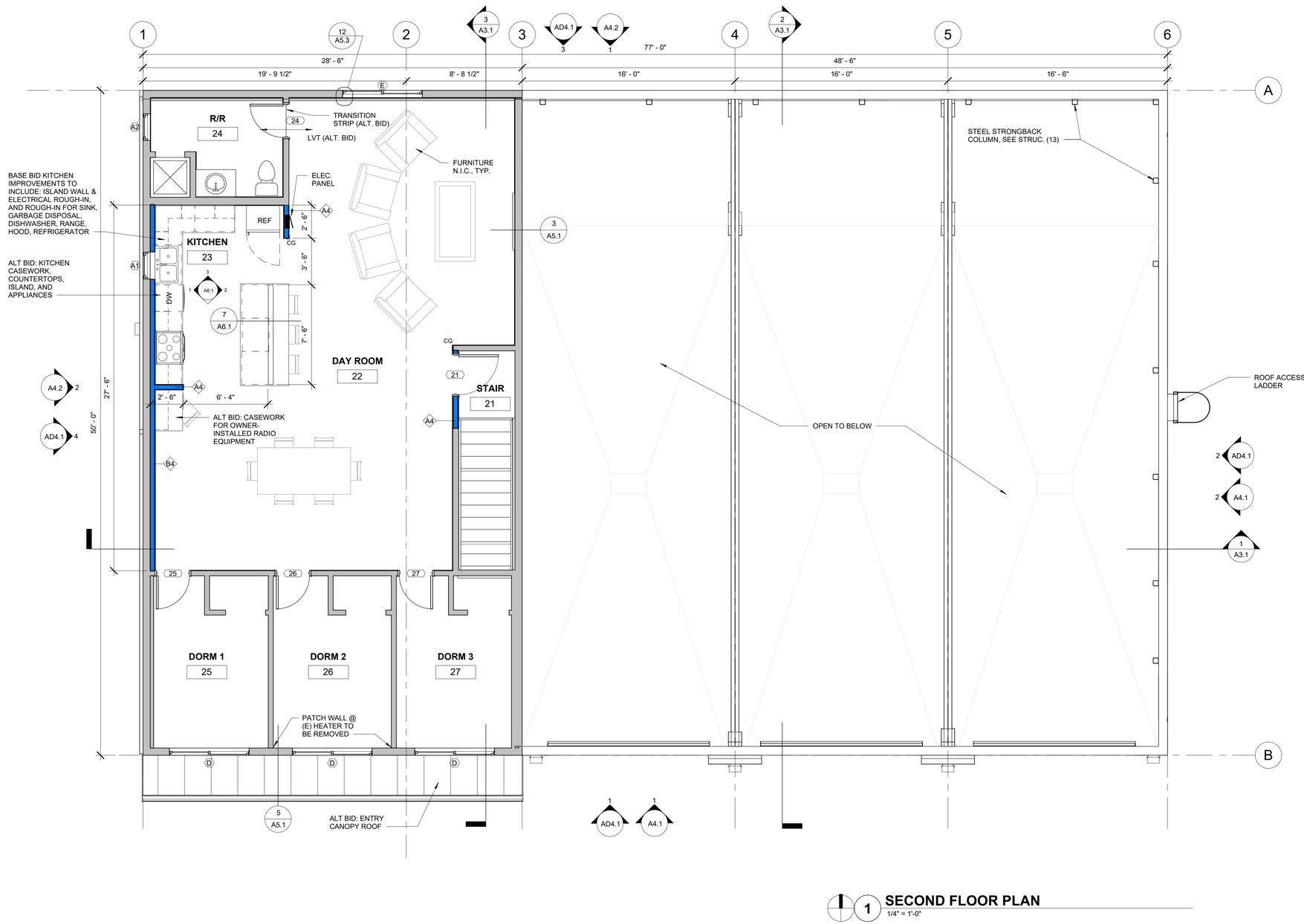
6. THE ARCHITECT WILL NOT BE RESPONSIBLE FOR, NOR HAVE CONTROL OF CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, NOR THE SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK AND WILL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
7. THE ARCHITECT WILL NOT BE RESPONSIBLE, NOR HAVE CONTROL OF, NOR BE IN CHARGE OF THE ACTS OR OMISSIONS OF THE CONTRACTOR, SUBCONTRACTOR, OR ANY OF THEIR AGENTS OR EMPLOYEES, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK.

FLOOR PLAN LEGEND

- EXISTING WALLS TO REMAIN
- WALLS
- 1/2 WALLS
- SHEAR WALLS, SEE STRUCTURAL
- X — WALL TYPE
- X — WINDOW TYPE
- XX — DOOR NUMBER
- NEW SLAB



1 **SECOND FLOOR PLAN**
1/4" = 1'-0"

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REGISTERED ARCHITECT 2840 JOSEPH A. SLACK COOS BAY, OREGON STATE OF OREGON

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#	DATE	DESCRIPTION

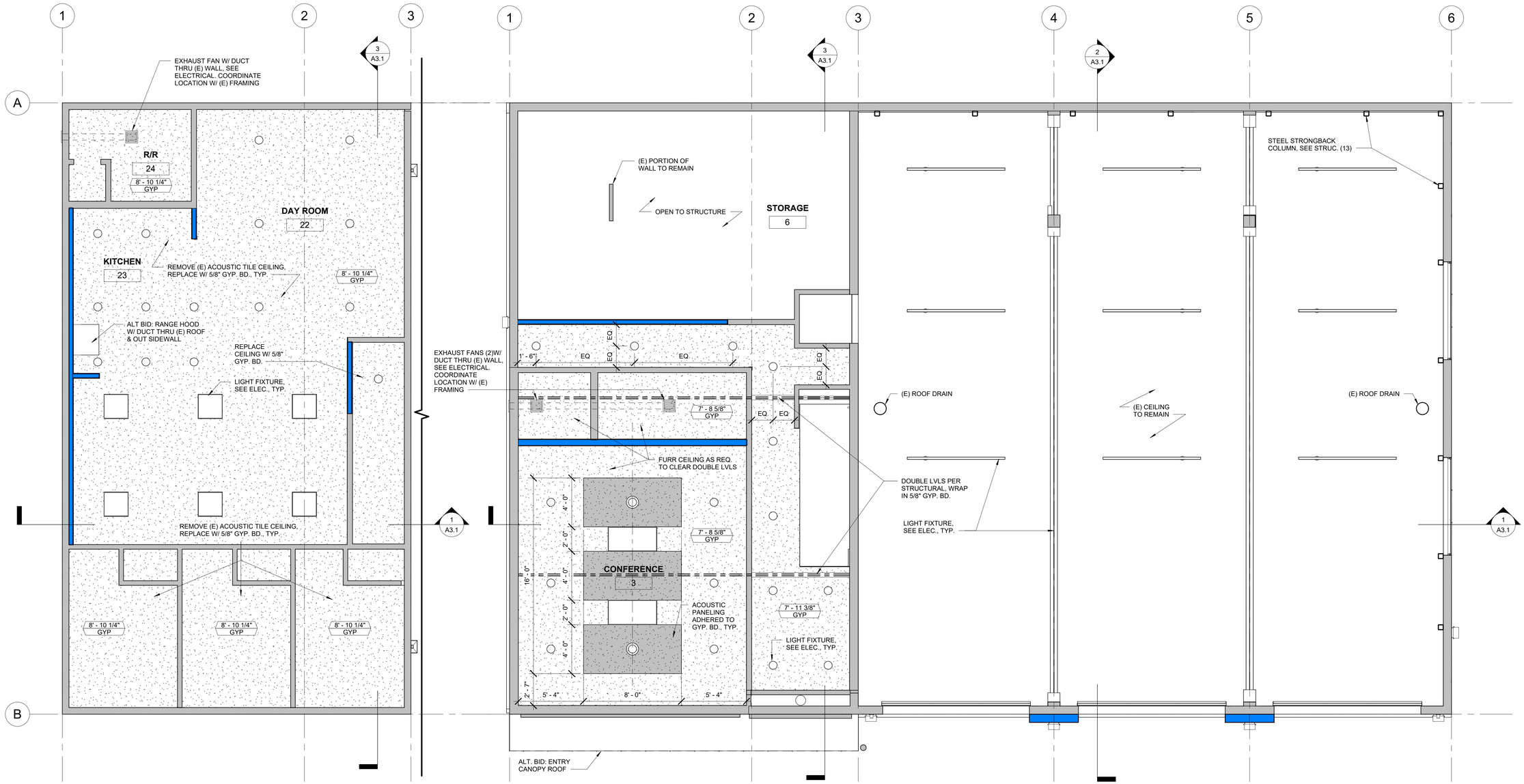
DATE: SEPTEMBER 2025

SHEET TITLE:
2ND FLR PLAN

A2.2

REFLECTED CEILING PLAN LEGEND

- EXIST. CEILING TO REMAIN
- LIGHT FIXTURES, SEE ELECTRICAL.
- GYP. BOARD CEILING
- 10'-0" ACT — CEILING ELEVATION



2 SECOND FLOOR RCP
1/4" = 1'-0"

1 FIRST FLOOR RCP
1/4" = 1'-0"

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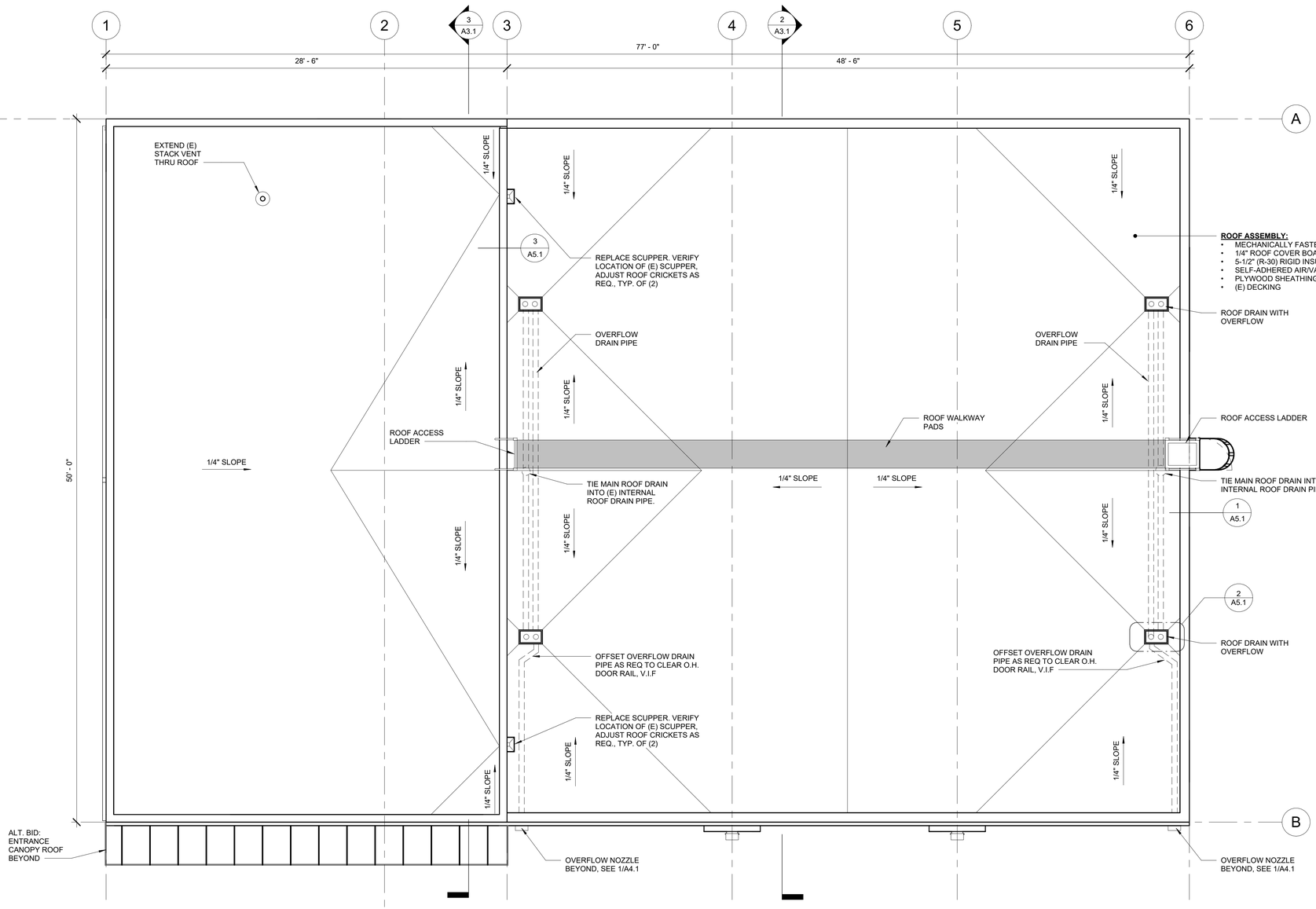
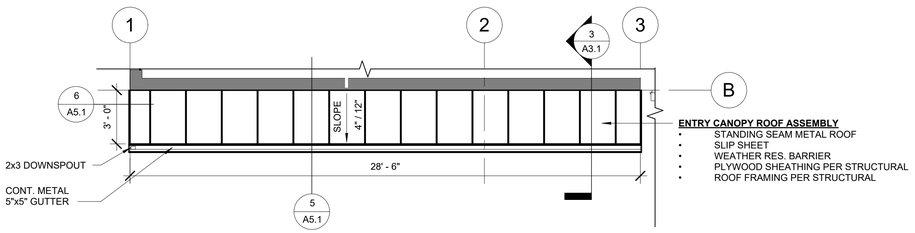
DATE: SEPTEMBER 2025

SHEET TITLE:
REFLECTED CEILING PLANS

A2.3



1 ROOF PLAN
1/4" = 1'-0"



2 ROOF PLAN - ENTRY CANOPY (ALT. BID)
1/4" = 1'-0"

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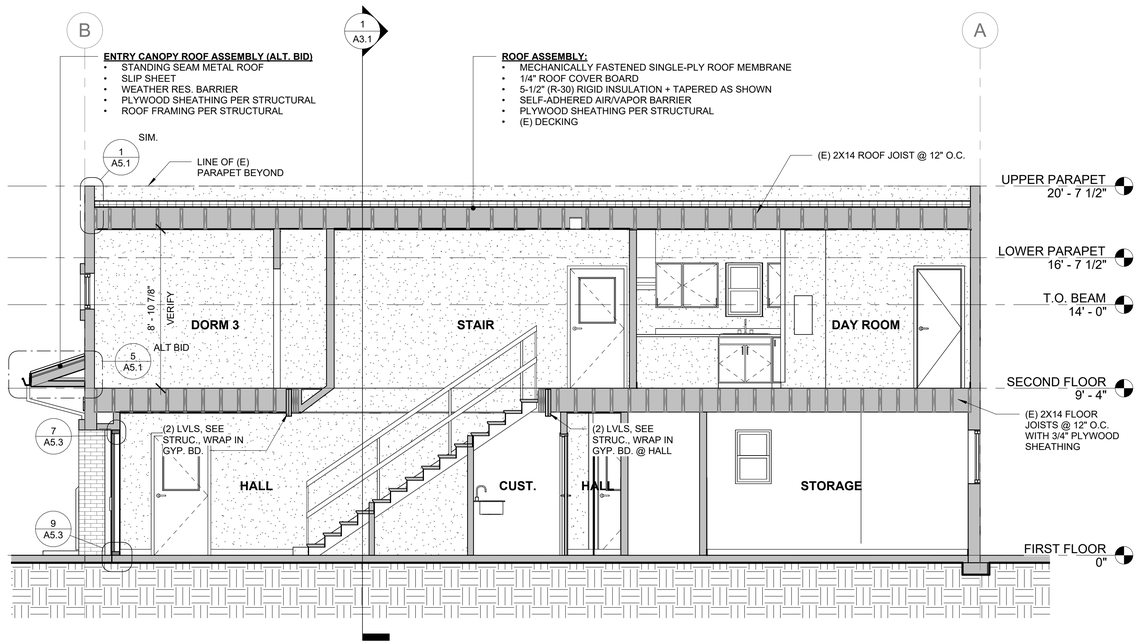
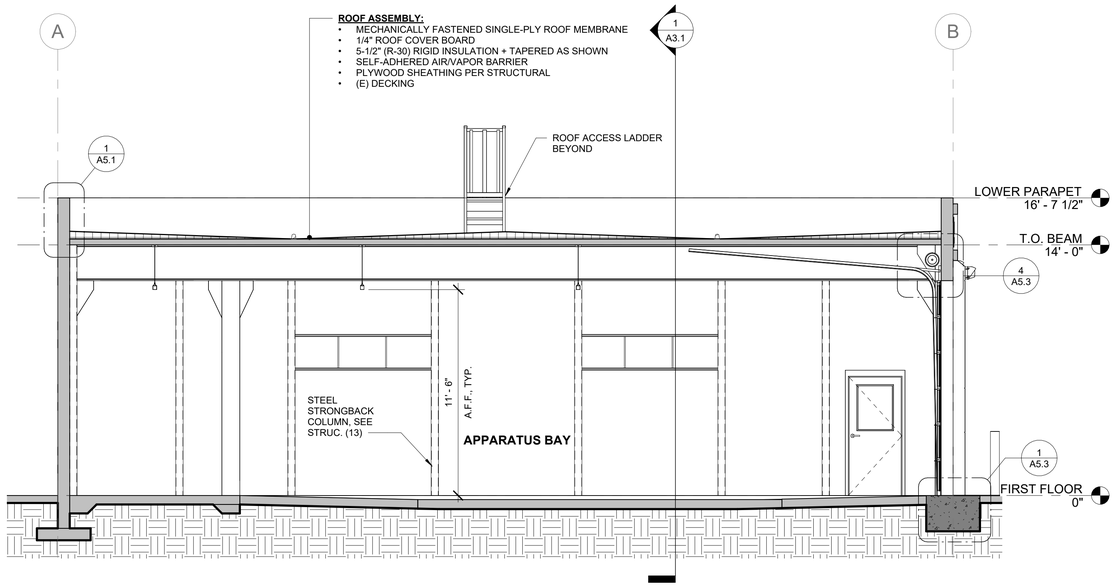
SHEET TITLE:
ROOF PLAN

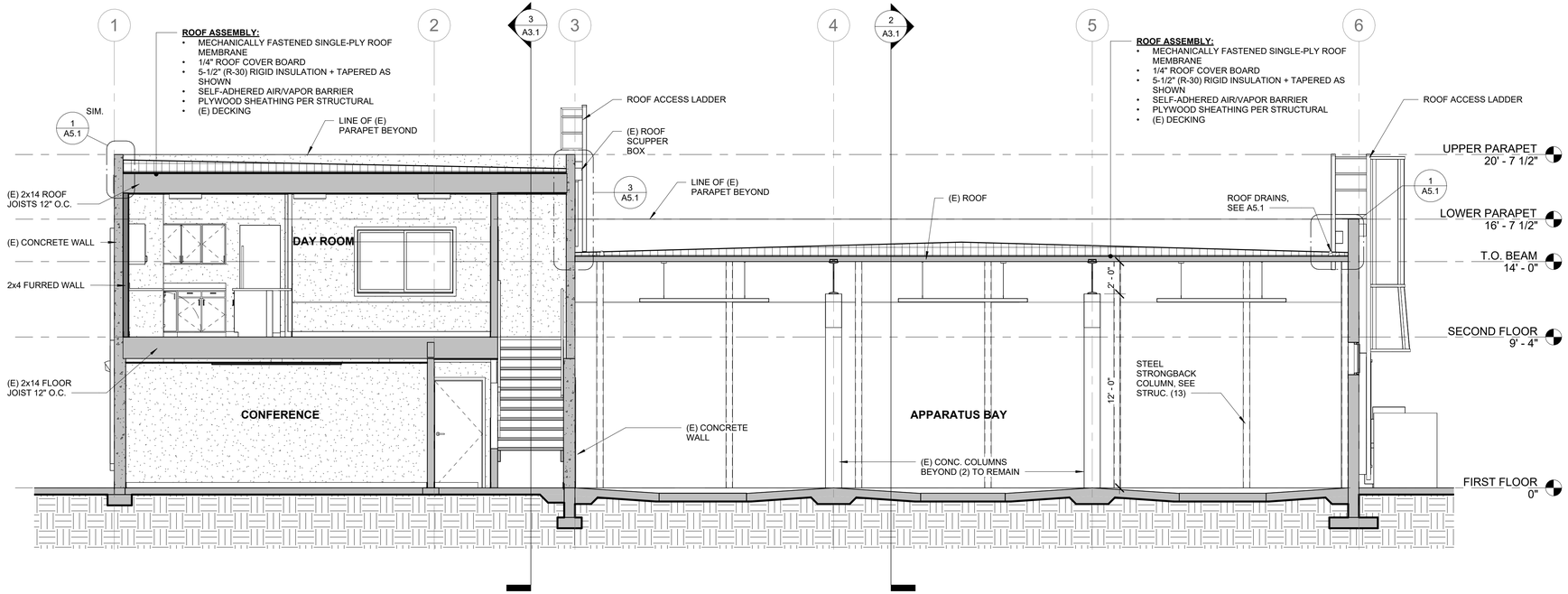
A2.4



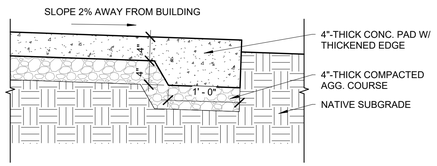
3 N-S SECTION 2
1/4" = 1'-0"



2 N-S SECTION 1
1/4" = 1'-0"



1 E-W SECTION 1
1/4" = 1'-0"



4 CONC. PAD EDGE (ALT. BID)
1" = 1'-0"

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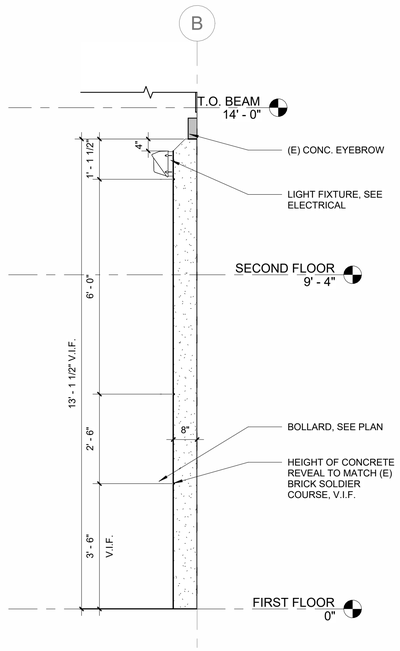
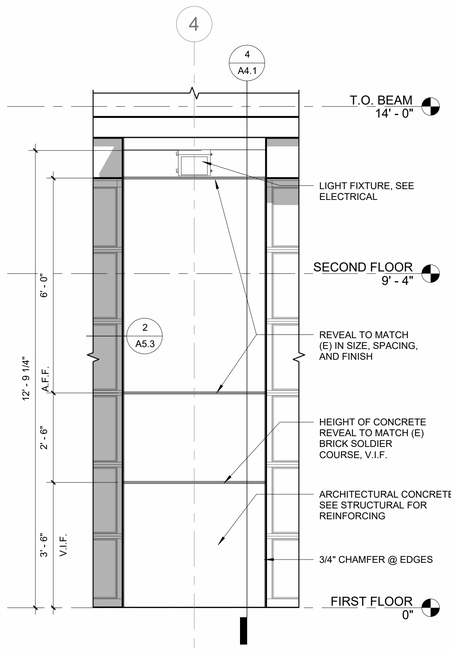
SHEET TITLE:
BUILDING SECTIONS

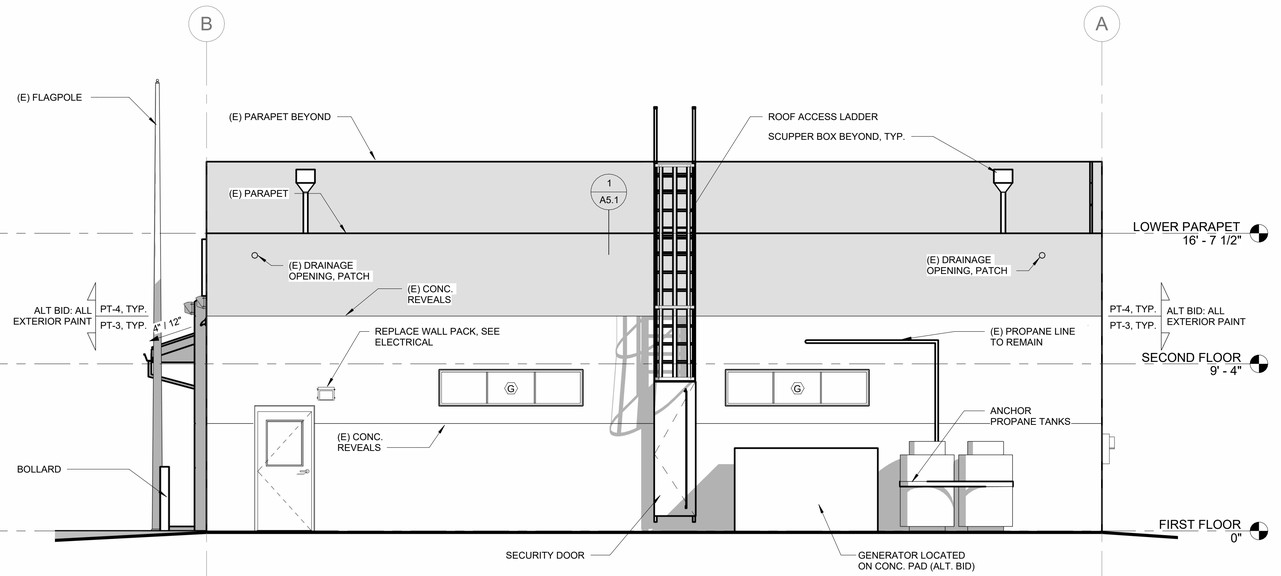
A3.1



4 REINFORCEMENT WALL SECTION
1/2" = 1'-0"

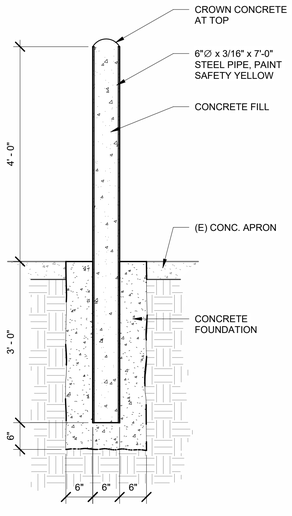


3 REINFORCEMENT WALL DETAIL
1/2" = 1'-0"

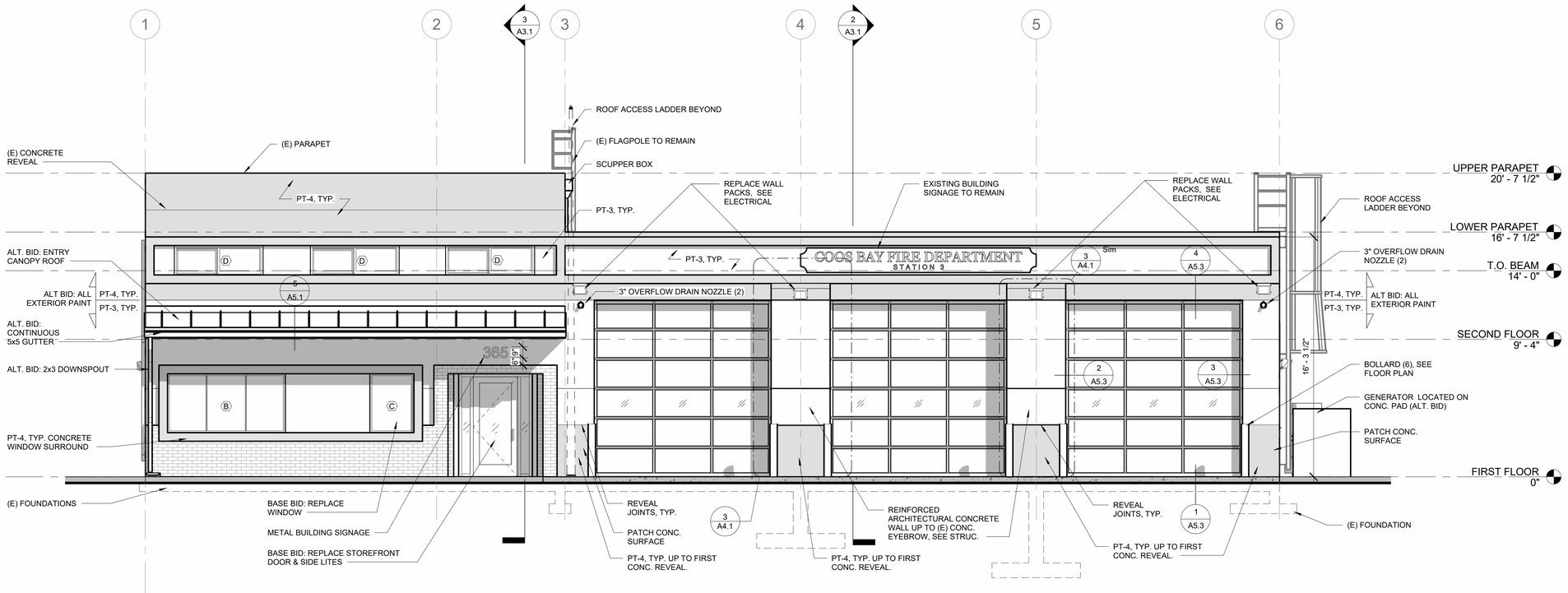


2 EAST ELEVATION
1/4" = 1'-0"

NOTE: REFER TO DOOR & WINDOW SCHEDULE FOR BASE BID AND ALT. BID (E) DOOR & WINDOW REPLACEMENTS



5 BOLLARD
3/4" = 1'-0"



1 SOUTH ELEVATION
1/4" = 1'-0"

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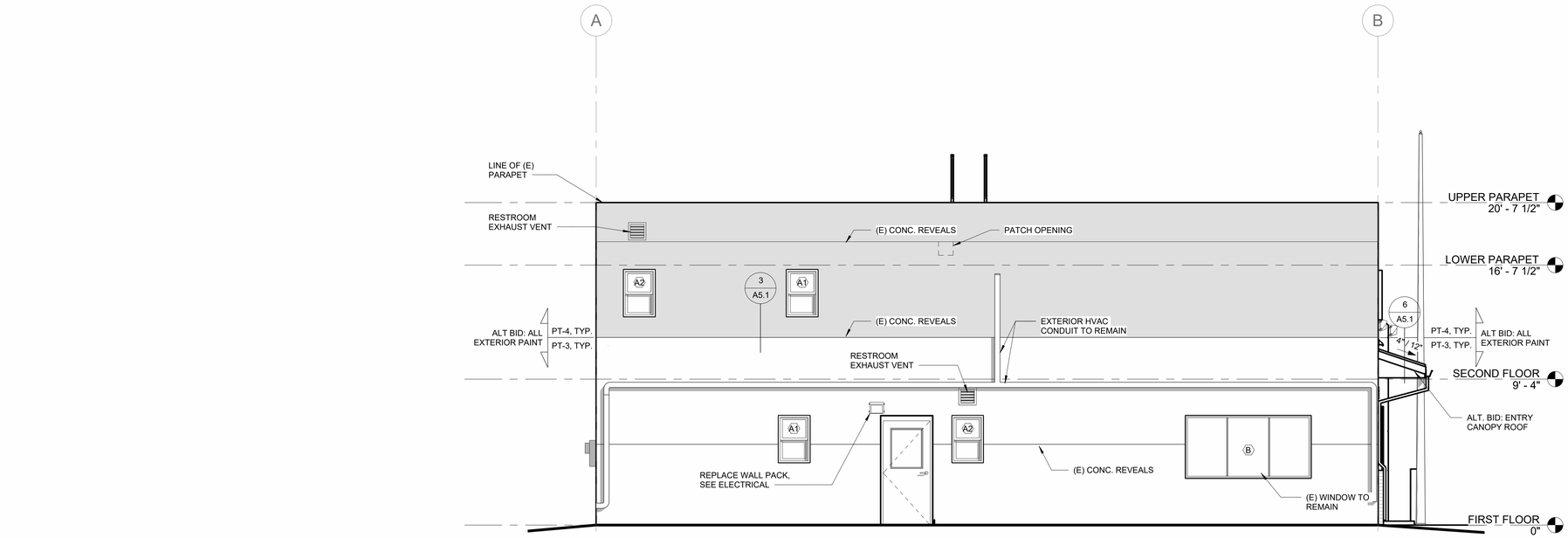
SHEET TITLE:
BUILDING ELEVATIONS

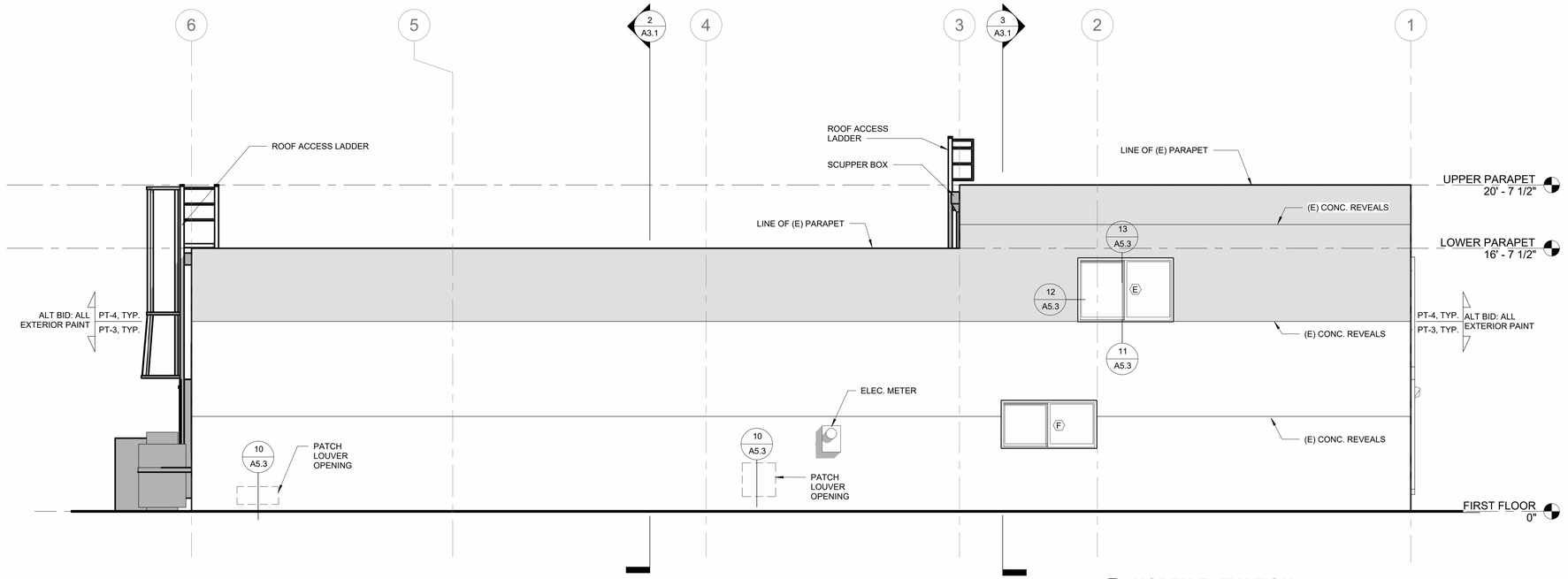
A4.1



2 WEST ELEVATION

1/4" = 1'-0"

NOTE: REFER TO DOOR & WINDOW SCHEDULE FOR BASE BID AND ALT. BID (E) DOOR & WINDOW REPLACEMENTS



1 NORTH ELEVATION

1/4" = 1'-0"

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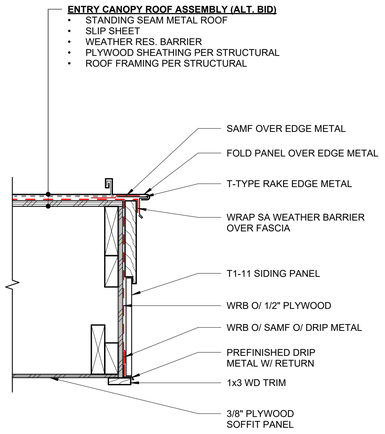
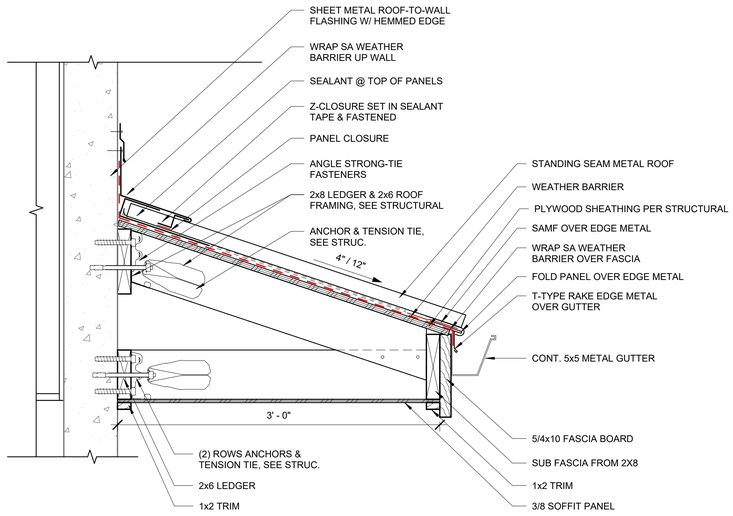
BUILDING ELEVATIONS

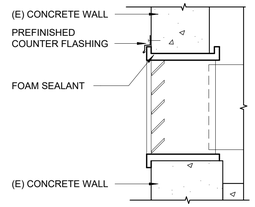
A4.2



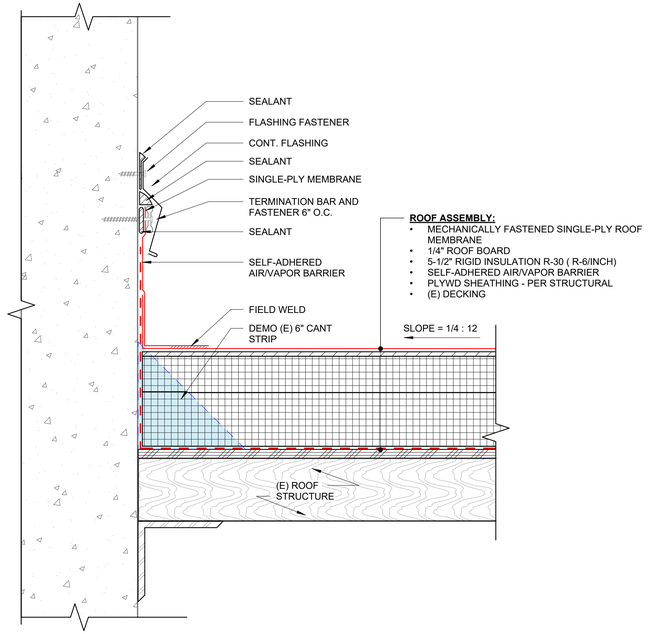
6 ALT. BID - ENTRY CANOPY ROOF RAKE
1 1/2" = 1'-0"



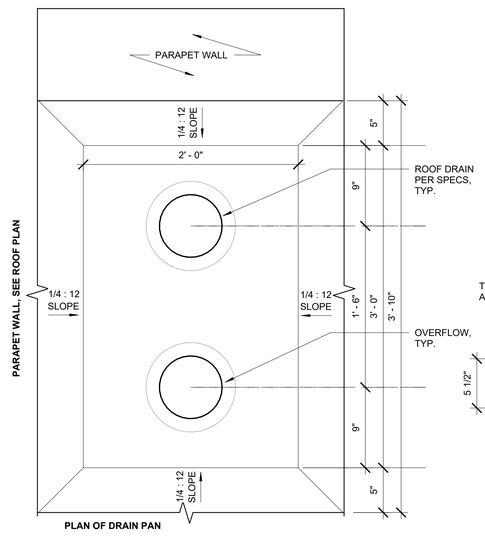
5 ALT. BID - ENTRY CANOPY ROOF
1 1/2" = 1'-0"

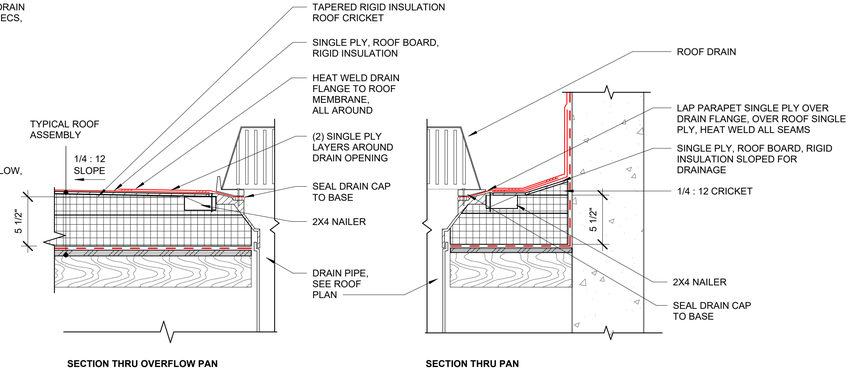


4 VENT IN (E) WALL
1 1/2" = 1'-0"

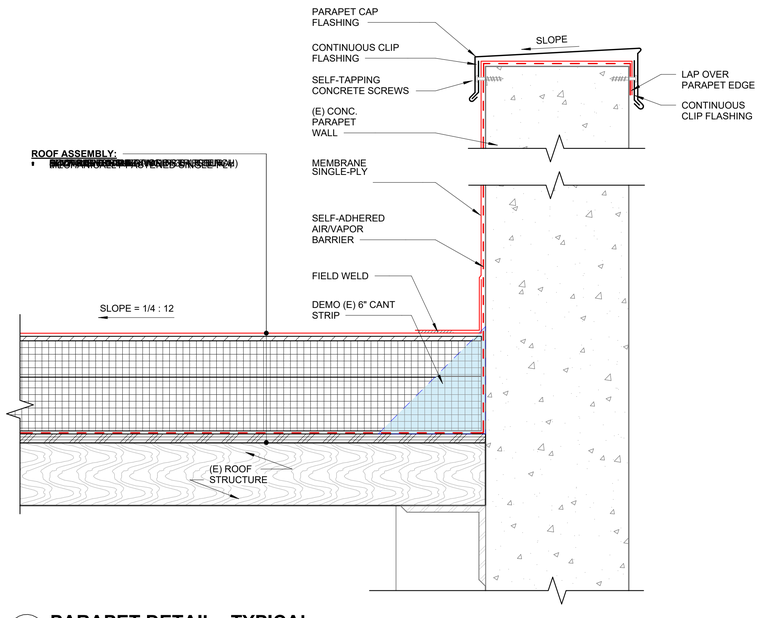


3 ROOF MEMBRANE TERMINATION @ TALL PARAPET
3" = 1'-0"





2 ROOF DRAIN & OVERFLOW - TYPICAL
1 1/2" = 1'-0"



1 PARAPET DETAIL - TYPICAL
3" = 1'-0"

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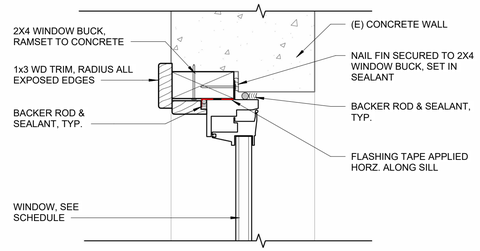
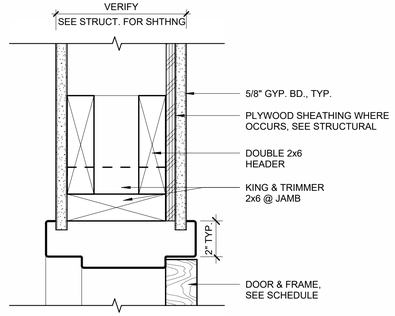
SHEET TITLE:
BUILDING DETAILS

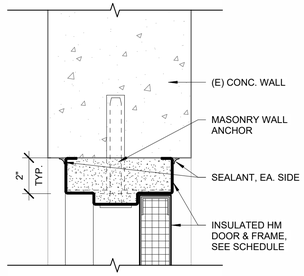
A5.1



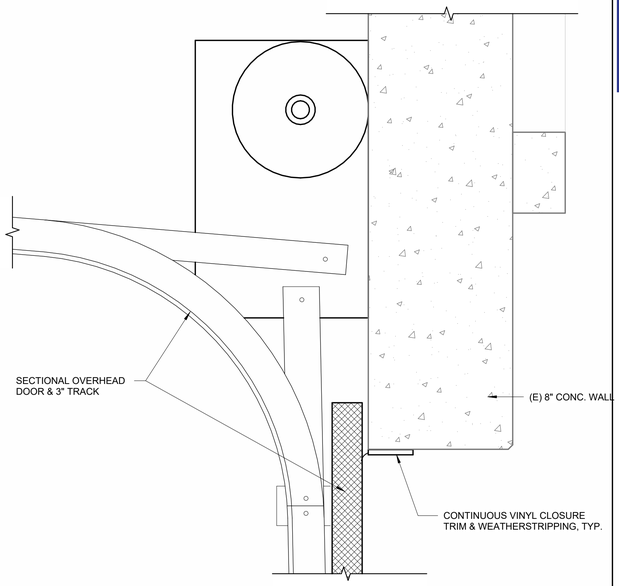
13 TYP. WINDOW HEAD (ALT. BID)
3" = 1'-0"



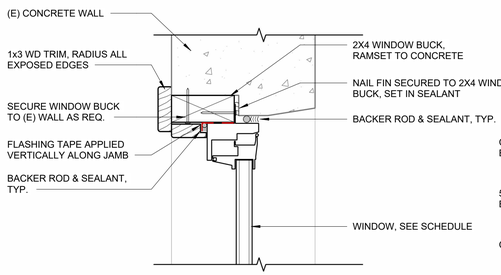
6 HM DOOR HEAD @ WOOD WALL, JAMB SIM.
3" = 1'-0"



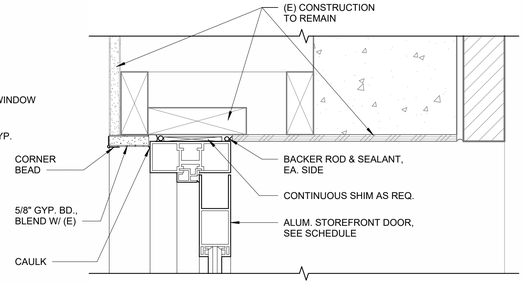
5 HM DOOR HEAD @ (E) CONC. WALL, JAMB SIM. (ALT. BID)
3" = 1'-0"



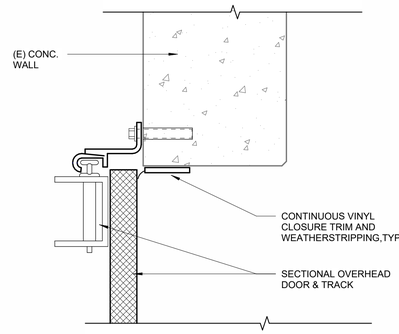
4 OVERHEAD DOOR HEAD
3" = 1'-0"



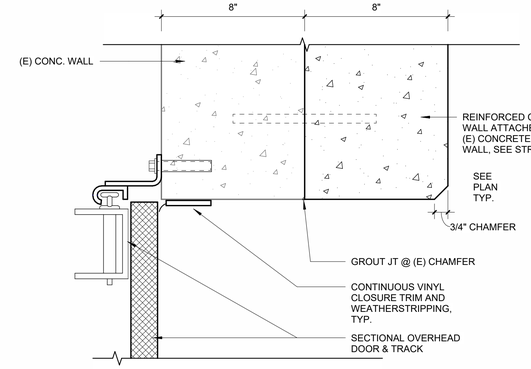
12 TYP. WINDOW JAMB (ALT. BID)
3" = 1'-0"



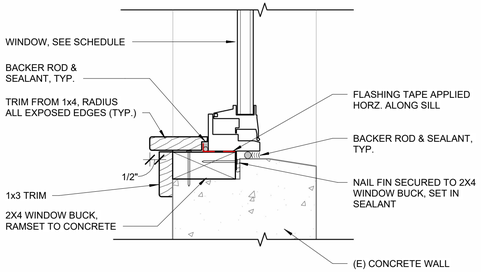
7 STOREFRONT DOOR HEAD
3" = 1'-0"



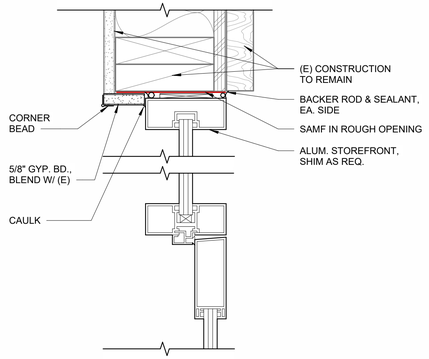
3 OVERHEAD DOOR JAMB @ (E) WALL
3" = 1'-0"



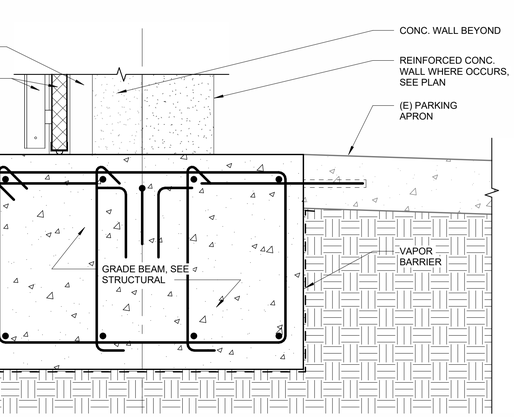
2 OVERHEAD DOOR JAMB @ REINFORCED CONC. WALL
3" = 1'-0"



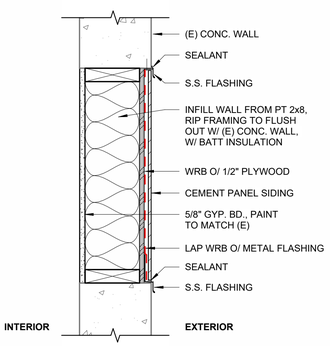
11 TYP. WINDOW SILL (ALT. BID)
3" = 1'-0"



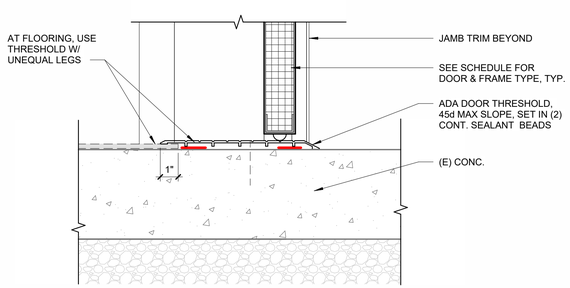
8 STOREFRONT JAMB
3" = 1'-0"



1 OVERHEAD DOOR @ GRADE BEAM
1 1/2" = 1'-0"



10 WALL PATCH, JAMB SIM.
1 1/2" = 1'-0"



9 EXTERIOR DOOR THRESHOLD
3" = 1'-0"

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REGISTERED ARCHITECT
2840
JOSEPH A. SLACK
COOS BAY, OREGON
STATE OF OREGON

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

PROJECT NO.: 22.22.2

CONSTRUCTION

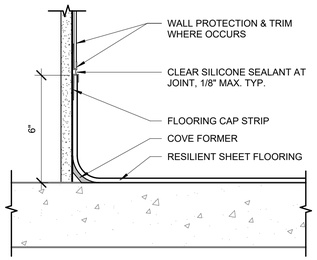
REVISIONS:

#	DATE	DESCRIPTION

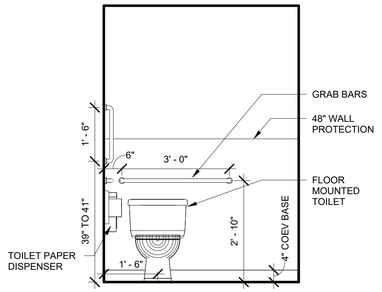
DATE: SEPTEMBER 2025

SHEET TITLE:
OPENING DETAILS

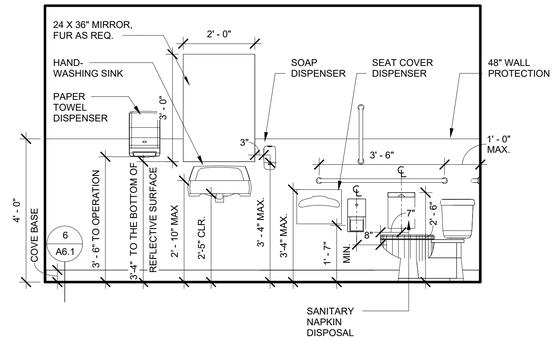
A5.3



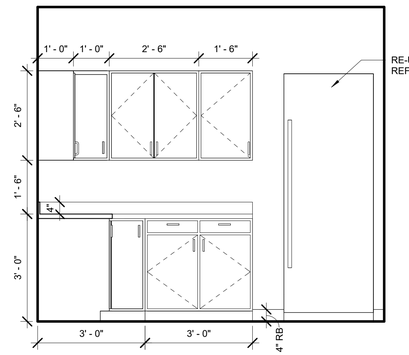
6 COVE BASE DETAIL
3" = 1'-0"



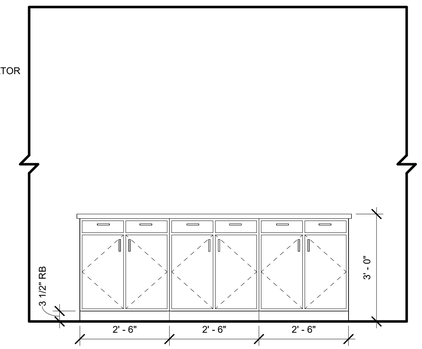
8 ADA R/R - WEST
1/2" = 1'-0"



9 ADA R/R - SOUTH
1/2" = 1'-0"

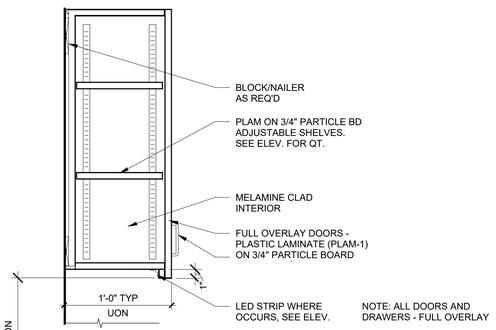


3 KITCHEN - NORTH - ALT BID
1/2" = 1'-0"

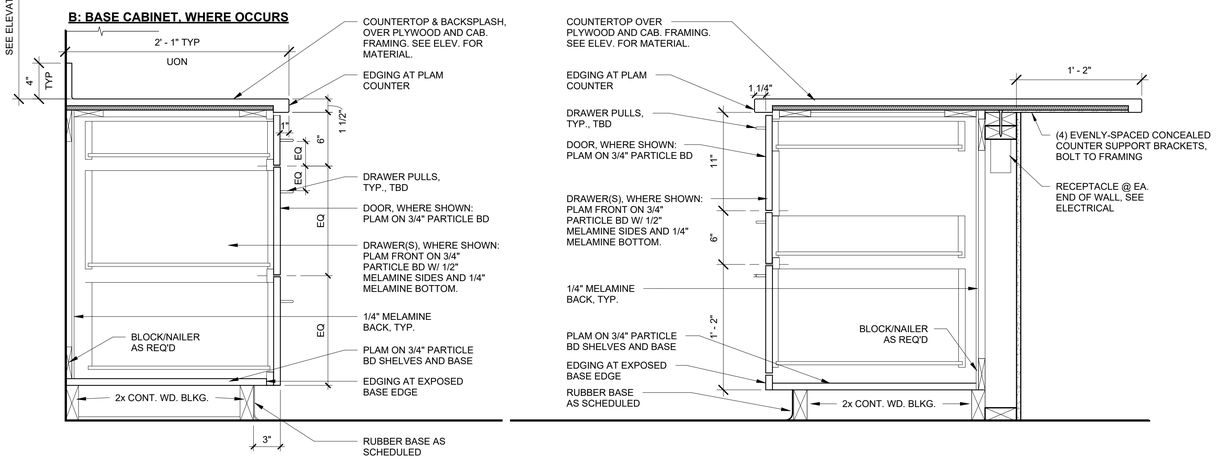


2 KITCHEN ISLAND - ALT BID
1/2" = 1'-0"

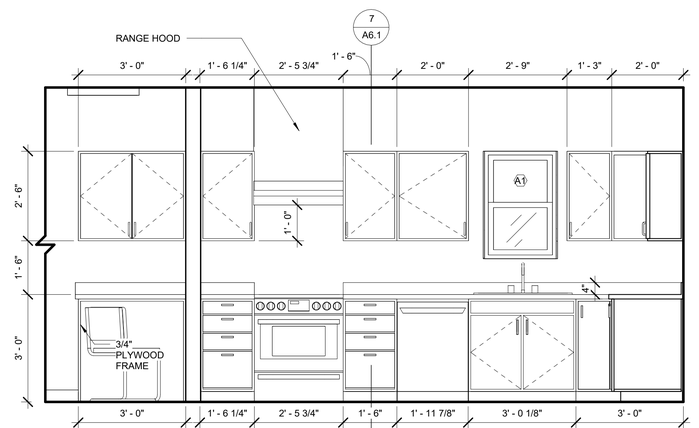
A: UPPER CABINET, WHERE OCCURS



B: BASE CABINET, WHERE OCCURS



7 CABINET DETAILS (ALT. BID)
1 1/2" = 1'-0"



1 KITCHEN - WEST - ALT BID
1/2" = 1'-0"

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PROJECT NO.: 22.22.2

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
INTERIOR ELEVATIONS

A6.1

FINISH LEGEND

ACT	ACOUSTIC CEILING TILE
CONC.	CONCRETE
FRP-1	FIBER REINFORCED POLYMER BOARD
GYP	GYPSUM BOARD
PT-1	PAINT COLOR TBD TYPICAL WALL
PT-2	PAINT COLOR TBD TYPICAL CEILING
PT-3	PAINT COLOR TBD EXTERIOR CONC. WALLS
PT-4	PAINT COLOR TBD EXTERIOR PANEL SIDING
LVT	LUXURY VINYL TILE FLOORING
RES	RESILIENT SHEET FLOORING & COVE BASE
RBR	RESILIENT RUBBER BASE, 4"

SEE SPECIFICATIONS FOR MORE

NOTE:

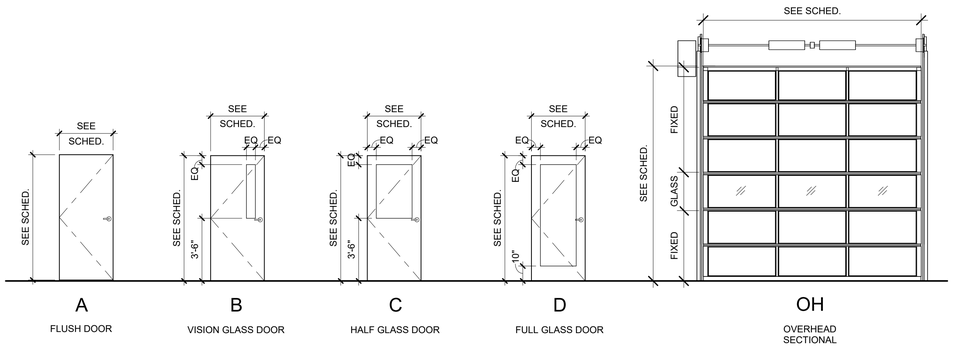
- **ALL EXTERIOR PAINT TO BE INCLUDED IN ALT BID ONLY.**

ROOM FINISH SCHEDULE

ROOM NAME	ROOM NO.	FLOOR FINISH	BASE	NORTH WALL	SOUTH WALL	EAST WALL	WEST WALL	CEILING FINISH	NOTES
HALL	1	LVT	RBR	GYP / PT-1	PT-1	PT-1	GYP / PT-1	GYP / PT-2	
CUST.	2	(E)	-	-	-	-	-	-	EXISTING TO REMAIN
CONFERENCE	3	LVT	RBR	GYP / PT-1	PT-1	GYP / PT-1	PT-1	GYP / PT-2 / ACT	
ADA R/R	4	RES	RES	FRP-1 / PT-1	FRP-1 / PT-1	FRP-1 / PT-1	FRP-1 / PT-1	GYP / PT-2	
(E) R/R	5	RES (ALT. BID)	RES (ALT. BID)	FRP-1 / PT-1 (ALT. BID)	FRP-1 / PT-1 (ALT. BID)	FRP-1 / PT-1 (ALT. BID)	FRP-1 / PT-1 (ALT. BID)	GYP / PT-2	BASE BID: PATCH BACK TO MATCH EXISTING @ SOUTH WALL
STORAGE	6	(E) CONC.	-	-	-	-	-	OPEN TO STRUCTURE	
APPARATUS BAY	7	(E) CONC.	-	PT-1	PT-1	PT-1	PT-1	(E) STRUCTURE / PT-2	
STAIR	21	LVT @ LANDING	RBR	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	
DAY ROOM	22	LVT (ALT. BID)	RBR (ALT. BID)	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	
KITCHEN	23	LVT (ALT. BID)	RBR (ALT. BID)	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	
R/R	24	(E)	(E)	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	
DORM 1	25	LVT	RBR	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	
DORM 2	26	LVT	RBR	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	
DORM 3	27	LVT	RBR	PT-1	PT-1	PT-1	PT-1	GYP / PT-2	

ROOM FINISH SCHEDULE NOTES:

1. REFER TO SCHEDULE FOR FINISHES INCLUDED IN ALT. BIDS.



DOOR LEGEND

1/4" = 1'-0"

DOOR SCHEDULE

DOOR NO.	ROOM NAME	SIZE (WxH)	TYPE	DOOR MATERIAL	FRAME MATERIAL	HARDWARE GROUP	NOTES
1A	HALL	3' - 0" X 6' - 9 1/2"	D	ALUM. / GLASS	ALUM.	HW-28	EXTERIOR STOREFRONT W/ SIDE LITES; NOTES 1, 2
1B	HALL	3' - 0" X 6' - 8"	C	WD / GLASS	HM	HW-45	NOTES 1, 2 (ALT. BID)
1C	HALL	2' - 6" X 6' - 8"	A	WD / GLASS	HM	HW-45	NOTES 1, 2 (ALT. BID)
1D	HALL	3' - 0" X 6' - 8"	C	HM / GLASS	HM	HW-55	EXTERIOR; NOTES 1, 2 (ALT. BID)
2	CUST.	2' - 6" X 6' - 8"	A	(E)	(E)	-	NOTE 3
3	CONFERENCE	3' - 0" X 6' - 8"	C	WD / GLASS	HM	HW-20	NOTE 2
4	ADA R/R	3' - 0" X 6' - 8"	A	WD	HM	HW-5	
5	(E) R/R	2' - 6" X 6' - 8"	A	WD	HM	HW-5	NOTE 2 (ALT. BID)
6	STORAGE	3' - 0" X 6' - 8"	A	WD	HM	HW-45	NOTES 1, 2
7A	APPARATUS BAY	12' - 0" X 12' - 0"	OH	MANUF.	MANUF.	-	SECTIONAL DOOR; NOTE 2
7B	APPARATUS BAY	12' - 0" X 12' - 0"	OH	MANUF.	MANUF.	-	SECTIONAL DOOR; NOTE 2
7C	APPARATUS BAY	12' - 0" X 12' - 0"	OH	MANUF.	MANUF.	-	SECTIONAL DOOR; NOTE 2
7D	APPARATUS BAY	3' - 0" X 6' - 8"	C	HM	HM	HW-55	EXTERIOR; NOTES 1, 2 (ALT. BID)
21	STAIR	3' - 0" X 6' - 8"	C	WD / GLASS	HM	HW-45	NOTE 1
24	R/R	2' - 6" X 6' - 8"	A	(E)	(E)	-	NOTE 3
25	DORM 1	2' - 6" X 6' - 8"	A	(E)	(E)	-	NOTE 3
26	DORM 2	2' - 6" X 6' - 8"	A	(E)	(E)	-	NOTE 3
27	DORM 3	2' - 6" X 6' - 8"	A	(E)	(E)	-	NOTE 3

DOOR SCHEDULE NOTES:

1. KEYPAD ACCESS CONTROL, SEE SPECIFICATIONS.
2. VERIFY EXISTING OPENING HEIGHT, WIDTH, AND JAMB DEPTH.
3. EXISTING DOOR TO REMAIN. PAINT DOOR & FRAME.
4. REFER TO SCHEDULE FOR DOORS INCLUDED IN ALT. BIDS.

WINDOW SCHEDULE

MARK	SIZE (WxH)	COUNT	TYPE	NOTES
A1	2' - 0" X 3' - 0"	2	SINGLE HUNG VINYL	NOTES 1, 2 (ALT. BID)
A2	2' - 0" X 3' - 0"	2	SINGLE HUNG VINYL	FROSTED PRIVACY GLASS; NOTES 1, 2 (ALT. BID)
B	8' - 0" X 4' - 0"	2	FIXED VINYL	NOTE 1 (ALT. BID)
C	3' - 0" X 4' - 0"	1	FIXED VINYL	NOTE 1
D	6' - 0" X 2' - 0"	3	HORIZ. SLIDER VINYL	NOTES 1, 2 (ALT. BID)
E	6' - 0" X 4' - 0"	1	HORIZ. SLIDER VINYL	NOTES 1, 2 (ALT. BID)
F	6' - 0" X 3' - 0"	1	HORIZ. SLIDER VINYL	NOTES 1, 2 (ALT. BID)
G	8' - 0" X 2' - 0"	2	FIXED VINYL	NOTE 1 (ALT. BID)

WINDOW SCHEDULE NOTES:

1. VERIFY EXISTING OPENING HEIGHTS & WIDTHS.
2. PROVIDE INSECT SCREEN.
3. REFER TO SCHEDULE FOR WINDOWS INCLUDED IN ALT. BIDS.

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REGISTERED ARCHITECT
2840
JOSEPH A. SLACK
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STATE OF OREGON

PROJECT NO.: 22.22.2

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:

SCHEDULES

A7.1

01.00.00 GENERAL REQUIREMENTS

1. **GOVERNING CODE:** THE DESIGN AND CONSTRUCTION OF THIS PROJECT IS GOVERNED BY THE "OREGON STRUCTURAL SPECIALTY CODE (OSSC)," 2022 EDITION, HEREAFTER REFERRED TO AS THE OSSC, AS ADOPTED AND MODIFIED BY THE CITY OF COOS BAY, OR UNDERSTOOD TO BE THE AUTHORITY HAVING JURISDICTION (AHJ).

2. **REFERENCE STANDARDS:** REFER TO CHAPTER 35 OF THE 2022 OSSC. WHERE OTHER STANDARDS ARE NOTED IN THE DRAWINGS, USE THE LATEST EDITION OF THE STANDARD UNLESS A SPECIFIC DATE IS INDICATED. REFERENCE TO A SPECIFIC SECTION IN A CODE DOES NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE ENTIRE STANDARD.

3. **DEFINITIONS:** THE FOLLOWING DEFINITIONS COVER THE MEANINGS OF CERTAIN TERMS USED IN THESE NOTES:
   - 3.1. "ARCHITECT/ENGINEER" - THE ARCHITECT OF RECORD AND THE STRUCTURAL ENGINEER OF RECORD.
   - 3.2. "STRUCTURAL ENGINEER OF RECORD" (SER) - THE STRUCTURAL ENGINEER WHO IS LICENSED TO STAMP AND SIGN THE STRUCTURAL DOCUMENTS FOR THE PROJECT. THE SER IS RESPONSIBLE FOR THE DESIGN OF THE PRIMARY STRUCTURAL SYSTEM.
   - 3.3. "SUBMIT FOR REVIEW" - SUBMIT TO THE ARCHITECT/SER FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION.
   - 3.4. "PER PLAN" - INDICATES REFERENCES TO THE STRUCTURAL PLANS, ELEVATIONS, AND STRUCTURAL GENERAL NOTES.
   - 3.5. "SEISMIC FORCE RESISTING SYSTEM" (SFRS) - A RECOGNIZED STRUCTURAL SYSTEM OF COMPONENTS (BEAMS, BRACES, DRAGS, STRUTS, COLLECTORS, DIAPHRAGMS, COLUMNS, WALLS, ETC.) OF THE PRIMARY STRUCTURE THAT ARE SPECIALLY DESIGNED AND PROPORTIONED TO RESIST EARTHQUAKE-INDUCED GROUND MOTIONS AND MAINTAIN STABILITY OF THE STRUCTURE. FABRICATION AND INSTALLATION OF COMPONENTS DESIGNATED AS PART OF THE SFRS REQUIRE THE GENERAL CONTRACTOR, SUBCONTRACTOR, OR SUPPLIER WHO IS RESPONSIBLE FOR ANY PORTION OF SFRS FABRICATION OR INSTALLATION TO COMPLY WITH SPECIAL REQUIREMENTS (INCLUDING, BUT NOT LIMITED TO, MATERIAL CONTROL, COMPLIANCE CERTIFICATIONS, PERSONNEL QUALIFICATIONS, DOCUMENTATION, REPORTING REQUIREMENTS, ETC.) AND TO PROVIDE THE REQUIRED QUALITY CONTROL INCLUDING THE REQUIRED COORDINATION OF SPECIAL INSPECTIONS (QUALITY ASSURANCE - QA). SPECIAL PROVISIONS APPLY TO ANY MEMBER DESIGNATED AS PART OF THE SFRS. REFER TO PLANS, ELEVATIONS, DETAILS, DESIGN CRITERIA, AND SYMBOLS AND LEGENDS FOR APPLICABLE MEMBERS AND CONNECTIONS.
   - 3.6. "SPECIALTY STRUCTURAL ENGINEER" (SSE) - A PROFESSIONAL ENGINEER (PE OR SE), LICENSED IN THE STATE IN WHICH THE PROJECT IS LOCATED (TYPICALLY NOT THE SER), WHO PERFORMS SPECIALTY STRUCTURAL ENGINEERING SERVICES FOR SELECTED SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS AND WHO HAS EXPERIENCE AND TRAINING IN THE SPECIALTY. DOCUMENTS STAMPED AND SIGNED BY THE SSE SHALL BE COMPLETED BY, OR UNDER, THE DIRECT SUPERVISION OF THE SSE.
   - 3.7. "BIDDER-DESIGNED" - COMPONENTS OF THE STRUCTURE THAT REQUIRE THE GENERAL CONTRACTOR, SUBCONTRACTOR, OR SUPPLIER WHO IS RESPONSIBLE FOR THE DESIGN, FABRICATION, AND INSTALLATION OF SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS TO RETAIN THE SERVICES OF AN SSE. SUBMITTALS OF "BIDDER-DESIGNED" ELEMENTS SHALL BE STAMPED AND SIGNED BY THE SSE.

4. **SPECIFICATIONS:** REFER TO THE PROJECT SPECIFICATIONS ISSUED AS PART OF THE CONTRACT DOCUMENTS FOR INFORMATION SUPPLEMENTAL TO THESE DRAWINGS.

5. **OTHER DRAWINGS:** REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL AND PLUMBING DRAWINGS FOR ADDITIONAL INFORMATION INCLUDING, BUT NOT LIMITED TO, DIMENSIONS, ELEVATIONS, SLOPES, DOOR AND WINDOW OPENINGS, NON-BEARING WALLS, STAIRS, FINISHES, DRAINS, WATERPROOFING, RAILINGS, CURTAIN WALLS, ELEVATORS, CURBS, DEPRESSIONS, MECHANICAL UNIT LOCATIONS, AND OTHER NON-STRUCTURAL ITEMS.

6. **STRUCTURAL DETAILS:** THE STRUCTURAL DRAWINGS ARE INTENDED TO SHOW THE GENERAL CHARACTER AND EXTENT OF THE PROJECT AND ARE NOT INTENDED TO SHOW ALL DETAILS OF THE WORK. USE ENTIRE DETAIL SHEETS AND SPECIFIC DETAILS REFERENCED IN THE PLANS AS "TYPICAL" WHEREVER THEY APPLY. SIMILARLY, USE DETAILS ON ENTIRE SHEETS WITH "TYPICAL" IN THE NAME WHEREVER THEY APPLY.

7. **STRUCTURAL RESPONSIBILITIES:** THE SER IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE PRIMARY STRUCTURE IN ITS COMPLETED FORM.

8. **COORDINATION:** THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING DETAILS AND ACCURACY OF THE WORK, CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS, SELECTING FABRICATION PROCESSES, TECHNIQUES OF ASSEMBLY, AND PERFORMING WORK IN A SAFE AND SECURE MANNER.

9. **EXISTING CONDITIONS:** INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENT THE PRESENT KNOWLEDGE BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH CONTRACT DOCUMENTS TO THE ARCHITECT OR SER. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION FROM THE ARCHITECT AND/OR SER. ALL EXISTING DIMENSIONS AND INFORMATION SHALL BE FIELD VERIFIED PRIOR TO FABRICATION AS REQUIRED TO COORDINATE WITH NEW CONSTRUCTION.

10. **NEW CONSTRUCTION:** THE CONTRACTOR SHALL REMOVE ALL INTERFERING ITEMS FOR NEW CONSTRUCTION AND SHALL REPAIR OR REPLACE ALL REMOVED ITEMS TO MATCH THE EXISTING CONDITIONS IN ACCORDANCE WITH THE ARCHITECTURAL DRAWINGS. NEW CONSTRUCTION ELEMENTS SHALL BE DESIGNED AND INSTALLED PER THE CURRENT IBC AS ALLOWED BY THE OSSC.

11. **PRE-CONSTRUCTION MEETINGS:** THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING PRE-CONSTRUCTION MEETINGS PRIOR TO COMMENCING WORK. PRE-CON MEETINGS, SCHEDULED APPROXIMATELY TWO WEEKS PRIOR TO THE START OF THE RELEVANT WORK, ARE REQUIRED FOR THE FOLLOWING PHASES OF CONSTRUCTION: STRUCTURAL STEEL, WOOD FRAMING, ATTENDEES FOR PRE-CONSTRUCTION MEETING ARE TO INCLUDE THE CONTRACTOR, RELEVANT SUBCONTRACTORS, FABRICATORS, INSPECTORS, ARCHITECT/SER, AND A REPRESENTATIVE OF THE AHJ WHERE REQUIRED. MEETING AGENDAS ARE TO INCLUDE REVIEW OF THE WORK SCOPE, PROJECT SCHEDULE RELEVANT TO THE WORK, CONTACT INFORMATION OF RESPONSIBLE PARTIES, INSPECTION POINTS, REVIEW OF MATERIALS AND ANY SPECIAL CASES OR ISSUES, PROCEDURES FOR CLARIFICATIONS IF REQUIRED, TESTING AND ACCEPTANCE, ETC.

12. **MEANS, METHODS, AND SAFETY REQUIREMENTS:** THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB-RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH). THE CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION RELATED TO THE INTERMEDIATE STRUCTURAL CONDITIONS (I.E., MOVEMENT OF THE STRUCTURE DUE TO MOISTURE AND THERMAL EFFECTS, CONSTRUCTION SEQUENCE, TEMPORARY BRACING, ETC.).

13. **BRACING/SHORING DESIGN ENGINEER:** THE CONTRACTOR SHALL, AT THEIR DISCRETION, EMPLOY AN SSE FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING. SUBMIT CONSTRUCTION SEQUENCE TO ARCHITECT/ENGINEER FOR REVIEW.

14. **TEMPORARY SHORING, BRACING:** THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING, AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

15. **CONSTRUCTION LOADS:** LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA AND LOADS SECTION OF THESE GENERAL NOTES OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

16. **CHANGES IN LOADING:** THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE SER OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON, THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL, STRUCTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF THE LOCATION, LOAD, SIZE, AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 LB. PROVIDE MARKED-UP STRUCTURAL PLANS INDICATING THE LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT THESE PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

17. **NOTE PRIORITIES:** PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THESE GENERAL NOTES.

18. **DISCREPANCIES:** IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS, PLANS/DETAILS, OR REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.

19. **SITE VERIFICATION:** THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.

20. **ADJACENT UTILITIES:** THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATION WORK, SHORING AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NECESSARILY COMPLETE.

21. **ALTERNATES:** ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE, AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION (PROPER TEST REPORT, ETC.) TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.

22. **NARRATIVE:** THE FOLLOWING ITEMS WILL BE REPAIRED, MODIFIED, OR ADDED FOR THIS PROJECT:
   - 22.1. THE BUILDING IS A TWO STORY REINFORCED CONCRETE WALL BUILDING WITH WOOD FRAMING AT THE SECOND STORY FLOOR, ROOF AN PARTITION WALLS BUILT IN THE 1950. THE SEISMIC UPGRADE PROPOSED IN THESE PLANS ARE TO REMEDY TIER 1 DEFICIENCIES DESCRIBED IN THE DCI ENGINEERS REPORT DATED 01/04/2024.

23. **ADDITIONS/ALTERATIONS/REPAIRS:**
   - 23.1. ADDITIONS, ALTERATIONS, AND/OR REPAIRS TO THE EXISTING STRUCTURE HAVE BEEN ANALYZED FOR ADDITIONAL LOADING AND/OR MODIFICATION DUE TO THE ADDITION, ALTERATION, OR REPAIR.
   - 23.2. ALL AFFECTED EXISTING MEMBERS HAVE BEEN ANALYZED OR REINFORCED AS REQUIRED PER THE IEBC.
   - 23.3. ALL DEMOLITION OR REMOVAL OF ARCHITECTURAL, MECHANICAL, OR STRUCTURAL ELEMENTS SHALL NOT DAMAGE STRUCTURAL ITEMS TO REMAIN.

01.10.00 DESIGN CRITERIA AND LOADS

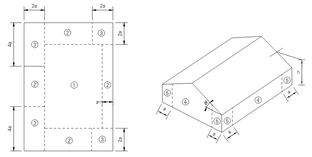
1. **OCCUPANCY:**

RISK CATEGORY OF BUILDING PER OSSC TABLE 1604.5	IV

2. **WIND DESIGN: MAIN WIND FORCE RESISTING SYSTEM:**

ULTIMATE DESIGN WIND SPEED, V_{ULT}	135 MPH
EXPOSURE CATEGORY	B
INTERNAL PRESSURE COEFFICIENT, C_{pi}	±0.18
TOPOGRAPHIC FACTOR, K_{zt}	1.0
WIND ANALYSIS PROCEDURE USED	ENVELOPE

3. **WIND DESIGN: COMPONENTS AND CLADDING (C&C) PRESSURES FOR DESIGN (PSF, ULTIMATE):**



EFFECTIVE WIND AREA (FT²)

a = 5'-0"	10	20	50	100	500
ZONE 1	-16 / +16	-16 / +16	-16 / +16	-16 / +16	-16 / +16
ZONE 2	-39 / +16	-39 / +16	-38 / +16	-37 / +16	-37 / +16
ZONE 2'	-47 / +16	-47 / +16	-45 / +16	-45 / +16	-45 / +16
ZONE 3	-53 / +16	-48 / +16	-42 / +16	-37 / +16	-37 / +16
ZONE 3'	-74 / +16	-66 / +16	-55 / +16	-47 / +16	-47 / +16
ZONE 4	-31 / +29	-30 / +28	-28 / +26	-27 / +25	-24 / +22
ZONE 5	-38 / +29	-36 / +28	-33 / +26	-30 / +25	-24 / +22

   - 3.1. COMPONENTS AND CLADDING WIND PRESSURES ARE BASED ON ASCE 7 CHAPTER 30 PART 1 "LOW-RISE BUILDINGS."
   - 3.2. COMPONENTS AND CLADDING ZONE LOCATIONS ARE BASED ON ASCE 7 FIGURE 30.3-5A FOR MONOSLOPE ROOFS 3° < Θ ≤ 10°.
   - 3.3. COMPONENTS AND CLADDING ZONE LOCATIONS ARE BASED ON ASCE 7 FIGURE 30.3-1 FOR WALLS.
   - 3.4. ALL PARAPET COMPONENTS AND CLADDING WIND PRESSURES SHALL BE DETERMINED THROUGH ASCE 7 FIGURE 30.6-1.

4. **SEISMIC DESIGN:**

SEISMIC DESIGN CATEGORY, SDC	D
SITE CLASS PER IEBC SECTION 301.1.4.2 AND ASCE 41 CHAPTER 2.4	D
BASIC SAFETY EARTHQUAKES (BSE)	
BSE-1E, LIFE SAFETY	
MAPPED RESPONSE ACCELERATION (SHORT PERIOD), S_{XS}	0.279g
MAPPED RESPONSE ACCELERATION (1-SECOND PERIOD), S_{X1}	0.082g
BSE-2E, IMMEDIATE OCCUPANCY	
MAPPED RESPONSE ACCELERATION (SHORT PERIOD), S_{XS}	0.834g
MAPPED RESPONSE ACCELERATION (1-SECOND PERIOD), S_{X1}	0.33g
SEISMIC ANALYSIS PROCEDURE USED	LINEAR STATIC
BASIC STRUCTURAL SYSTEM	BEARING WALL SYSTEMS
SEISMIC FORCE RESISTING SYSTEM	CONCRETE SHEAR WALLS

5. **SNOW LOAD:**

FLAT ROOF SNOW LOAD, P_f	25 PSF [NOTE 5.1]
SNOW DRIFT LOADING REQUIRED BY AHJ?	YES
SNOW LOAD IMPORTANCE FACTOR, I_s	1.20 [NOTE 5.3]
GROUND SNOW LOAD, P_g	1 PSF
SNOW EXPOSURE FACTOR, C_e	0.9
THERMAL FACTOR, C_t	1.0
SEE ROOF PLAN FOR DRIFT LOADING	

   - 5.1. SNOW LOAD IS UN-REDUCIBLE AND INCLUDES 5 PSF RAIN-ON-SNOW SURCHARGE WHERE GROUND SNOW LOAD IS 20 PSF OR LESS, BUT NOT ZERO, PER ASCE 7 SECTION 7.10.
   - 5.2. SNOW LOAD BASED ON ASCE FIGURE 7-1.
   - 5.3. SNOW LOAD IMPORTANCE FACTOR PER ASCE 7 TABLE 1.5-2.

6. **DESIGN LIVE LOADS:** SEE STRUCTURAL LOADING PLANS FOR AREA LOADS AND LINE LOADS. LOADS LISTED BELOW ARE FOR MISCELLANEOUS ITEMS.

AREA	LIVE LOADS
INTERIOR WALLS AND PARTITIONS	5 (APPLIED HORIZONTALLY)
ROOFS	20 PSF (REDUCIBLE) OR 300 LB [NOTE 6.1]. SEE SECTION 5 FOR SNOW LOAD

7. **DESIGN DEAD LOADS:**

ROOF DEAD LOAD (IN ADDITION TO STRUCTURE SELF-WEIGHT)	
ROOFING	20 PSF

8. **DEFLECTION LIMITS FOR SSE / BIDDER-DESIGNED ELEMENTS:**

VERTICAL [NOTE 8.1]	LIMIT
ROOF MEMBERS, DEAD + LIVE/SNOW/WIND, TOTAL LOAD (TL)	L / 240
ROOF, LIVE/SNOW/WIND LOAD (RLL)	L / 360

   - 8.1. MAXIMUM VERTICAL DEFLECTION IS WHICHEVER CRITERIA YIELDS LESS DEFLECTION. L IS THE CLEAR SPAN LENGTH IN INCHES.
   - 8.2. WIND LOAD IS REDUCIBLE TO 0.42 * THE COMPONENT AND CLADDING (C&C) LOADS PER TABLE 1604.3 FOOTNOTE F.

01.20.00 SUBMITTALS

1. **SUBMIT FOR REVIEW:** SUBMITTALS OF SHOP DRAWINGS, PRODUCT DATA, AND MILL TESTS ARE REQUIRED FOR ITEMS NOTED IN THE INDIVIDUAL MATERIALS SECTIONS OF THESE GENERAL NOTES AND FOR BIDDER-DESIGNED ELEMENTS.

2. **SUBMITTAL REVIEW PERIOD:** SUBMITTALS SHALL BE MADE IN TIME TO PROVIDE A MINIMUM OF TWO WEEKS OR TEN WORKING DAYS FOR REVIEW BY THE ARCHITECT/ENGINEER PRIOR TO THE ONSET OF FABRICATION.

3. **GENERAL CONTRACTOR'S PRIOR REVIEW:** PRIOR TO SUBMISSION TO THE ARCHITECT/ENGINEER, THE CONTRACTOR SHALL REVIEW THE SUBMITTAL FOR COMPLETENESS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE SER; THEREFORE, THEY MUST BE VERIFIED BY THE GENERAL CONTRACTOR. THE CONTRACTOR SHALL PROVIDE ANY NECESSARY DIMENSIONAL DETAILS REQUESTED BY THE DETAILER AND PROVIDE THE CONTRACTOR'S REVIEW STAMP AND SIGNATURE BEFORE FORWARDING TO THE ARCHITECT/ENGINEER.

4. **SHOP DRAWING REVIEW:** ONCE THE CONTRACTOR HAS COMPLETED THEIR REVIEW, THE SER WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND THE CONTRACT DOCUMENTS OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. MARKINGS OR COMMENTS SHALL NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR FROM COMPLIANCE WITH THE PROJECT PLANS AND SPECIFICATIONS, NOR DEPARTURES THERE FROM. THE SER WILL RETURN SUBMITTALS IN THE FORM IN WHICH THEY ARE SUBMITTED (EITHER HARD COPY OR ELECTRONIC). FOR HARD COPY SUBMITTALS, THE CONTRACTOR IS RESPONSIBLE FOR SUBMITTING THE REQUIRED NUMBER OF COPIES TO THE SER FOR REVIEW.

5. **SHOP DRAWING DEVIATIONS:** WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS, THEY SHALL BE DESIGNED AND STAMPED BY THE RESPONSIBLE SSE.

01.30.00 DEFERRED SUBMITTALS

1. **BIDDER-DESIGNED ELEMENTS:** SUBMIT "BIDDER-DESIGNED" DEFERRED SUBMITTALS TO THE ARCHITECT AND SER FOR REVIEW. THE DEFERRED SUBMITTALS SHALL ALSO BE SUBMITTED TO THE CITY FOR APPROVAL IF REQUIRED BY THE CITY. DESIGN OF PREFABRICATED, "BIDDER-DESIGNED," MANUFACTURED, PRE-ENGINEERED, OR OTHER FABRICATED PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:
   - 1.1. DESIGN CONSIDERS TRIBUTARY DEAD, LIVE, WIND, AND EARTHQUAKE LOADS IN COMBINATIONS REQUIRED BY THE OSSC.
   - 1.2. DESIGN WITHIN THE DEFLECTION LIMITS NOTED IN THESE GENERAL NOTES AND AS SPECIFIED OR REFERENCED IN THE OSSC.
   - 1.3. DESIGN SHALL CONFORM TO THE SPECIFICATIONS AND REFERENCE STANDARDS OF THE GOVERNING CODE.
   - 1.4. SUBMITTAL SHALL INCLUDE:
     - 1.4.1. CALCULATIONS PREPARED, STAMPED, AND SIGNED BY THE SSE DEMONSTRATING CODE CONFORMANCE.
     - 1.4.2. ENGINEERED COMPONENT DESIGN DRAWINGS ARE PREPARED, STAMPED, AND SIGNED BY THE SSE.
     - 1.4.3. PRODUCT DATA, TECHNICAL INFORMATION AND MANUFACTURER'S WRITTEN REQUIREMENTS, AND AGENCY APPROVALS AS APPLICABLE.
     - 1.4.4. THE SSE MAY SUBMIT TO THE ARCHITECT/ENGINEER A REQUEST TO UTILIZE RELEVANT ALTERNATE DESIGN CRITERIA OF SIMILAR NATURE AND GENERAL EQUIVALENCY WHICH IS RECOGNIZED BY THE CODE AND ACCEPTABLE TO THE AHJ. SUBMIT ADEQUATE DOCUMENTATION OF DESIGN.

2. **GENERAL CONTRACTOR'S PRIOR REVIEW:**
   - 2.1. ONCE THE CONTRACTOR HAS COMPLETED THEIR REVIEW OF THE SSE COMPONENT DRAWINGS, THE SER WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY.
   - 2.2. REVIEW OF THE SSE'S SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE SSE OF RESPONSIBILITY FOR THAT DESIGN.
   - 2.3. ALL NECESSARY BRACING, TIES, ANCHORAGE, AND PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE SSE'S DESIGN DRAWINGS AND CALCULATIONS.
   - 2.4. BIDDER-DESIGNED ELEMENTS INCLUDE BUT ARE NOT LIMITED TO:
     - 2.4.1. MECHANICAL, ELECTRICAL, PLUMBING, AND SPRINKLER HANGER PLANS
     - 2.4.2. MICROPILES / HELICAL PILES
     - 2.4.3. FAÇADE ACCESS EQUIPMENT / LIFELINE SUPPORTS

01.40.00 INSPECTIONS, QUALITY ASSURANCE, AND TEST REQUIREMENTS

1. **INSPECTIONS:** FOUNDATIONS, FOOTINGS, AND UNDER SLAB SYSTEMS AND FRAMING ARE SUBJECT TO INSPECTION BY THE BUILDING OFFICIAL IN ACCORDANCE WITH OSSC SECTION 110.3. THE CONTRACTOR SHALL COORDINATE ALL REQUIRED INSPECTIONS WITH THE BUILDING OFFICIAL.

2. **SPECIAL INSPECTIONS, VERIFICATIONS, AND TESTS:** SPECIAL INSPECTIONS, VERIFICATIONS, AND TESTING SHALL BE DONE IN ACCORDANCE WITH OSSC CHAPTER 17, THE STATEMENT AND SCHEDULES OF SPECIAL INSPECTIONS LISTED IN THESE DRAWINGS, AND THE AHJ STATEMENT OF SPECIAL INSPECTION AND/OR STATEMENT OF STRUCTURAL OBSERVATIONS.

3. **STRUCTURAL OBSERVATION:**
   - 3.1. STRUCTURAL OBSERVATION FOR THIS PROJECT IS REQUIRED PER OSSC SECTION 1704.6. THE CONTRACTOR SHALL NOTIFY THE SER IN A TIMELY MANNER TO ALLOW REQUIRED STRUCTURAL OBSERVATIONS TO OCCUR. REPORTS WILL BE DISTRIBUTED TO THE ARCHITECT, CONTRACTOR, SPECIAL INSPECTOR, AND AHJ.
     - 3.1.1. THE FREQUENCY AND EXTENT OF OBSERVATIONS IS AT THE DISCRETION OF THE STRUCTURAL OBSERVER.
     - 3.1.2. ONLY SIGNIFICANT STAGES OF CONSTRUCTION IDENTIFIED BY THE STRUCTURAL OBSERVER REQUIRE OBSERVATION.
     - 3.1.3. FOR REPETITIVE OR SIMILAR STRUCTURAL ELEMENTS IDENTIFIED AS SIGNIFICANT, ONLY THE FIRST ELEMENT OF A STAGE REQUIRES OBSERVATION UNLESS NOTED OTHERWISE.
     - 3.1.4. THE FOLLOWING SIGNIFICANT STAGES OF CONSTRUCTION REQUIRE OBSERVATION:
       - 3.1.4.1. PRIOR TO FOUNDATION CONCRETE PLACEMENT
       - 3.1.4.2. AFTER ROOF DIAPHRAGM IS COMPLETE PRIOR TO ROOFING

4. **CONTRACTOR RESPONSIBILITY:** PRIOR TO ISSUANCE OF THE BUILDING PERMIT, THE CONTRACTOR IS REQUIRED TO PROVIDE THE AHJ A SIGNED, WRITTEN ACKNOWLEDGEMENT OF THE CONTRACTOR'S RESPONSIBILITIES ASSOCIATED WITH THE STATEMENT OF SPECIAL INSPECTIONS, PREVIOUSLY REFERENCED, ADDRESSING THE REQUIREMENTS LISTED IN OSSC SECTION 1704.4. THE CONTRACTOR IS REFERRED TO OSSC SECTIONS 1705.13.5 AND 1705.13.6 FOR ARCHITECTURAL AND MEP BUILDING SYSTEMS THAT MAY BE SUBJECT TO ADDITIONAL INSPECTIONS (BASED ON THE BUILDING'S DESIGNATED SEISMIC DESIGN CATEGORY LISTED IN THE DESIGN CRITERIA AND LOADS SECTION OF THESE GENERAL NOTES), INCLUDING ANCHORAGE OF HVAC DUCTWORK CONTAINING HAZARDOUS MATERIALS, PIPING SYSTEMS AND MECHANICAL UNITS CONTAINING FLAMMABLE, COMBUSTIBLE, OR HIGHLY TOXIC MATERIALS, ELECTRICAL EQUIPMENT USED FOR EMERGENCY OR STANDBY POWER, EXTERIOR WALL PANELS, AND SUSPENDED CEILING SYSTEMS.

02.00.00 SOILS AND FOUNDATION

1. **REFERENCE STANDARDS:** CONFORM TO OSSC CHAPTER 18 "SOILS AND FOUNDATIONS"

2. **GEOTECHNICAL REPORT:** RECOMMENDATIONS CONTAINED IN GEOTECHNICAL INVESTIGATION REPORT AND SITE SEISMIC HAZARD STUDY EASTSIDE FIRE STATION #3 IMPROVEMENTS REPORT NO. 10-242116-001 BY EARTH ENGINEERS DATED 02/07/2025 WERE USED FOR DESIGN.

3. **CONTRACTOR'S RESPONSIBILITIES:** CONTRACTOR SHALL BE RESPONSIBLE TO REVIEW THE GEOTECHNICAL REPORT AND SHALL FOLLOW THE RECOMMENDATIONS SPECIFIED THEREIN INCLUDING, BUT NOT LIMITED TO, SUBGRADE PREPARATIONS, PILE INSTALLATION PROCEDURES, GROUND WATER MANAGEMENT, AND STEEP SLOPE BEST MANAGEMENT PRACTICES.

4. **GEOTECHNICAL SUBGRADE INSPECTION:** THE GEOTECHNICAL ENGINEER SHALL INSPECT ALL SUBGRADES AND PREPARED SOIL BEARING SURFACES PRIOR TO THE PLACEMENT OF FOUNDATION REINFORCING STEEL AND CONCRETE. GEOTECHNICAL ENGINEERS SHALL PROVIDE A LETTER TO THE OWNER STATING THAT SOILS ARE ADEQUATE TO SUPPORT THE "ALLOWABLE FOUNDATION BEARING PRESSURE(S)" SHOWN BELOW. ASSUMED VALUES SHALL BE FIELD VERIFIED BY THE BUILDING OFFICIAL OR THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.

DRAWING LEGEND

MARK	DESCRIPTION	MARK	DESCRIPTION
F2.0	FOOTING SYMBOL (REFER TO SPREAD FOOTING SCHEDULE)	Ɪ	INDICATES WIDE FLANGE COLUMN
▨	INDICATES CONCRETE COLUMN	□	INDICATES HOLLOW STRUCTURAL SECTION (HSS) COLUMN OR TUBE STEEL (TS) COLUMN
▩	INDICATES PRECAST CONCRETE COLUMN	○	INDICATES HOLLOW STRUCTURAL SECTION (HSS) COLUMN OR STEEL PIPE COLUMN
2W4	SHEAR WALL SYMBOL (REFER TO SHEAR WALL SCHEDULE)	⊠	INDICATES WOOD POST
△0 RFI 00	REVISION TRIANGLE	■	INDICATES BUNDLED STUDS
T/FTG = X'-X"	ELEVATION SYMBOL (T/ REFERS TO COMPONENT THAT THE ELEVATION REFERENCES)	▶—	INDICATES MOMENT FRAME CONNECTION
③	STUD BUBBLE (INDICATES NUMBER OF STUDS REQUIRED IF EXCEEDS NUMBER SPECIFIED IN PLAN NOTE)	●—	INDICATES DRAG CONNECTION
S	INDICATES STEP IN FOOTING (REFER TO TYPICAL STEP IN FOOTING DETAIL)	- - -	INDICATES A LEDGER
XX SXXX	DETAILS OR SECTION CUT (DETAIL NUMBER/SHEET NUMBER)		INDICATES WOOD OR STEEL STUD BEARING WALL LINE PER KEY ON SHEET
00 S0.0	DETAILS OR SECTION CUT IN PLAN VIEW (DETAIL NUMBER/SHEET NUMBER)	OR	INDICATES WOOD OR STEEL STUD SHEAR WALL LINE AND HOLD-DOWNS PER KEY ON SHEET
XX/SXX.XX	INDICATES LOCATION OF CONCRETE WALLS, SHEAR WALLS OR BRACED FRAME ELEVATIONS		INDICATES MASONRY/CMU WALL
	STRUCTURAL EXTENT SYMBOL SINGLE ARROW - END OF EXTENT DOUBLE ARROW - CONTINUOUS EXTENT ALONG THE ELEMENT LINE UNTIL THE ELEMENT IS INTERRUPTED		INDICATES CONCRETE/TILT-UP CONCRETE WALL
			INDICATES BEARING WALL BELOW
	INDICATES DIRECTION OF DECK SPAN		INDICATES EXISTING WALL

ABBREVIATIONS

L	Angle	EXCAV	Excavation	PLF	Pounds Per Linear Foot
AB	Anchor Bolt	FB	Factory-Built	PLWD	Plywood
ADDL	Additional	FD	Floor Drain	PREFAB	Prefabricated
ADH	Adhesive	FDN	Foundation	PSF	Pounds per Square Foot
ALT	Alternate	FIN	Finish	PSI	Pounds Per Square Inch
ARCH	Architectural	FLR	Floor	PSL	Parallel Strand Lumber
B or BOT	Bottom	FRP	Fiberglass Reinforced Plastic	P-T	Post-Tensioned
B/	Bottom Of	FRT	Fire Retardant Treated	PT	Pressure Treated
BLDG	Building	FTG	Footing	R	Radius
BLKG	Blocking	F/	Face of	RD	Roof Drain
BMU	Brick Masonry Unit	GA	Gage	REF	Refer/Reference
BP	Baseplate	GALV	Galvanized	REINF	Reinforcing
BRBF	Buckling Restrained	GEOTECH	Geotechnical	REQD	Required
	Braced Frame	GL	Glue Laminated Timber	RET	Retaining
BRG	Bearing	GWB	Gypsum Wall Board	SB	Site-Built
BTWN	Between	HDR	Header	SCBF	Special Concentric
C	Camber	HF	Hem-Fir		Braced Frame
CB	Castellated Beam	HGR	Hanger	SCHED	Schedule
C'BORE	Counterbore	HD	Hold-down	SER	Structural Engineer of
CL or ℄	Centerline	HORIZ	Horizontal		Record
CLT	Cross-Laminated Timber	HP	High Point	SFRS	Seismic Force-
CIP	Cast in Place	HSS = TS	(Hollow Structural Section)		Resisting System
CFS	Cold Formed Steel	IBC	International Building Code	SHTHG	Sheathing
CJ	Construction or	ID	Inside Diameter	SIM	Similar
	Control Joint	IE	Invert Elevation	SLBB	Short Leg Back-to-Back
CJP	Complete Joint	IF	Inside Face	SMF	Special Moment Frame
	Penetration	INT	Interior	SOG	Slab on Grade
CLR	Clear	k	Kips	SP	Southern Pine
CLG	Ceiling	KSF	Kips Per Square Foot	SPEC	Specification
CMU	Concrete Masonry Unit	LF	Lineal Foot	SQ	Square
COL	Column	LL	Live Load	SR	Studrail
CONC	Concrete	LLBB	Long Leg Back-to-Back	SF	Square Foot
CONN	Connection	LLH	Long Leg Horizontal	SST	Stainless Steel
CONST	Construction	LLV	Long Leg Vertical	STAGG	Stagger/Staggered
CONT	Continuous	LP	Low Point	STD	Standard
C'SINK	Countersink	LONGIT	Longitudinal	STIFF	Stiffener
CTRD	Centered	LSL	Laminated Strand Lumber	STL	Steel
DIA	Diameter	LVL	Laminated Veneer Lumber	STRUCT	Structural
DB	Drop Beam	MAS	Masonry	SWWJ	Solid Web Wood Joist
DBA	Deformed Bar Anchor	MAX	Maximum	SYM	Symmetrical
DBL	Double	MECH	Mechanical	T	Top
DEMO	Demolish	MEP	Mechanical, Electrical,	T/	Top Of
DEV	Development		Plumbing	T&B	Top & Bottom
DF	Douglas Fir	MEZZ	Mezzanine	TC AX LD	Top Chord Axial Load
DIAG	Diagonal	MFR	Manufacturer	TCX	Top Chord Extension
DIST	Distributed	MIN	Minimum	TDS	Tie Down System
DL	Dead Load	MISC	Miscellaneous	T&G	Tongue & Groove
DN	Down	NIC	Not In Contract	THKND	Thickened
DO	Ditto	NLT	Nail-Laminated Timber	THRD	Threaded
DP	Depth/Deep	NTS	Not To Scale	THRU	Through
DWG	Drawing	OC	On Center	TRANSV	Transverse
(E)	Existing	OCBF	Ordinary Concentric Braced	TYP	Typical
EA	Each		Frame	UNO	Unless Noted Otherwise
EF	Each Face	OD	Outside Diameter	URM	Unreinforced Masonry
EL	Elevation	OF	Outside Face		Unit
ELEC	Electrical	OPNG	Opening	VERT	Vertical
ELEV	Elevator	OPP	Opposite	W	Wide
EMBED	Embedment	OWSJ	Open Web Steel Joist	W/	With
EQ	Equal	OWWJ	Open Web Wood Joist	W/O	Without
EQUIP	Equipment	PL	Plate	WHS	Welded Headed Stud
EW	Each Way	PAF	Powder Actuated Fastener	WP	Working Point
EXP	Expansion	PC	Precast	WWF	Welded Wire Fabric
EXP JT	Expansion Joint	PERP	Perpendicular	±	Plus or Minus
EXT	Exterior	PJP	Partial Joint Penetration		

EAST SIDE FIRE STATION STRUCTURAL SHEET LIST

SHEET NUMBER	SHEET TITLE
NIU - S2.4	STRUCTURAL - UPPER ROOF FRAMING PLAN
S1.1	STRUCTURAL - GENERAL NOTES, LEGEND, AND ABBREVIATIONS
S1.2	STRUCTURAL - GENERAL NOTES CONTINUED
S1.3	STRUCTURAL - GENERAL NOTES CONTINUED
S1.4	STRUCTURAL - SPECIAL INSPECTIONS
S2.1	STRUCTURAL - FIRST FLOOR FOUNDATION PLAN
S2.2	STRUCTURAL - SECOND FLOOR FRAMING PLAN
S2.3	STRUCTURAL - ROOF FRAMING PLAN
S3.1	STRUCTURAL - ELEVATIONS
S3.2	STRUCTURAL - ELEVATIONS
S4.1	STRUCTURAL - FOUNDATION DETAILS
S4.2	FOUNDATION DETAILS
S5.1	STRUCTURAL - FRAMING DETAILS
S6.0	STRUCTURAL - FRAMING DETAILS
S7.0	STRUCTURAL - FRAMING DETAILS
Sheet Total: 15	

FOR PERMIT
The Contractor shall not use these drawings for construction until Contractor receives written approval for use in construction by the authority having jurisdiction and DCI Engineers.

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CIVIL / STRUCTURAL

74858PE
SHIRLEY CHALUPA
MARCH 23, 2011
EXPIRES: 12-31-25

PROJECT NO.: 22.22.2

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
STRUCTURAL GENERAL NOTES, LEGEND, AND ABBREVIATIONS

S1.1

5. **PILE TESTING**:
    - 5.1. 10% (2 MINIMUM) OF THE PILES SHOULD BE PROOF TESTED IN TENSION TO 200% OF THE DESIGN LOAD SHOWN ON PLAN. THE GEOTECHNICAL ENGINEER SHALL SELECT PILES TO BE PROOF TESTED. PROOF TESTING MUST BE OBSERVED AND RECORDED BY THE GEOTECHNICAL ENGINEER.
    - 5.2. ALL LOADING, TESTING, AND MEASURING EQUIPMENT SHOULD BE PROVIDED BY THE PILE CONTRACTOR AND APPROVED BY THE GEOTECHNICAL ENGINEER. THE LOADING AND MEASURING EQUIPMENT SHOULD MEET THE REQUIREMENTS SPECIFIED IN ASTM TEST METHOD D1143
    - 5.3. PROOF LOAD SHALL BE MAINTAINED FOR 10 MINUTES. DISPLACEMENT (CREEP) OF THE PILES SHALL BE RECORDED AT 0, 1, 2, 3, 4, 5, 6, 7, 8, 9, AND 10 MINUTES AND A PLOT MADE AGAINST THE LOG OF TIME. IF THE PILE DISPLACEMENT BETWEEN 1 AND 10 MINUTES EXCEEDS 0.04 INCH THE PROOF LOAD SHALL BE MAINTAINED FOR AN ADDITIONAL 50 MINUTES AND DISPLACEMENT RECORDED AT 20, 30, 40, 50 AND 60 MINUTES.
    - **5.4. ACCEPTANCE -** THE PILES WILL BE CONSIDERED ACCEPTABLE IF THE MEASURED DISPLACEMENT DOES NOT EXCEED 0.04 INCH IN THE MAXIMUM LOAD PROOF TEST FOR THE 10-MINUTE HOLD. FOR THE A 60-MINUTE HOLD, THE CREEP MOVEMENT SHALL NOT EXCEED 0.08IN DURING THE MAXIMUM HOLD.
    - 5.5. ALL LOADS SHOULD BE MAINTAINED WITHIN 5% OF THE INTENDED LOAD. ONCE THE MAXIMUM LOADING CONDITION IS REACHED, UNLOADING SHOULD BE ACCOMPLISHED IN TWO INCREMENTS: 50% AND 0% OF THE MAXIMUM LOAD.

03.20.00 CONCRETE REINFORCEMENT

1. **REFERENCE STANDARDS:** CONFORM TO:
    - 1.1. ACI 301 "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE," SECTION 3 "REINFORCEMENT AND REINFORCEMENT SUPPORTS"
    - 1.2. ACI SP-66 "ACI DETAILING MANUAL"
    - 1.3. CRSI MSP "MANUAL OF STANDARD PRACTICE"
    - 1.4. ANSI/AWS D1.4 "STRUCTURAL WELDING CODE - REINFORCING STEEL"
    - 1.5. OSSC CHAPTER 19 "CONCRETE"
    - 1.6. ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
    - 1.7. ACI 117 "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS"
2. **SUBMITTALS:**
    - 2.1. CONFORM TO ACI 301 SECTION 3.1.2. SUBMIT PLACING DRAWINGS SHOWING FABRICATION DIMENSIONS AND PLACEMENT LOCATIONS OF REINFORCEMENT AND REINFORCEMENT SUPPORTS.
    - 2.2.
3. **MATERIALS:**

REINFORCING BARS [NOTE 3.1]	ASTM A615, GRADE 60, DEFORMED BARS
SMOOTH WELDED WIRE FABRIC	ASTM A1064
DEFORMED WELDED WIRE FABRIC	ASTM A1064
BAR SUPPORTS	CRSI MSP CHAPTER 3
TIE WIRE	16 GAGE OR HEAVIER, BLACK ANNEALED

    - 3.1. ASSUME GRADE 60 REINFORCEMENT UNLESS NOTED OTHERWISE ON PLAN. REFERENCE PLANS FOR HIGH STRENGTH REINFORCING LOCATIONS, INDICATED BY (GR 80) OR (GR 100).
4. **EARTHQUAKE REQUIREMENTS:** LONGITUDINAL BARS IN DUCTILE FRAMES AND SHEAR WALLS AND COUPLING BEAMS OF SHEAR WALLS. SHALL CONFORM TO ASTM A706, GRADE 60 OR SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
    - 4.1. WELDING: WELDING IS NOT PERMITTED EXCEPT AS SPECIFIED IN THE DRAWINGS. WELD IN ACCORDANCE WITH AWS D1.4.
    - 4.2. MILL TESTS: SUBMIT MILL CERTIFICATES INDICATING PHYSICAL AND CHEMICAL PROPERTIES
    - 4.3. YIELD STRENGTH: ACTUAL YIELD STRENGTH, BASED ON MILL TESTS, DOES NOT EXCEED THE SPECIFIED YIELD STRENGTH BY MORE THAN 18,000 PSI. (RETESTS SHALL NOT EXCEED THIS VALUE BY MORE THAN AN ADDITIONAL 3,000 PSI.)
    - 4.4. ULTIMATE STRENGTH: THE RATIO OF THE ACTUAL TENSILE STRENGTH TO THE ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25.
    - 4.5. STRAIN: MINIMUM ELONGATION IN 8″ SHALL BE AT LEAST:
        - 4.5.1. 14% FOR BAR SIZES #3 THROUGH #6
        - 4.5.2. 12% FOR BAR SIZES #7 THROUGH #11
        - 4.5.3. 10% FOR BAR SIZES #14 AND #18
5. **FABRICATION:** CONFORM TO ACI 301 SECTION 3.2.2 AND ACI SP-66.
6. **WELDING:** BARS SHALL NOT BE WELDED UNLESS AUTHORIZED. WHEN AUTHORIZED, CONFORM TO ACI 301 SECTION 3.2.2.2 AND AWS D1.4, AND PROVIDE ASTM A706, GRADE 60 REINFORCEMENT.
7. **PLACING:** CONFORM TO ACI 301 SECTION 3.3.2. PLACING TOLERANCES SHALL CONFORM TO ACI 117.
8. **CONCRETE COVER:** CONFORM TO THE FOLLOWING COVER REQUIREMENTS UNLESS NOTED OTHERWISE ON PLAN:

CONCRETE CAST AGAINST EARTH	3″
CONCRETE EXPOSED TO EARTH OR WEATHER	2″
TIES IN COLUMNS AND BEAMS	1-1/2″
BARS IN SLABS	3/4″
BARS IN WALLS	3/4″

9. **SPLICES:** CONFORM TO ACI 301 SECTION 3.3.2.7. REFER TO TYPICAL LAP SPLICE AND DEVELOPMENT LENGTH SCHEDULE IN THESE DRAWINGS FOR TYPICAL REINFORCEMENT SPLICES. REFER TO "COLUMN VERTICAL REINFORCING SPLICE SCHEDULE" AND "SHEAR WALL REINFORCING SPLICE SCHEDULE" IN THESE DRAWINGS FOR THOSE SPECIFIC ELEMENTS. SPLICES INDICATED ON INDIVIDUAL SHEETS SHALL CONTROL OVER THE SCHEDULE. MECHANICAL CONNECTIONS MAY BE USED WHEN APPROVED BY THE SER. FOR REINFORCING WITHIN THE LATERAL SYSTEM SHEAR WALLS AND REINFORCING CONNECTING THE DIAPHRAGM SLAB TO THE LATERAL SYSTEM, MECHANICAL SPLICE STRENGTH IS INCREASED TO DEVELOP 125% OF THE SPECIFIED TENSILE STRENGTH OF THE SPLICES BAR.
10. **FIELD BENDING:** CONFORM TO ACI 301 SECTION 3.3.2.8. BAR SIZES #3 THROUGH #5 MAY BE FIELD BENT COLD THE FIRST TIME. SUBSEQUENT BENDS AND OTHER BAR SIZES REQUIRE PREHEATING. DO NOT TWIST BARS. BARS SHALL NOT BE BENT PAST 45°.

03.30.00 CAST-IN-PLACE CONCRETE

1. **REFERENCE STANDARDS:** CONFORM TO:
    - 1.1. ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE"
    - 1.2. OSSC CHAPTER 19 "CONCRETE"
    - 1.3. ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
    - 1.4. ACI 117 "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS"
2. **FIELD REFERENCE:** THE CONTRACTOR SHALL KEEP A COPY OF ACI FIELD REFERENCE MANUAL SP-15.
3. **CONCRETE MIXTURES:** CONFORM TO ACI 301 SECTION 4 AND OSSC SECTION 1904.1.
4. **MATERIALS:** CONFORM TO ACI 301 SECTION 4.2.1 FOR REQUIREMENTS FOR CEMENTITIOUS MATERIALS, AGGREGATES, MIXING WATER, AND ADMIXTURES.
5. **SUBMITTALS:**
    - 5.1. PROVIDE ALL SUBMITTALS REQUIRED BY ACI 301 SECTION 4.1.2. SUBMIT MIX DESIGNS FOR EACH MIX IN THE TABLE BELOW. SUBSTANTIATING STRENGTH RESULTS FROM PAST TESTS SHALL NOT BE OLDER THAN 24 MONTHS PER ACI 318 SECTION 26.4.3.1(B).
6. **MIX DESIGN REQUIREMENTS [NOTES 7.1 TO 7.11, TYPICAL UNO]**

FOOTINGS	
STRENGTH, f'_c	4,000 PSI
TEST AGE	56 DAYS
NOMINAL MAXIMUM AGGREGATE	1″
INTERIOR SLABS ON GRADE	
STRENGTH, f'_c	3,000 PSI
TEST AGE	28 DAYS
NOMINAL MAXIMUM AGGREGATE	1″
SHEAR WALLS	
STRENGTH, f'_c	6,000 PSI
TEST AGE	56 DAYS
NOMINAL MAXIMUM AGGREGATE	1″

7. **MIX DESIGN REQUIREMENT NOTES:**
    - 7.1. W/C RATIO: WATER-CEMENTITIOUS MATERIAL RATIOS SHALL BE BASED ON THE TOTAL WEIGHT OF CEMENTITIOUS MATERIALS. MAXIMUM RATIOS ARE CONTROLLED BY STRENGTH NOTED IN THE TABLE OF MIX DESIGN REQUIREMENTS AND DURABILITY REQUIREMENTS GIVEN IN ACI 318 SECTION 19.3. W/C RATIOS MAY BE EXCEEDED WITH APPROVAL OF SER AS LONG AS POTENTIAL SHRINKAGE IMPACTS ARE ACCOUNTED FOR.
    - 7.2. CEMENTITIOUS MATERIALS:
        - 7.2.1. DCI ENCOURAGES THE REDUCTION OF CEMENT CONTENT AND/OR THE USE OF ALTERNATE CEMENTITIOUS MATERIALS. WHERE REQUIREMENTS OF THIS SECTION PROHIBIT INCLUSION OF ANY OF THESE MIXES, CONTACT DCI FOR FURTHER COORDINATION.
        - 7.2.2. CEMENTITIOUS MATERIALS SHALL CONFORM TO THE RELEVANT ASTM STANDARDS LISTED IN ACI 318 SECTION 26.4.1.1.1(A).
        - 7.2.3. FOR CONCRETE USED IN ELEVATED FLOORS, MINIMUM CEMENTITIOUS MATERIALS CONTENT SHALL CONFORM TO ACI 301 TABLE 4.2.1.1(B). ACCEPTANCE OF LOWER CEMENT CONTENT IS CONTINGENT ON PROVIDING SUPPORTING DATA TO THE SER FOR REVIEW AND ACCEPTANCE.
        - 7.2.4. CEMENT REPLACEMENT: THE USE OF FLY ASH, OTHER POZZOLANS, SILICA FUME, OR SLAG SHALL CONFORM TO ACI 318 SECTIONS 19.3.2 AND 26.4.2.2. SUPPLEMENTAL CEMENTITIOUS MATERIAL (SCM) QUANTITIES SHALL MEET REQUIREMENTS OUTLINED IN THE TABLE BELOW. APPROACHING MAXIMUM CEMENT REPLACEMENT LIMITS MAY AFFECT CONCRETE SETTING TIME AND STRENGTH GAIN. CONTRACTOR AND SUPPLIER SHALL COORDINATE ON MIX DESIGNS WITH REGARD TO SCHEDULE, WORKABILITY, SHRINKAGE, AND FINISHABILITY REQUIREMENTS. WHERE SCM QUANTITIES DO NOT MEET THE FOLLOWING REQUIREMENTS, SUBMIT FOR SER APPROVAL. CONCERNS BY THE CONSTRUCTION TEAM WITH THE MIX DESIGN PROVIDED HEREIN SHALL BE BROUGHT TO THE SER'S ATTENTION IN THE MIX DESIGN SUBMITTAL PRIOR TO POURING CONCRETE.

REPLACEMENT	FOUNDATION	SLABS	WALLS/COLUMNS
MAXIMUM	70%	25%	50%
PREFERRED MINIMUM	25%	15%	25%

    - 7.3. GENERAL CONTRACTOR SHALL COORDINATE MEANS AND METHODS NECESSARY TO SUPPORT EXTENDED TEST AGES INCLUDING, BUT NOT LIMITED TO, DELAYED STRENGTH GAIN, SHORING SEQUENCING, MODULUS OF ELASTICITY REQUIREMENTS, DEFLECTION, AND APPEARANCE.
    - 7.4. AIR CONTENT: CONFORM TO ACI 318 SECTION 19.3.3.1. MINIMUM STANDARDS FOR EXPOSURE CLASS ARE NOTED IN THE TABLE. IF FREEZING AND THAWING CLASS IS NOT NOTED, AIR CONTENT GIVEN IS THAT REQUIRED BY THE SER. TOLERANCE IS ±1-1/2%. AIR CONTENT SHALL BE MEASURED AT POINT OF PLACEMENT.
    - 7.5. AGGREGATES SHALL CONFORM TO ASTM C33. LIGHTWEIGHT AGGREGATES SHALL CONFORM TO ASTM C330. INSULATING LIGHTWEIGHT AGGREGATES SHALL CONFORM TO ASTM C332.
    - 7.6. SLUMP: CONFORM TO ACI 301 SECTION 4.2.2.1. SLUMP SHALL BE DETERMINED AT POINT OF PLACEMENT.
    - 7.7. CHLORIDE CONTENT: CONFORM TO ACI 318 TABLE 19.3.2.1.
    - 7.8. NON-CHLORIDE ACCELERATOR: NON-CHLORIDE ACCELERATING ADMIXTURE MAY BE USED IN CONCRETE PLACED AT AMBIENT TEMPERATURES BELOW 50°F AT THE CONTRACTOR'S OPTION.
    - 7.9. ACI 318 SECTION 19.3.1.1 EXPOSURE CLASSES SHALL BE ASSUMED TO BE F0 UNLESS DIFFERENT EXPOSURE CLASSES ARE LISTED IN THE TABLE OF MIX DESIGN REQUIREMENTS THAT MODIFY THESE BASE REQUIREMENTS.
    - 7.10. RECYCLED CARBON DIOXIDE (CO_2) IS PERMISSIBLE TO BE INJECTED INTO THE MIX AS AN INGREDIENT DURING MIXING, SUCH THAT CO_2 IS CHEMICALLY MINERALIZED INTO CONCRETE. CO_2 INJECTED INTO THE MIX MUST BE POST-INDUSTRIAL CO_2 SOURCED FROM AN EMITTER.
    - 7.11. MODULUS OF ELASTICITY SHALL BE A MINIMUM OF 57,000 * $\sqrt{f'_c}$ FOR ALL MIX DESIGNS.
    - 7.12. CORROSION INHIBITING ADMIXTURE: BEAMS AND PILE CAPS WITH A MARINE EXPOSURE SHALL CONTAIN 4-1/2 GALLONS OF CALCIUM NITRITE PER CUBIC YARD. TOPPING SLABS EXPOSED TO DEICING SALTS SHALL CONTAIN 2-1/2 GALLONS OF CALCIUM NITRITE PER CUBIC YARD.
8. **STRENGTH TESTING AND ACCEPTANCE:**
    - 8.1. TESTING: OBTAIN SAMPLES AND CONDUCT TESTS IN ACCORDANCE WITH ACI 301 SECTION 1.7.3.3. ADDITIONAL SAMPLES MAY BE REQUIRED TO OBTAIN CONCRETE STRENGTHS AT ALTERNATE INTERVALS THAN SHOWN BELOW AND SHOULD BE STANDARD CURED PER ACI SECTION 26.5.3.2.
        - 8.1.1. CURE FOUR CYLINDERS FOR 28-DAY TEST AGE. TEST ONE CYLINDER AT 7 DAYS, TEST TWO CYLINDERS AT 28 DAYS, AND HOLD ONE CYLINDER IN RESERVE FOR USE AS THE ENGINEER DIRECTS. AFTER 56 DAYS, UNLESS NOTIFIED BY THE ENGINEER TO THE CONTRARY, THE RESERVE CYLINDER MAY BE DISCARDED WITHOUT BEING TESTED FOR SPECIMENS MEETING 28-DAY STRENGTH REQUIREMENTS.
        - 8.1.2. CURE FIVE CYLINDERS FOR 56-DAY TEST AGE. TEST ONE AT 7 DAYS, ONE AT 28 DAYS, TWO AT 56 DAYS AND HOLD ONE CYLINDER IN RESERVE FOR USE AS THE ENGINEER DIRECTS. AFTER 72 DAYS, UNLESS NOTIFIED BY THE ENGINEER TO THE CONTRARY, THE RESERVE CYLINDER MAY BE DISCARDED WITHOUT BEING TESTED FOR SPECIMENS MEETING 56-DAY STRENGTH REQUIREMENTS.
        - 8.1.3. CURE SIX CYLINDERS FOR 90-DAY TEST AGE. TEST ONE AT 7 DAYS, ONE AT 28 DAYS, ONE AT 56 DAYS, TWO AT 90 DAYS, AND HOLD ONE CYLINDER IN RESERVE FOR USE AS THE ENGINEER DIRECTS. AFTER 106 DAYS, UNLESS NOTIFIED BY THE ENGINEER TO THE CONTRARY, THE RESERVE CYLINDER MAY BE DISCARDED WITHOUT BEING TESTED FOR SPECIMENS MEETING 90-DAY STRENGTH REQUIREMENTS.
        - 8.1.4. THE NUMBER OF CYLINDERS INDICATED ABOVE REFERENCE 6″x12″ CYLINDERS. IF 4″x8″ CYLINDERS ARE TO BE USED, ADDITIONAL CYLINDERS MUST BE CURED FOR TESTING OF THREE CYLINDERS AT TEST AGE PER THE TABLE OF MIX DESIGN REQUIREMENTS IN THESE GENERAL NOTES.
    - 8.2. ACCEPTANCE:
        - 8.2.1. STRENGTH IS SATISFACTORY WHEN THE AVERAGES OF ALL SETS OF THREE CONSECUTIVE TESTS EQUAL OR EXCEED THE SPECIFIED STRENGTH AND NO INDIVIDUAL TEST FALLS BELOW THE SPECIFIED STRENGTH BY MORE THAN 500 PSI.
        - 8.2.2. A "TEST" FOR ACCEPTANCE IS THE AVERAGE STRENGTH OF TWO 6″x12″ CYLINDERS OR THREE 4″x8″ CYLINDERS TESTED AT THE SPECIFIED TEST AGE.
9. **MEASURING, MIXING, AND DELIVERY:** CONFORM TO ACI 301 SECTION 4.3.
10. **HANDLING, PLACING, CONSTRUCTING AND CURING:** CONFORM TO ACI 301 SECTION 5. IN ADDITION, HOT WEATHER CONCRETING SHALL CONFORM TO ACI 305R AND COLD WEATHER CONCRETING SHALL CONFORM TO ACI 306R.
11. **POST-INSTALLED ANCHORS TO CONCRETE:** ANCHOR LOCATION, TYPE, DIAMETER, AND EMBEDMENT SHALL BE AS INDICATED ON DRAWINGS. REFERENCE THE POST-INSTALLED ANCHORS SECTION OF THESE GENERAL NOTES FOR APPLICABLE POST-INSTALLED ANCHOR ADHESIVES. ANCHORS SHALL BE INSTALLED AND INSPECTED IN STRICT ACCORDANCE WITH THE APPLICABLE ICC-EVALUATION SERVICE REPORT (ESR). SPECIAL INSPECTION SHALL BE PER THE TESTS AND INSPECTIONS SECTION OF THESE GENERAL NOTES.

05.05.19 POST-INSTALLED ANCHORS (INTO CONCRETE)

1. **REFERENCE STANDARDS:** CONFORM TO:
    - 1.1. OSSC CHAPTER 19 "CONCRETE"
    - 1.2. ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE"
    - 1.3. OSSC CHAPTER 21 "MASONRY"
    - 1.4. TMS 402 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES"
2. **POST-INSTALLED ANCHORS:** INSTALL ONLY WHERE SPECIFICALLY SHOWN IN THE DETAILS OR ALLOWED BY THE SER. ALL POST-INSTALLED ANCHOR TYPES AND LOCATIONS SHALL BE APPROVED BY THE SER AND SHALL HAVE A CURRENT ICC-ESR THAT PROVIDES RELEVANT DESIGN VALUES NECESSARY TO VALIDATE THE AVAILABLE STRENGTH EXCEEDS THE REQUIRED STRENGTH. SUBMIT CURRENT MANUFACTURER'S DATA AND ICC-ESR TO THE SER FOR APPROVAL REGARDLESS OF WHETHER OR NOT IT IS A PRE-APPROVED ANCHOR. ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH ICC-ESR AND THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT DEPTH AS INDICATED ON THE DRAWINGS. THE CONTRACTOR SHALL ARRANGE FOR A MANUFACTURER'S FIELD REPRESENTATIVE TO PROVIDE INSTALLATION TRAINING FOR ALL PRODUCTS TO BE USED PRIOR TO THE COMMENCEMENT OF WORK. ONLY TRAINED INSTALLERS SHALL PERFORM POST-INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND BE MADE AVAILABLE TO THE SER AS REQUESTED. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI OR AN APPROVED EQUIVALENT. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION. NO REINFORCING BARS SHALL BE DAMAGED DURING INSTALLATION OF POST-INSTALLED ANCHORS. SPECIAL INSPECTION SHALL BE PER THE TESTS AND INSPECTIONS SECTION OF THESE GENERAL NOTES. ANCHOR TYPE, DIAMETER, AND EMBEDMENT SHALL BE AS INDICATED ON DRAWINGS.
    - 2.1. ADHESIVE ANCHORS: THE FOLLOWING ADHESIVE-TYPE ANCHORING SYSTEMS HAVE BEEN USED IN THE DESIGN AND SHALL BE USED FOR ANCHORAGE TO CONCRETE OR MASONRY AS APPLICABLE AND IN ACCORDANCE WITH THE CORRESPONDING CURRENT ICC ESR REPORT. REFERENCE THE CORRESPONDING ICC-ESR FOR REQUIRED MINIMUM AGE OF CONCRETE, CONCRETE TEMPERATURE RANGE, MOISTURE CONDITION, LIGHT WEIGHT CONCRETE, AND HOLE DRILLING AND PREPARATION REQUIREMENTS. DRILLED-IN ANCHOR EMBEDMENT LENGTHS SHALL BE AS SHOWN ON DRAWINGS OR NOT LESS THAN SEVEN TIMES THE ANCHOR NOMINAL DIAMETER (7D). ADHESIVE ANCHORS ARE TO BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS UNLESS OTHERWISE SPECIFIED IN THE ICC-ESR.

        DEWALT "PURE 220+" - ICC-ESR 5144 FOR ANCHORAGE TO CONCRETE
    - 2.2. SCREW ANCHORS: THE FOLLOWING SCREW TYPE ANCHOR IS PRE-APPROVED FOR ANCHORAGE TO CONCRETE IN ACCORDANCE WITH ASTM C1892 TESTING PRINCIPLES:

        SIMPSON "TITEN HD" - ICC-ESR 2713 FOR CARBON STEEL TO CONCRETE, ICC-ESR 1056 FOR MASONRY ONLY, IAPMO 493 FOR STAINLESS STEEL TO CONCRETE

05.12.00 STRUCTURAL STEEL

1. **REFERENCE STANDARDS:** CONFORM TO:
    - 1.1. OSSC CHAPTER 22 "STEEL"
    - 1.2. ANSI/AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"
    - 1.3. AISC "MANUAL OF STEEL CONSTRUCTION"
    - 1.4. ANSI/AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"
    - 1.5. AWS D1.1 "STRUCTURAL WELDING CODE - STEEL"
    - 1.6. RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS"
2. **SUBMITTALS:** SUBMIT THE FOLLOWING DOCUMENTS TO THE SER FOR REVIEW:
    - 2.1. SHOP DRAWINGS COMPLYING WITH AISC 360 SECTIONS M1 AND N3 AND AISC 303 SECTION 4.
    - 2.2. ERECTION DRAWINGS COMPLYING AISC 360 SECTIONS M1 AND N3 AND AISC 303 SECTION 4.
    - 2.3. WELD PROCEDURE SPECIFICATIONS (WPS'S) FOR SHOP AND FIELD WELDING.
    - 2.4. MANUFACTURER'S CERTIFICATES OF CONFORMANCE FOR ELECTRODES, FLUXES, AND GASES (WELDING CONSUMABLES).
3. **COPIES:** MAKE COPIES OF THE FOLLOWING DOCUMENTS AVAILABLE UPON REQUEST TO THE SER OR OWNER'S INSPECTION AGENCY IN ELECTRONIC OR PRINTED FORM PRIOR TO FABRICATION PER AISC 360 SECTION N3.2 REQUIREMENTS:
    - 3.1. FABRICATOR'S WRITTEN QUALITY CONTROL MANUAL THAT INCLUDES, AS A MINIMUM:
        - 3.1.1. MATERIAL CONTROL PROCEDURES
        - 3.1.2. INSPECTION PROCEDURES
        - 3.1.3. NON-CONFORMANCE PROCEDURES
    - 3.2. STEEL AND ANCHOR ROD SUPPLIERS' MATERIAL TEST REPORTS (MTRS) INDICATING COMPLIANCE WITH SPECIFICATIONS.
    - 3.3. FASTENER MANUFACTURER'S CERTIFICATION DOCUMENTING CONFORMANCE WITH THE SPECIFICATION.
    - 3.4. FILLER METAL MANUFACTURER'S PRODUCT DATA FOR SMAW, FCAW, AND GMAW INDICATING:
        - 3.4.1. PRODUCT SPECIFICATION COMPLIANCE
        - 3.4.2. RECOMMENDED WELDING PARAMETERS
        - 3.4.3. RECOMMENDED STORAGE AND EXPOSURE REQUIREMENTS INCLUDING BAKING
        - 3.4.4. LIMITATIONS OF USE
    - 3.5. WELDED HEADED (SHEAR) STUD ANCHORS MANUFACTURER'S CERTIFICATION INDICATING THEY MEET SPECIFICATIONS.
    - 3.6. PROCEDURE QUALIFICATION RECORDS (PQRS) FOR WPS'S THAT ARE NOT PREQUALIFIED IN ACCORDANCE WITH AWS.
    - 3.7. WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (WPQRS) AND CONTINUITY RECORDS CONFORMING TO AWS STANDARDS
4. **MATERIALS:** STRUCTURAL STEEL MATERIALS SHALL CONFORM TO MATERIALS AND REQUIREMENTS LISTED IN AISC 360 SECTION A3 INCLUDING, BUT NOT LIMITED TO:

SHAPE	TYPE
SQUARE/RECT HSS	ASTM A500, GRADE C, F_y = 50 KSI

5. **FABRICATION:**
    - 5.1. CONFORM TO AISC 360 SECTION M2 AND AISC 303 SECTION 6.
    - 5.2. QUALITY CONTROL (QC) SHALL CONFORM TO:
        - 5.2.1. AISC 360 CHAPTER N
        - 5.2.2. AISC 303 SECTION 8
        - 5.2.3. FABRICATOR AND ERECTOR SHALL ESTABLISH AND MAINTAIN WRITTEN QC PROCEDURES PER AISC 360 SECTION N3.
        - 5.2.4. FABRICATOR SHALL PERFORM SELF-INSPECTIONS PER AISC 360 SECTION N5 TO ENSURE THAT THEIR WORK IS PERFORMED IN ACCORDANCE WITH CODE OF STANDARD PRACTICE, THE AISC SPECIFICATION, CONTRACT DOCUMENTS, AND THE APPLICABLE BUILDING CODE.
        - 5.2.5. QC INSPECTIONS MAY BE COORDINATED WITH QUALITY ASSURANCE (QA) INSPECTIONS PER AISC 360 SECTION N5.3 WHERE FABRICATORS QA PROCEDURES PROVIDE THE NECESSARY BASIS FOR MATERIAL CONTROL, INSPECTION, AND CONTROL OF THE WORKMANSHIP EXPECTED BY THE SPECIAL INSPECTOR.
6. **WELDING:**
    - 6.1. WELDING SHALL CONFORM TO AWS D1.1 WITH PREQUALIFIED WELDING PROCESSES EXCEPT AS MODIFIED BY AISC 360 SECTION J2 WELDERS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS D1.1 REQUIREMENTS.
    - 6.2. USE 70 KSI STRENGTH, LOW-HYDROGEN TYPE ELECTRODES (E7018) OR E71T AS APPROPRIATE FOR THE PROCESS SELECTED.
    - 6.3. WELDING OF HIGH-STRENGTH ANCHOR RODS IS PROHIBITED UNLESS APPROVED BY ENGINEER.
    - 6.4. WELDING OF HEADED STUD ANCHORS SHALL BE IN ACCORDANCE WITH AWS D1.1 CHAPTER 7.
7. **PROTECTIVE COATING REQUIREMENTS:**
    - 7.1. SHOP PAINTING: CONFORM TO AISC 360 SECTION M3 AND AISC 303 SECTION 6.5 UNLESS OTHERWISE SPECIFIED BY THE PROJECT SPECIFICATIONS.
    - 7.2. STEEL NEED NOT BE PRIMED OR PAINTED UNLESS NOTED OTHERWISE ON PLAN OR IN THE PROJECT SPECIFICATIONS. CONFORM TO AISC 360 SECTION M3 AND AISC 303 SECTION 6.5 UNLESS A MULTI-COAT SYSTEM IS REQUIRED PER THE PROJECT SPECIFICATIONS.
8. **ARCHITECTURALLY EXPOSED STRUCTURAL STEEL:** STEEL IDENTIFIED BY THE ARCHITECT ON THE ARCHITECTURAL DRAWINGS AS ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) SHALL CONFORM TO AISC 303 SECTION 10.

06.11.00 WOOD FRAMING

1. **REFERENCE STANDARDS:** CONFORM TO:
    - 1.1. OSSC CHAPTER 23 "WOOD"
    - 1.2. ANSI/AWC NDS "NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION - WITH NDS SUPPLEMENT"
    - 1.3. ANSI/AWC SDPWS "SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC"
    - 1.4. APA PDS "PANEL DESIGN SPECIFICATION"
    - 1.5. APA REPORT TT-045B "MINIMUM NAIL PENETRATION FOR WOOD STRUCTURAL PANEL CONNECTIONS SUBJECT TO LATERAL LOADS"
    - 1.6. APA REPORT TT-061C "1-5/16″ THICK I-JOIST FLANGES AND DIAPHRAGM NAIL PENETRATION"
2. **SUBMITTALS:**
    - 2.1. SUBMIT SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE MEMBER SIZE, SPACING, CAMBER, MATERIAL TYPE, GRADE, SHOP AND FIELD ASSEMBLY DETAILS AND CONNECTIONS, AND TYPES AND LOCATION OF BOLTS AND OTHER FASTENERS. SUPPLY SHOP DRAWINGS FOR THE FOLLOWING:
        - 2.1.1. LSL MEMBERS
        - 2.1.2. TAPERED AND PARALLEL WOOD I JOISTS (SOLID WEB-WOOD JOISTS)
    - 2.2. SOFTWOOD LUMBER, SOFTWOOD PLYWOOD, ORIENTED STRAND BOARD, GLUED LAMINATED TIMBER, LAMINATED VENEER LUMBER, LAMINATED STRAND LUMBER, AND WOOD I-JOISTS. EPDS MUST CONFORM TO ISO 14025 AND EN 15804 OR ISO 21930 AND HAVE AT LEAST A CRADLE-TO-GATE SCOPE.
3. **IDENTIFICATION:** ALL SAWN LUMBER AND PRE-MANUFACTURED WOOD PRODUCTS SHALL BE IDENTIFIED BY THE GRADE MARK OR A CERTIFICATE OF INSPECTION ISSUED BY THE CERTIFYING AGENCY.
4. **MATERIALS:**
    - 4.1. SAWN LUMBER: CONFORM TO GRADING RULES OF WWPA, WCLIB OR NLGA AND REQUIREMENTS BELOW. FINGER JOINTED STUDS ACCEPTABLE AT INTERIOR WALLS ONLY.

MEMBER USE	SIZE	SPECIES	GRADE
WALL STUD/TOP AND BOTTOM PLATES	2x4, 3x4, 2x6, 3x6	DOUG FIR LARCH	NO. 2
SILL PLATES (AT CONCRETE)	2x4, 3x4, 2x6, 3x6	PT DOUG FIR LARCH	NO. 2
POSTS	4x4, 4x6, 4x8	DOUG FIR LARCH	NO. 2
FLOORS OR ROOF JOISTS	2x6 THROUGH 2x16	DOUG FIR LARCH	NO. 2

    - 4.2. WOOD STRUCTURAL SHEATHING (PLYWOOD): WOOD APA-RATED STRUCTURAL SHEATHING INCLUDES: ALL VENEER PLYWOOD, ORIENTED STRAND BOARD, WAFERBOARD, PARTICLEBOARD, T1-11 SIDING, AND COMPOSITES OF VENEER AND WOOD-BASED MATERIAL WITH T&G JOINT. ARCHITECT MAY DISALLOW OSB. CONFIRM WITH ARCHITECT. CONFORM TO "STRUCTURAL PLYWOOD" BASED ON PRODUCT STANDARD PS 1-19 BY THE U.S. DEPT. OF COMMERCE, AND "PERFORMANCE STANDARD FOR WOOD STRUCTURAL PANELS" BASED ON PRODUCT STANDARD PS 2-18 BY THE U.S. DEPT. OF COMMERCE AND "PANEL DESIGN SPECIFICATION" BASED ON APA D510 BY THE ENGINEERED WOOD ASSOCIATION. UNLESS NOTED OTHERWISE, SHEATHING SHALL COMPLY WITH THE FOLLOWING TABLE:
    - 4.3. WOOD STRUCTURAL SHEATHING (PLYWOOD) - MINIMUM THICKNESS AND APA RATING REQUIREMENTS:

LOCATION	THICKNESS	SPAN RATING	PLYWOOD GRADE
ROOF	15/32″	32/16	C-D

        - 4.3.1. ALL EXPOSURES TO BE CATEGORY 1.
        - 4.3.2. UNLESS NOTED OTHERWISE ON PLAN, INSTALL ROOF AND FLOOR PANELS WITH LONG DIMENSION ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. END JOINTS SHALL OCCUR OVER SUPPORTS.
    - 4.4. TIMBER CONNECTORS: SHALL BE "STRONG TIE" BY SIMPSON COMPANY AS SPECIFIED IN THEIR LATEST CATALOG. ALTERNATE CONNECTORS BY OTHER MANUFACTURERS MAY BE SUBSTITUTED PROVIDED THEY HAVE CURRENT ICC APPROVAL FOR EQUIVALENT OR GREATER LOAD CAPACITIES AND ARE REVIEWED AND APPROVED BY THE SER PRIOR TO ORDERING. CONNECTORS SHALL BE INSTALLED PER THE MANUFACTURER'S INSTRUCTIONS. WHERE CONNECTOR STRAPS CONNECT TWO MEMBERS, PLACE HALF OF THE NAILS OR BOLTS IN EACH MEMBER. WHERE CONNECTORS ARE IN EXPOSED EXTERIOR APPLICATIONS IN CONTACT WITH PRESERVATIVE TREATED WOOD (PT) OTHER THAN CCA, CONNECTORS SHALL BE EITHER BATCH HOT-DIPPED GALVANIZED (HDG), MECHANICALLY GALVANIZED (ASTM B695, CLASS 55 MINIMUM) STAINLESS STEEL, OR PROVIDED WITH 1.85 OZ/SF OF ZINC GALVANIZING EQUAL TO OR BETTER THAN SIMPSON ZMAX FINISH.
        - 4.4.1. WHERE STRAPS ARE USED AS HOLD-DOWNS, NAIL STRAPS TO WOOD FRAMING JUST PRIOR TO DRYWALL APPLICATION, AS LATE AS POSSIBLE IN THE FRAMING PROCESS TO ALLOW THE WOOD TO SHRINK AND THE BUILDING TO SETTLE. PREMATURE NAILING OF THE STRAP MAY LEAD TO STRAP BUCKLING AND POTENTIAL FINISH DAMAGE.
    - 4.5. FASTENERS (NAILS, BOLTS, SCREWS, ETC.) ATTACHING TIMBER CONNECTORS (JOIST HANGERS, POST CAPS AND BASES, ETC.) TO PT WOOD SHALL HAVE SIMILAR CORROSION RESISTANCE PROPERTIES (MATCHING PROTECTIVE TREATMENTS) AS THE PROTECTED CONNECTOR. FASTENERS (NAILS, BOLTS, SCREWS, ETC.) ATTACHING SAWN TIMBER MEMBERS OR SHEATHING (SHEAR WALLS) TO PT WOOD SHALL BE CORROSION RESISTANT; NAILS AND LAG BOLTS SHALL BE EITHER HDG (ASTM A153) OR STAINLESS STEEL. VERIFY THE SUITABILITY OF THE FASTENER PROTECTION/COATING WITH THE WOOD TREATMENT CHEMICAL MANUFACTURER/SUPPLIER.
        - 4.5.1. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD.
    - 4.6. LAG SCREWS/BOLTS: CONFORM TO ASTM A307 AND NDS CHAPTER 12.
        - 4.6.1. LAG SCREW INSTALLATION SHALL CONFORM TO NDS SECTION 12.1.4 INCLUDING REQUIRED DRILLED LEAD AND/OR CLEARANCE HOLES.
    - 4.7. NAILS AND STAPLES: CONFORM TO ASTM F1667 AND OSSC SECTIONS 2303.6 AND 2304.10.
    - 4.8. ENGINEERED WOOD PRODUCTS (TRUSJOIST) - REQUIREMENTS:

TYPE	USE	WIDTH(S)	E (10^6 PSI)	F_b (PSI)	F_v (PSI)	F_c // (PSI)
LSL RIMBOARD	RIMBOARD OR STAIR STRINGER	1-1/2″	1.3	1,700	425	1,835

5. **NAILING REQUIREMENTS:** CONFORM TO OSSC SECTION 2304.10. UNLESS NOTED OTHERWISE ON PLAN, NAIL PER OSSC TABLE 2304.10.2. NAILING FOR ROOFS/FLOORS AND DIAPHRAGMS/SHEAR WALLS SHALL BE PER DRAWINGS. NAILS SHALL BE DRIVEN FLUSH AND SHALL NOT FRACTURE THE SURFACE OF SHEATHING. ALTERNATE NAILS MAY BE USED BUT ARE SUBJECT TO REVIEW AND APPROVAL BY THE SER. SUBSTITUTION OF STAPLES FOR THE NAILING OF RATED SHEATHING IS SUBJECT TO REVIEW BY THE SER PRIOR TO CONSTRUCTION.
6. **STANDARD LIGHT-FRAME CONSTRUCTION:** UNLESS NOTED OTHERWISE ON PLAN, CONSTRUCTION SHALL CONFORM TO OSSC SECTION 2308.
7. **MOISTURE CONTENT:** THE CONTRACTOR SHALL MAKE PROVISIONS DURING HANDLING AND CONSTRUCTION TO PREVENT THE STRUCTURAL WOOD MEMBERS FROM EXCEEDING THE APPROPRIATE MOISTURE CONTENT LIMITS. THE MOISTURE CONTENT FOR SOLID SAWN WOOD MATERIAL USED FOR THIS PROJECT SHALL NOT EXCEED 19%. THE MOISTURE CONTENT FOR ENGINEERED WOOD PRODUCTS, LAMINATED LUMBER AND SHEATHING SHALL NOT EXCEED THE LIMITS REQUIRED BY THE MANUFACTURER OR 12%, WHICHEVER IS LESS. THE MOISTURE CONTENT LIMITS MAY BE MORE STRINGENT FOR PARTICULAR PRODUCT REQUIREMENTS (E.G., FINISHES, CLADDING, INSULATION SYSTEMS, ETC.). THE CONTRACTOR SHALL REFER TO THE ARCHITECT'S DRAWINGS, PROJECT SPECIFICATIONS, OR INSTALLER/PRODUCT REQUIREMENTS FOR ADDITIONAL REQUIREMENTS.
8. **STORAGE AND HANDLING:**
    - 8.1. ALL STORAGE AND HANDLING IS TO BE A MEANS AND METHODS PROVIDED BY THE CONTRACTOR. THE CONTRACTOR IS TO DETERMINE THE BEST PRACTICES IN ORDER TO AVOID DAMAGE TO THE MEMBERS DURING STORAGE, SUCH AS FUNGAL GROWTH AND EXPOSURE TO WEATHER CONDITIONS. THE FOLLOWING ARE SUGGESTIONS TO AID THE CONTRACTOR. ALL MATERIALS SHOULD BE STORED LEVEL ON SITE AND MUST BE RAISED OFF THE GROUND A MINIMUM OF 6″ BY MEANS OF BLOCKING AND SEPARATING SPACERS. IT IS RECOMMENDED THAT THE MATERIALS ARE COVERED WITH AN ADDITIONAL OPAQUE WATERPROOF MATERIAL (I.E. GOOD QUALITY TARPAULIN). WHEN MEMBERS ARE WRAPPED IN POLY OR ANOTHER MATERIAL WHICH MAY INHIBIT AIR FLOW, THE MATERIAL SHOULD HAVE SLOTS OR PERFORATIONS TO ALLOW FOR AIR FLOW AND PREVENT THE ACCUMULATION OF WATER AND/OR CONDENSATION. ENSURE THAT ALL EXPOSED MEMBERS ARE PROTECTED. PROTECTIVE MATERIAL SHOULD BE REMOVED ONLY AFTER THE ROOF OR STRUCTURE PROVIDING COVER IS INSTALLED. MEMBERS SHOULD BE CONSTANTLY PROTECTED FROM WEATHER DURING TRANSPORTATION, STORAGE, AND ERECTION.

FOR PERMIT
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OREGON
MARCH 23, 2011
SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 22.22.2
EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID
CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
STRUCTURAL GENERAL NOTES CONTINUED

S1.2

8.2. FOR INTERIOR GLULAM MEMBERS, THE HEAT IN THE BUILDING SHOULD BE GRADUALLY INCREASED OVER A TWO-TO-THREE-WEEK PERIOD IN ORDER TO PROVIDE A GRADUAL CHANGE IN MOISTURE CONTENT. DO NOT DIRECT ANY FORCED AIR HEATING SYSTEMS ONTO THE GLULAM MEMBERS. IT IS RECOMMENDED TO APPLY THE FINAL FINISH TO THE GLULAM MEMBER BEFORE HEAT IS APPLIED.

8.3. MEMBERS THAT ARE TO BE EXPOSED TO VIEW IN THE FINISHED STRUCTURE SHOULD BE HANDLED USING NYLON OR FABRIC SLINGS TO PREVENT SURFACE DAMAGE. THE CONTRACTOR SHOULD ALSO USE MEANS TO PROTECT CORNERS OF MEMBERS TO PREVENT CRUSHING DURING TRANSPORTATION, STORAGE, AND ERECTION. ALL BOLTS SHOULD BE GALVANIZED OR MADE SURE THAT THEY ARE FREE OF OIL TO PREVENT STAINING. GLULAM MEMBERS SHOULD BE TREATED AND STAINED PER THE ARCHITECT OF RECORD'S RECOMMENDATIONS. THE FOLLOWING ARE PROVIDED IN ORDER TO HELP GUIDE THE CONTRACTOR IN THE BEST PRACTICES TO PRESERVE THE QUALITY OF WOOD PRODUCTS. THESE NOTES ARE NOT INTENDED TO BE COMPREHENSIVE AND AN END ALL SOLUTION AND SHOULD BE TAKEN UNDER CONSIDERATION BY THE CONTRACTOR AND SUPPLEMENTED AS NECESSARY.

9. **PRESERVATIVE TREATMENT (PT):** WOOD MATERIALS THAT ARE REQUIRED TO BE “TREATED WOOD” IN ACCORDANCE WITH OSSC SECTION 2304.12 SHALL CONFORM TO THE APPROPRIATE STANDARDS OF THE AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) FOR SAWN LUMBER, GLUED LAMINATED TIMBER, ROUND POLES, WOOD PILES, AND MARINE PILES. FOLLOW AMERICAN LUMBER STANDARDS COMMITTEE (ALSC) QUALITY ASSURANCE PROCEDURES. PRODUCTS SHALL BEAR THE APPROPRIATE MARK. FASTENERS OR ANCHORS IN TREATED WOOD SHALL BE STAINLESS STEEL OR HOT-DIPPED GALVANIZED OR AS PER OSSC SECTION 2304.10.6.

9.1. MUD SILL PLATES IN NORMALLY DRY INTERIOR APPLICATIONS MAY BE TREATED WITH SODIUM BORATE (DOT - DISODIUM OCTABORATE TETRAHYDRATE) AS RECENT STUDIES HAVE NOTED LESS CONNECTOR CORROSION POTENTIAL THAN OTHER AVAILABLE WOOD TREATMENTS OR THE ORIGINAL CCA TREATED SILL PLATES. WOOD TREATED WITH SODIUM BORATE SHALL BE PROTECTED DURING SHIPMENT, STORAGE AND INSTALLATION TO MINIMIZE LEACHING OF THE WATER-SOLUBLE PRESERVATIVE FROM THE LUMBER. SODIUM BORATE PRESSURE TREATED PLATES DO NOT REQUIRE HOT-DIPPED GALVANIZED CONNECTORS.

9.2. IF USING PRESERVATIVE TREATMENTS OTHER THAN CCA OR SODIUM BORATE, FASTENERS MUST BE HOT-DIPPED GALVANIZED OR STAINLESS-STEEL. WOOD TREATED WITH ALKALINE COPPER QUATERNARY (ACQ) REQUIRES STEEL COMPONENTS IN CONTACT WITH THE WOOD TO BE STAINLESS (NAILS, BOLTS, SCREWS, WASHERS, AND LAG SCREWS). FASTENERS (NAILS, BOLTS, SCREWS, WASHERS, AND LAG SCREWS) ATTACHING TIMBER CONNECTORS (JOIST HANGERS, POST CAPS AND BASES, ETC.) TO PT WOOD SHALL HAVE SIMILAR CORROSION RESISTANCE PROPERTIES (MATCHING PROTECTIVE TREATMENTS) AS THE PROTECTED CONNECTOR; THAT IS, USE HOT-DIPPED GALVANIZED OR STAINLESS-STEEL FASTENERS. FASTENERS (NAILS, BOLTS, SCREWS, WASHERS, AND LAG SCREWS) ATTACHING SAWN TIMBER MEMBERS OR SHEATHING (SHEAR WALLS) TO PRESSURE TREATED WOOD SHALL BE CORROSION RESISTANT (HOT-DIPPED GALVANIZED OR STAINLESS-STEEL).

9.3. ALWAYS VERIFY THE SUITABILITY OF THE FASTENER PROTECTION/COATING WITH THE WOOD TREATMENT CHEMICAL MANUFACTURER/SUPPLIER.

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SHEET TITLE:
STRUCTURAL GENERAL NOTES CONTINUED

S1.3

50.00.00 SPECIAL INSPECTIONS

1. THE FOLLOWING STATEMENT AND SCHEDULES OF INSPECTIONS ARE THOSE SPECIAL INSPECTIONS AND TESTS THAT SHALL BE PERFORMED FOR THIS PROJECT. SPECIAL INSPECTORS SHALL REFERENCE THESE PLANS AND OSSC CHAPTER 17 FOR ALL SPECIAL INSPECTION REQUIREMENTS. THE OWNER SHALL RETAIN AN "APPROVED AGENCY" PER OSSC SECTION 1703 TO PROVIDE SPECIAL INSPECTIONS FOR THIS PROJECT. SPECIAL INSPECTORS SHALL BE QUALIFIED PERSONS PER OSSC SECTION 1704.2.4. SPECIAL INSPECTION REPORTS SHALL BE PROVIDED ON A WEEKLY BASIS. SUBMIT COPIES OF ALL INSPECTION REPORTS TO THE ARCHITECT/ENGINEER AND THE AUTHORITY HAVING JURISDICTION FOR REVIEW. IN ADDITION TO SPECIAL INSPECTION REPORTS AND TESTS, SUBMIT REPORTS AND CERTIFICATES NOTED IN OSSC SECTION 1704.5 TO THE AUTHORITY HAVING JURISDICTION. FINAL SPECIAL INSPECTION REPORTS WILL BE REQUIRED BY EACH SPECIAL INSPECTION FIRM PER OSSC SECTION 1704.2.4.

2. **STATEMENT OF SPECIAL INSPECTIONS:**
   - 2.1. THIS STATEMENT OF SPECIAL INSPECTIONS HAS BEEN WRITTEN WITH THE UNDERSTANDING THAT THE BUILDING OFFICIAL WILL:
     - 2.1.1. REVIEW AND APPROVE THE QUALIFICATIONS OF THE SPECIAL INSPECTOR.
     - 2.1.2. MONITOR THE SPECIAL INSPECTION ACTIVITY ON THE PROJECT SITE TO ENSURE THAT SPECIAL INSPECTORS ARE QUALIFIED AND PERFORMING THEIR DUTY AS STATED WITHIN THIS STATEMENT.
     - 2.1.3. REVIEW ALL SPECIAL INSPECTION REPORTS SUBMITTED TO THEM BY THE SPECIAL INSPECTOR.
     - 2.1.4. PERFORM INSPECTIONS AS REQUIRED BY OSSC SECTION 110.3.
   - 2.2. THE FOLLOWING SPECIAL INSPECTIONS ARE APPLICABLE TO THIS PROJECT:

     SPECIAL INSPECTIONS FOR STANDARD BUILDINGS (PER OSSC SECTION 1705.1) REQUIRED
     SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE (PER OSSC SECTION 1705.13) REQUIRED

3. **SPECIAL INSPECTION OF SHOP FABRICATED, GRAVITY LOAD-BEARING MEMBERS AND ASSEMBLIES:**
   - 3.1. SPECIAL INSPECTION OF SHOP FABRICATED, GRAVITY LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL BE VERIFIED BY THE SPECIAL INSPECTOR AS STATED IN SECTION 1704.2.5.
   - 3.2. INCLUDE THE FOLLOWING:
     - 3.2.1. PRIOR TO THE START OF FABRICATION: SPECIAL INSPECTOR(S), REPRESENTING THE OWNER, SHALL VISIT THE FABRICATOR'S SHOP(S) WHERE THE WORK IS TO BE PERFORMED, AND VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION, CONTROL OF WORKMANSHIP, MATERIAL CONTROL, AND FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS.
     - 3.2.2. FABRICATOR SHALL HAVE AVAILABLE FOR INSPECTOR'S REVIEW DETAILED PROCEDURES FOR MATERIAL CONTROL THAT DEMONSTRATE THE FABRICATOR'S ABILITY TO MAINTAIN SUITABLE RECORDS AND PROCEDURES SUCH THAT, AT ANY TIME DURING THE FABRICATION PROCESS, THE MATERIAL SPECIFICATION, GRADE, AND APPLICABLE TEST REPORTS FOR PRIMARY LOAD-CARRYING MEMBERS ARE CAPABLE OF BEING DETERMINED.

4. **STRUCTURAL STEEL:** PER OSSC SECTION 1705.2.1
   - 4.1. A QUALIFIED SPECIAL INSPECTOR OF AN "APPROVED AGENCY" PROVIDING QUALITY ASSURANCE (QA) SPECIAL INSPECTIONS FOR THE PROJECT SHALL REVIEW AND CONFIRM THE FABRICATOR AND ERECTOR'S QUALITY CONTROL (QC) PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO AISC 360 CHAPTER N, AISC 303, AWS D1.1, AND 2022 OSSC CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK.
   - 4.2. REQUIREMENTS:
     - 4.2.1. QA AGENCY PROVIDING SPECIAL INSPECTIONS SHALL PROVIDE PERSONNEL MEETING THE MINIMUM QUALIFICATION REQUIREMENTS FOR INSPECTION AND NONDESTRUCTIVE TESTING (NDT) PER AISC 360 SECTION N4.
       - 4.2.1.1. QC AGENCY SHALL SUBMIT QUALIFICATION DOCUMENTS PER AISC 341 SECTION J2 ON PROJECTS SUBJECT TO SPECIAL INSPECTIONS ON SEISMIC FORCE RESISTING SYSTEMS WITH R > 3.
       - 4.2.1.2. NDT PERSONNEL SHALL BE QUALIFIED PER AISC 341 SECTION J4.
       - 4.2.1.3. PROVIDE QC AND QA INSPECTIONS PER AISC 341 SECTION J5 THROUGH J10 AS APPLICABLE.
     - 4.2.2. VERIFY FABRICATOR AND ERECTOR QC PROGRAM PER AISC 360 SECTION N2.
     - 4.2.3. INSPECTION OF WELDS AND BOLTS BY BOTH QC AND QA PERSONNEL SHALL BE PER THE SCHEDULE OF SPECIAL INSPECTIONS IN THESE DRAWINGS. ALL PROVISIONS OF AWS D1.1 FOR STATICALLY LOADED STRUCTURES SHALL APPLY.
     - 4.2.4. NONDESTRUCTIVE TESTING (NDT) OF WELDS:
       - 4.2.4.1. NDT OF WELDED JOINTS PER AISC 360 SECTION N5
       - 4.2.4.2. RISK CATEGORY FOR DETERMINATION OF EXTENT OF NDT PER AISC 360 N5.5B IS NOTED IN THE DESIGN CRITERIA AND LOADS SECTION OF THESE GENERAL REQUIREMENTS.
       - 4.2.4.3. NDT PERFORMED SHALL BE DOCUMENTED AND REPORTS SHALL IDENTIFY THE TESTED WELD BY PIECE MARK AND LOCATION OF THE PIECE.
       - 4.2.4.4. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY LOCATION IN THE STRUCTURE, PIECE MARK AND LOCATION OF THE PIECE.
     - 4.2.5. ADDITIONAL INSPECTION TASKS PER AISC 360 SECTION N5.8.
     - 4.2.6. INSPECTION FOR COMPOSITE CONSTRUCTION SHALL BE DONE PER AISC 360 SECTION N6.
   - 4.3. POST-INSTALLED ANCHORS TO CONCRETE AND MASONRY: SHALL COMPLY WITH OSSC SECTION 1703. INSPECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS SET FORTH IN THE APPROVED ICC EVALUATION SERVICE REPORT (ICC-ESR) AND AS INDICATED BY THE DESIGN REQUIREMENTS SPECIFIED ON THE DRAWINGS. REFER TO THE POST-INSTALLED ANCHORS SECTION OF THE GENERAL NOTES FOR ANCHORS THAT ARE THE BASIS OF THE DESIGN. SPECIAL INSPECTOR SHALL VERIFY ANCHORS ARE AS SPECIFIED IN THE POST-INSTALLED ANCHORS SECTION OF THE GENERAL NOTES OR AS OTHERWISE SPECIFIED ON THE DRAWINGS. SUBSTITUTIONS REQUIRE APPROVAL BY THE SER AND REQUIRE SUBSTANTIATING CALCULATIONS AND A CURRENT 2022 OSSC RECOGNIZED ICC-ESR. SPECIAL INSPECTOR SHALL DOCUMENT IN THEIR SPECIAL INSPECTION REPORT COMPLIANCE WITH EACH OF THE ELEMENTS REQUIRED WITHIN THE APPLICABLE ICC-ESR.
   - 4.4. PREFABRICATED CONSTRUCTION: ALL PREFABRICATED CONSTRUCTION SHALL CONFORM TO OSSC SECTION 1703.

5. **SCHEDULES OF SPECIAL INSPECTIONS:**

5.1. TABLE 1705.6 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

NO.	SPECIAL INSPECTION	CONTINUOUS	PERIODIC
1	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY	-	X
2	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	-	X
3	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	-	X
4	DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL	X	-
5	PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	-	X

5.2. TABLE 1705.3 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

NO.	SPECIAL INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD
1	INSPECT REINFORCEMENT, INCLUDING PRE-STRESSING TENDONS, AND VERIFY PLACEMENT	-	X	ACI 318 CHAPTER 20 ACI 318 SECTIONS 25.2, 25.3, & 26.6.1-26.6.3
2	REINFORCING BAR WELDING:	-	-	-
2a	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706	-	X	AWS D1.4 ACI 318 SECTION 26.6.4
2b	INSPECT SINGLE PASS FILLET WELD MAXIMUM 5/16"	-	X	-
2c	INSPECT ALL OTHER WELDS	X	-	-
3	INSPECT ANCHORS CAST IN CONCRETE	-	X	ACI 318 SECTION 17.8.2
4	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS:	-	-	-
4a	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X	-	ACI 318 SECTION 17.8.2.4
4b	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4a	-	X	ACI 318 SECTION 17.8.2
5	VERIFY USE OF REQUIRED DESIGN MIX	-	X	ACI 318 CHAPTER 19 ACI SECTIONS 26.4.3 & 26.4.4 OSSC SECTION 1904.1, 1904.2
6	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-	ASTM C172 & C31 ACI 318 SECTIONS 26.5 & 26.12
7	INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-	ACI 318 SECTION 26.5
8	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	X	ACI 318 SECTIONS 26.5.3-26.5.5
14	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	-	X	ACI 318 SECTION 26.11.1.2(B)

5.3. REQUIRED SPECIAL INSPECTIONS OF WOOD CONSTRUCTION

NO.	SPECIAL INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD
1	HIGH LOAD DIAPHRAGMS:	-	-	OSSC SECTION 1705.5.1
1a	PANEL SHEATHING FOR GRADE AND THICKNESS	X	-	-
1b	NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES	X	-	-
1c	NAIL AND STAPLE DIAMETERS AND LENGTHS	X	-	-
1d	NUMBER OF FASTENER LINES, SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS	X	-	-
2	METAL PLATE CONNECTED WOOD TRUSSES SPANNING 60 FEET OR GREATER: TEMPORARY INSTALLATION RESTRAINT/BRACING AND PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING INSTALLATION	X	-	OSSC SECTION 1705.5.2
3	SHEAR WALLS (WHERE FASTENER SPACING OF THE SHEATHING IS 4 INCHES OR LESS ON CENTER):	-	-	
3a	ANCHOR BOLTS INCLUDING PROPER BOTTOM PLATE SIZES (2X AND 3X) AND PLATE WASHERS	-	X	-
3b	HOLD-DOWNS (HD) AND CONTINUOUS ROD TIE-DOWN SYSTEMS (TDS) INCLUDING SQUASH BLOCKS AND ANCHORS TO CONCRETE	-	X	-
3c	A35 AND LPT SHEAR CONNECTORS	-	X	-
3d	STRAP CONNECTORS	-	X	-
3e	BOUNDARY EDGE NAILING	-	X	-
3f	PLATE NAILING AND PANEL EDGE NAILING FOR SIZE AND SPACING	-	X	-
3g	BLOCKING	-	X	-
4	BLOCKED AND UNBLOCKED DIAPHRAGMS (WHERE FASTENER SPACING OF THE SHEATHING IS 4 INCHES OR LESS ON CENTER):	-	-	
4a	BLOCKING AND STRAP CONNECTIONS	-	X	-
4b	BOUNDARY EDGE AND PANEL SHEAR NAILING SIZE AND SPACING	-	X	-
5	MOISTURE CONTENT OF WOOD STUDS, PLATES, BEAMS, DECKING, AND JOISTS	-	-	AS DIRECTED BY THE CONTRACTOR TO MEET MOISTURE CONTENT REQUIREMENTS
6	ROOF TRUSS 'HURRICANE CLIPS'	-	X	-

5.4. MINIMUM REQUIREMENTS FOR INSPECTIONS OF STRUCTURAL STEEL CONSTRUCTION

NO.	SPECIAL INSPECTION	QC	QA	REFERENCE STANDARD
	INSPECTION TASKS PRIOR TO WELDING			
1	WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	P	O	AISC 360 TABLE N5.4-1
2	WELDING PROCEDURE SPECIFICATIONS (WPS'S) AVAILABLE	P	P	AISC 360 TABLE N5.4-1
3	MANUFACTURING CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	P	P	AISC 360 TABLE N5.4-1
4	MATERIAL IDENTIFICATION (TYPE/GRADE)	O	O	AISC 360 TABLE N5.4-1
5	WELDER IDENTIFICATION SYSTEM	O	O	AISC 360 TABLE N5.4-1
6	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) JOINT PREPARATION DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) CLEANLINESS (CONDITION OF STEEL SURFACES) TACKING (TACK WELDING QUALITY AND LOCATION) BACKING TYPE AND FIT (IF APPLICABLE)	O	O	AISC 360 TABLE N5.4-1
7	FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y-, AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY) JOINT PREPARATION DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) CLEANLINESS (CONDITION OF STEEL SURFACES) TACKING (TACK WELDING QUALITY AND LOCATION)	P	O	AISC 360 TABLE N5.4-1
8	CONFIGURATION AND FINISH OF ACCESS HOLES	O	O	AISC 360 TABLE N5.4-1
9	FIT-UP OF FILLET WELDS DIMENSIONS (ALIGNMENT, GAPS AT ROOT) CLEANLINESS (CONDITION OF STEEL SURFACES) TACKING (TACK WELDING QUALITY AND LOCATION)	O	O	AISC 360 TABLE N5.4-1
10	CHECK WELDING EQUIPMENT	O	-	AISC 360 TABLE N5.4-1
	INSPECTION TASKS DURING WELDING			
1	USE OF QUALIFIED WELDERS	O	O	AISC 360 TABLE N5.4-2
2	CONTROL AND HANDLING OF WELDING CONSUMABLES PACKAGING EXPOSURE CONTROL	O	O	AISC 360 TABLE N5.4-2
3	NO WELDING OVER CRACKED TACK WELDS	O	O	AISC 360 TABLE N5.4-2
4	ENVIRONMENTAL CONDITIONS WIND SPEED WITHIN LIMITS PRECIPITATION AND TEMPERATURE	O	O	AISC 360 TABLE N5.4-2
5	WPS FOLLOWED SETTINGS ON WELDING EQUIPMENT TRAVEL SPEED SELECTED WELDING MATERIALS SHIELDING GAS TYPE/ FLOWRATE PREHEAT APPLIED INTERPASS TEMPERATURE MAINTAINED (MIN/MAX) PROPER POSITION (F, V, H, OH)	O	O	AISC 360 TABLE N5.4-2
6	WELDING TECHNIQUES INTERPASS AND FINAL CLEANING EACH PASS WITHIN PROFILE EACH PASS MEETS QUALITY REQUIREMENTS	O	O	AISC 360 TABLE N5.4-2
	INSPECTION TASKS AFTER WELDING			
1	WELDS CLEANED	O	O	AISC 360 TABLE N5.4-3
2	SIZE, LENGTH, AND LOCATIONS OF WELDS	P	P	AISC 360 TABLE N5.4-3
3	WELDS MEET VISUAL ACCEPTANCE CRITERIA CRACK PROHIBITION WELD/BASE-METAL FUSION CRATER CROSS SECTION WELD PROFILES WELD SIZE UNDERCUT POROSITY	P	P	AISC 360 TABLE N5.4-3
4	ARC STRIKES	P	P	AISC 360 TABLE N5.4-3
5	K-AREA	P	P	AISC 360 TABLE N5.4-3
6	WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES	P	P	AISC 360 TABLE N5.4-3
7	BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	P	P	AISC 360 TABLE N5.4-3
8	REPAIR ACTIVITIES	P	P	AISC 360 TABLE N5.4-3
9	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	P	P	AISC 360 TABLE N5.4-3
10	NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR	P	P	AISC 360 TABLE N5.4-3

5.4.1. (O) - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS

5.4.2. (P) - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER, EACH BOLTED CONNECTION, OR EACH STEEL ELEMENT

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MARCH 23, 2011
SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 22.22.2

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

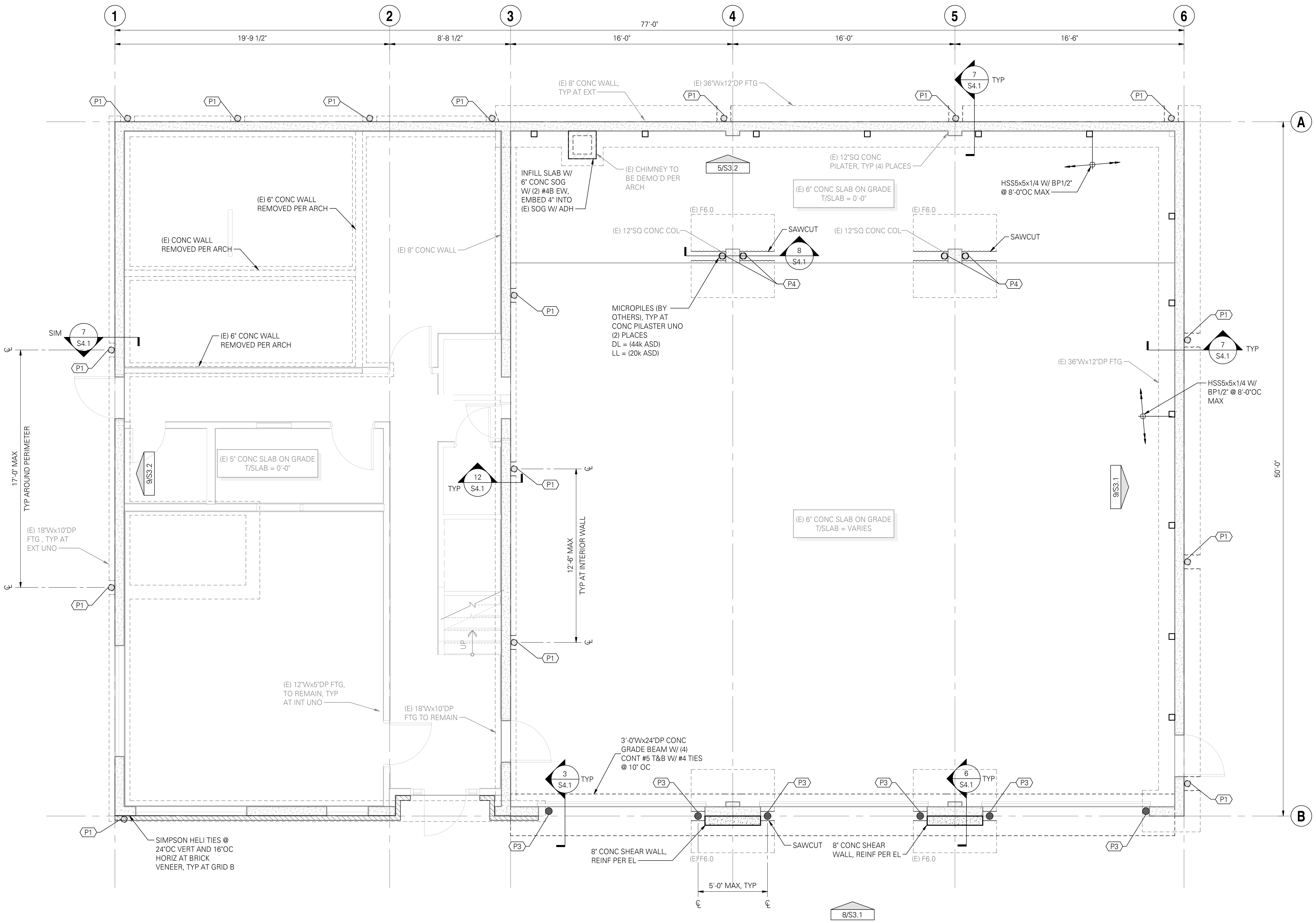
#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
STRUCTURAL SPECIAL INSPECTIONS

S1.4

FOR PERMIT
The Contractor shall not use these drawings for construction until Contractor receives written approval for use in construction by the authority having jurisdiction and DCI Engineers.



FIRST FLOOR FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION PLAN NOTES:

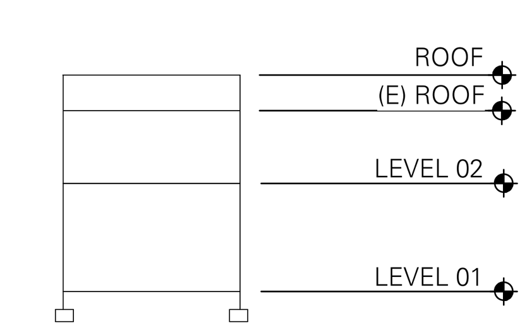
1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
3. TYPICAL TOP OF INTERIOR (T/INTERIOR) FOOTING ELEVATION = 0'-0", UNO. TYPICAL TOP OF EXTERIOR (T/EXTERIOR) FOOTING ELEVATIONS = 0'-10", UNO.
4. ALL FOOTINGS AND SLABS TO BEAR ON COMPETENT NATIVE SOIL AND/OR STRUCTURAL FILL, IMPROVED SOIL. SUBGRADE PREPARATION, STRUCTURAL FILL, DRAINAGE SYSTEM, AND OTHER REQUIREMENTS PER GEOTECH REPORT AS NOTED IN THE STRUCTURAL GENERAL NOTES.
5. MICRO-PILES / HELICAL PILES PER GENERAL NOTES.
   ○ = INDICATES MICRO-PILE AT EXISTING CONCRETE
   ● = INDICATES MICRO-PILE AT NEW GRADE BEAM
6. TYPICAL DETAILS PER:
   - 1/S4.1 TYPICAL LAP SPLICE SCHEDULE AND DEVELOPMENT LENGTH SCHEDULE
   - 2/S4.1 STANDARD HOOKS AND BAR BENDS
   - 4/S4.1 SLAB ON GRADE INFILL

SPREAD FOOTING SCHEDULE

MARK	SIZE			REINFORCING	COMMENTS
	LENGTH	WIDTH	DEPTH		
(E) F6.0	6'-0"	6'-0"	1'-0"	(E) (12) #6B EW	

MICROPILE FORCES

PILE TYPE	MICROPILE FORCES			
	P_D	P_L	P_S BSE-2E	P_S BSE-1E
P1	40k	30k	---	---
P2	8k	10k	---	---
P3	15k	5k	120k	55k
P4	45k	20k	---	---



NOTE:
DARKENED LINES DESIGNATE AREA OF WORK.

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CONSTRUCTION

REVISIONS:
DATE DESCRIPTION

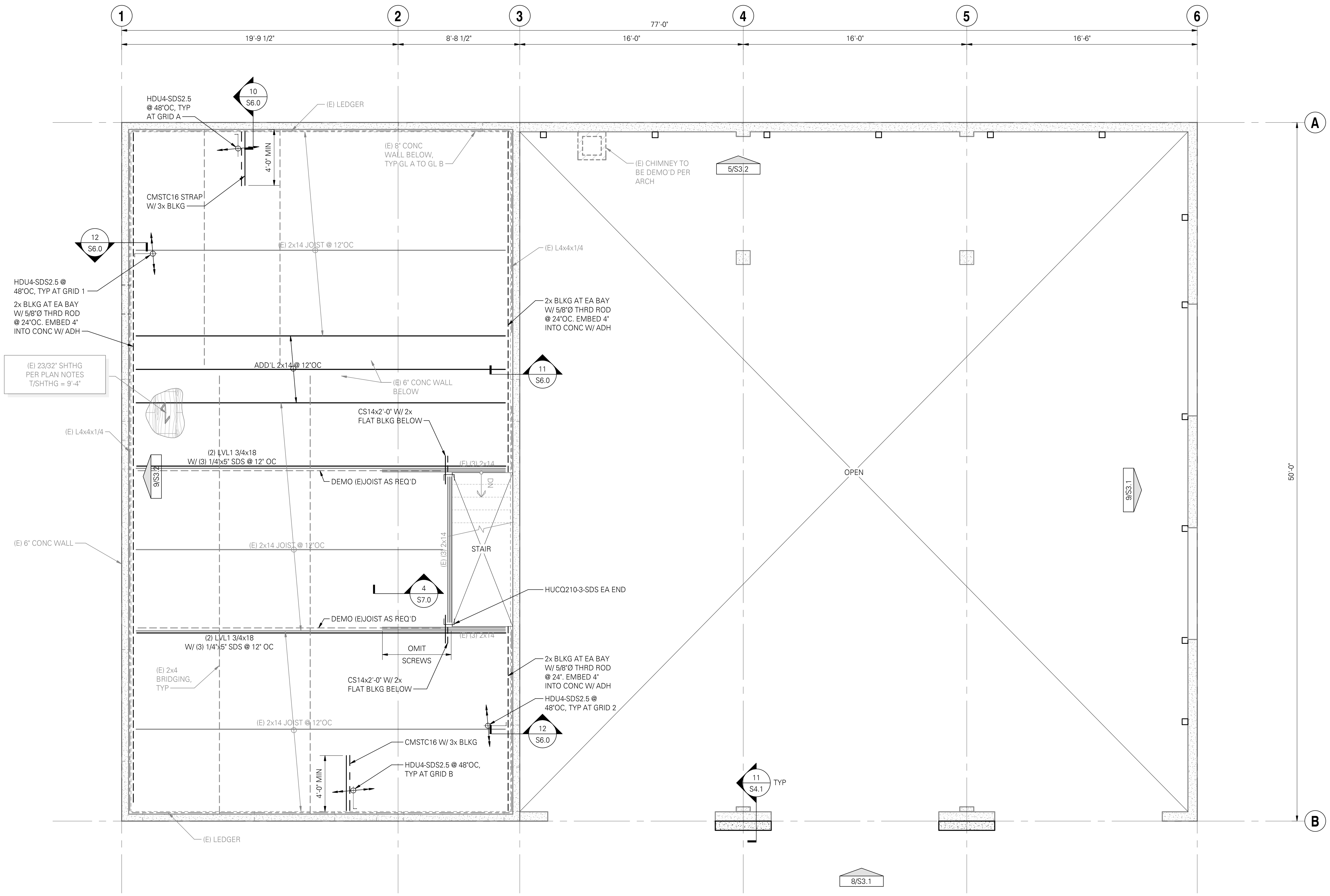
DATE: SEPTEMBER 2025

SHEET TITLE:
STRUCTURAL FIRST FLOOR FOUNDATION PLAN

S2.1

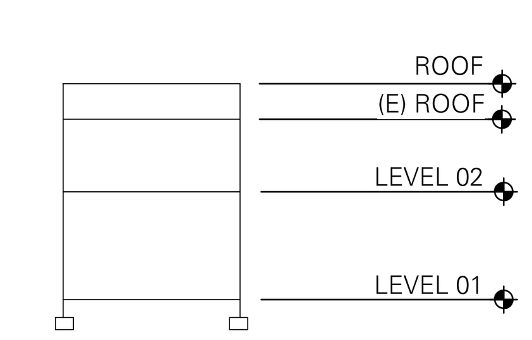
FLOOR FRAMING PLAN NOTES:

1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
3. ALL 2x HANGERS TO BE FACE MOUNT TYPE LUS, UNO.



SECOND FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"



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REVISIONS:

#	DATE	DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
STRUCTURAL SECOND FLOOR FRAMING PLAN

S2.2

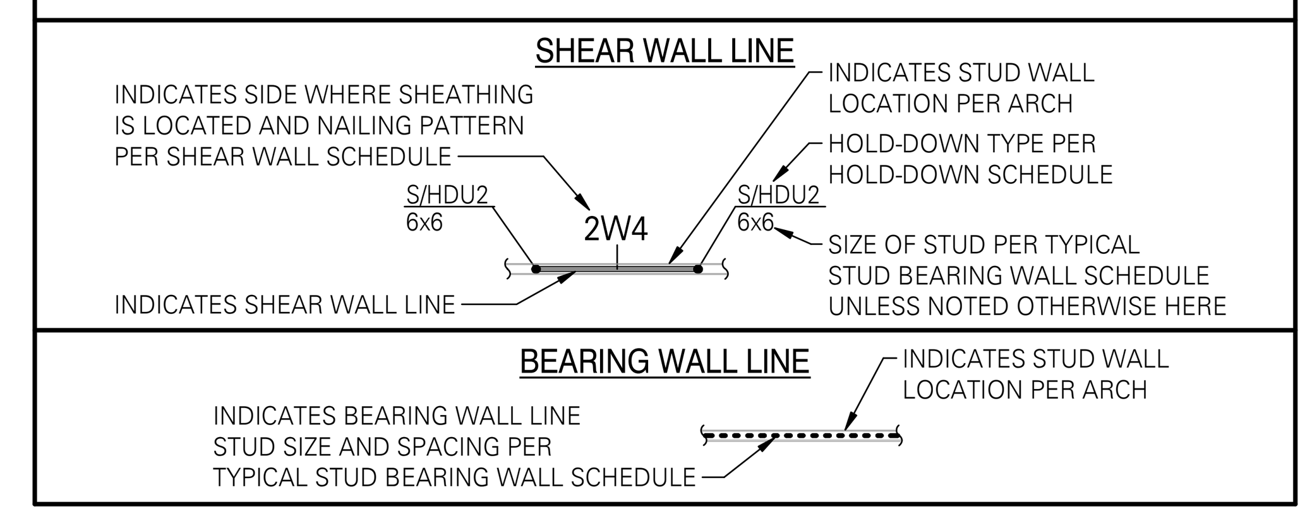
ROOF FRAMING PLAN NOTES:

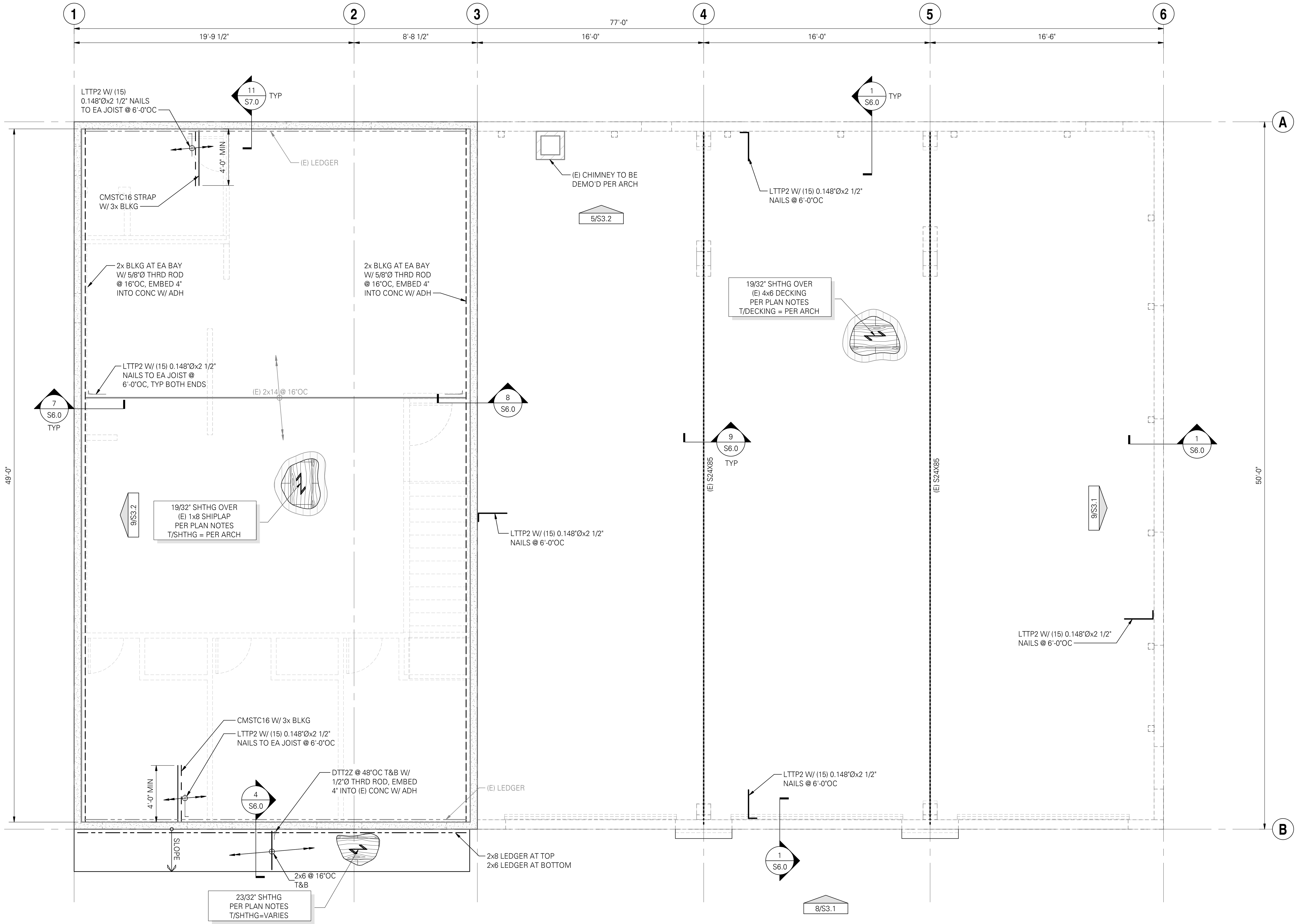
1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
3. ROOF SHEATHING PER PLAN AND STRUCTURAL GENERAL NOTES. SHEATHING TO BE NAILED TO ROOF FRAMING WITH 0.148"DIAx3" NAILS @ 4"OC AT SUPPORTED PANEL EDGES AND 6"OC AT FIELD. LAY SHEATHING WITH FACE GRAIN (LONG DIRECTION) PERPENDICULAR TO SUPPORTS AND STAGGER PANEL END JOINTS.
4. ALL FASCIA BOARDS TO BE 1 1/4x5 1/4 WEATHER RESISTANT LUMBER MINIMUM UNO ON PLANS, DETAILS OR SCHEDULES.

STUD AND SHEAR WALL PLAN NOTES:

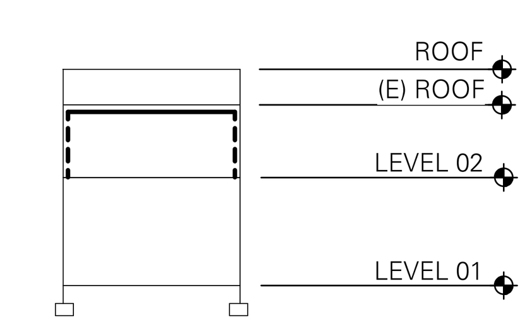
1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. LUMBER GRADE PER STRUCTURAL GENERAL NOTES.
3. SHEAR WALL AND NAILING REQUIREMENTS PER SHEAR WALL SCHEDULE 10/S5.1.
4. ALL NON-BEARING W6 WALLS TO BE 2x6 @ 16"OC, TYP UNO.

BEARING/SHEAR WALL LINE KEY





ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



NOTE:
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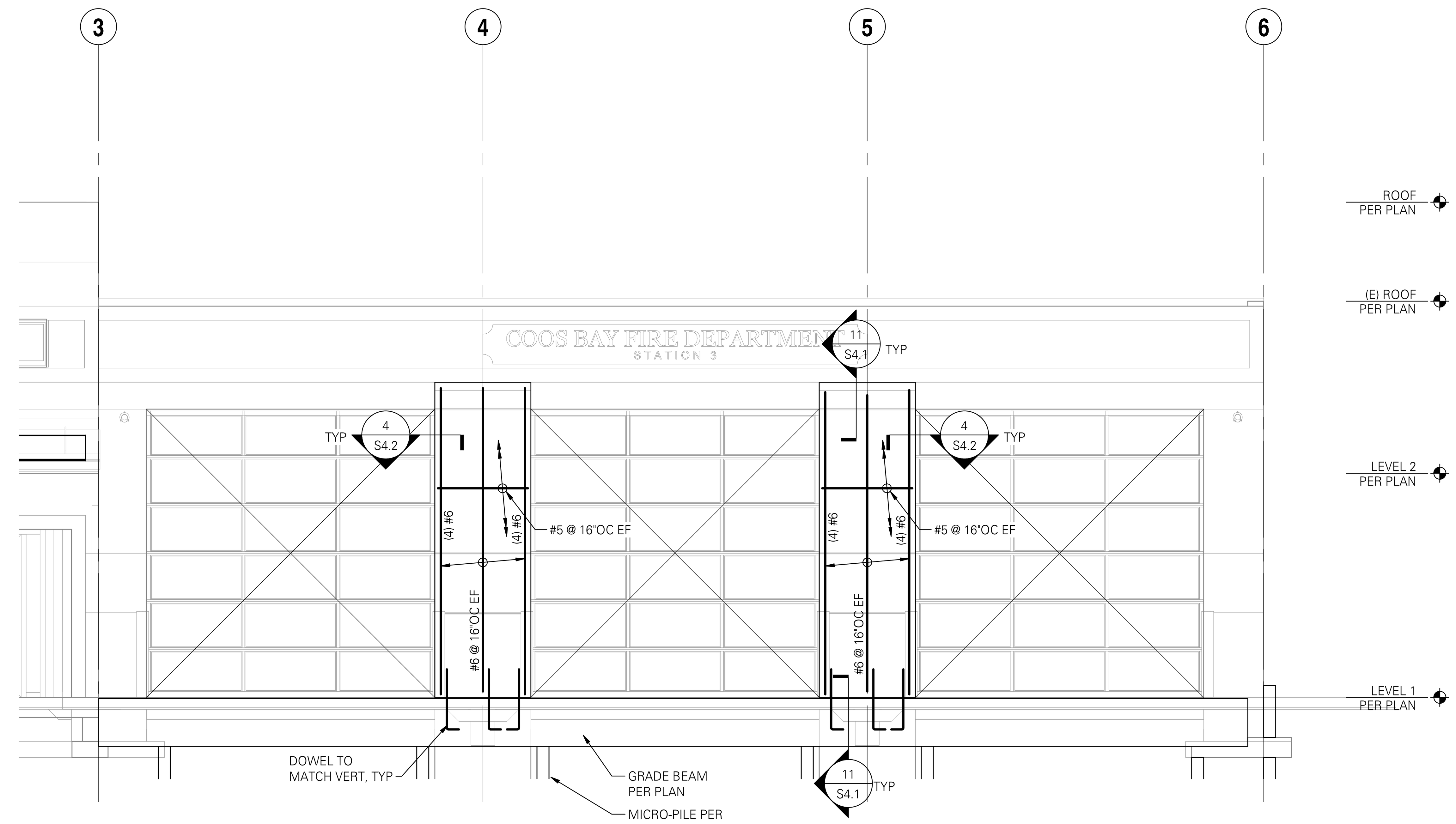
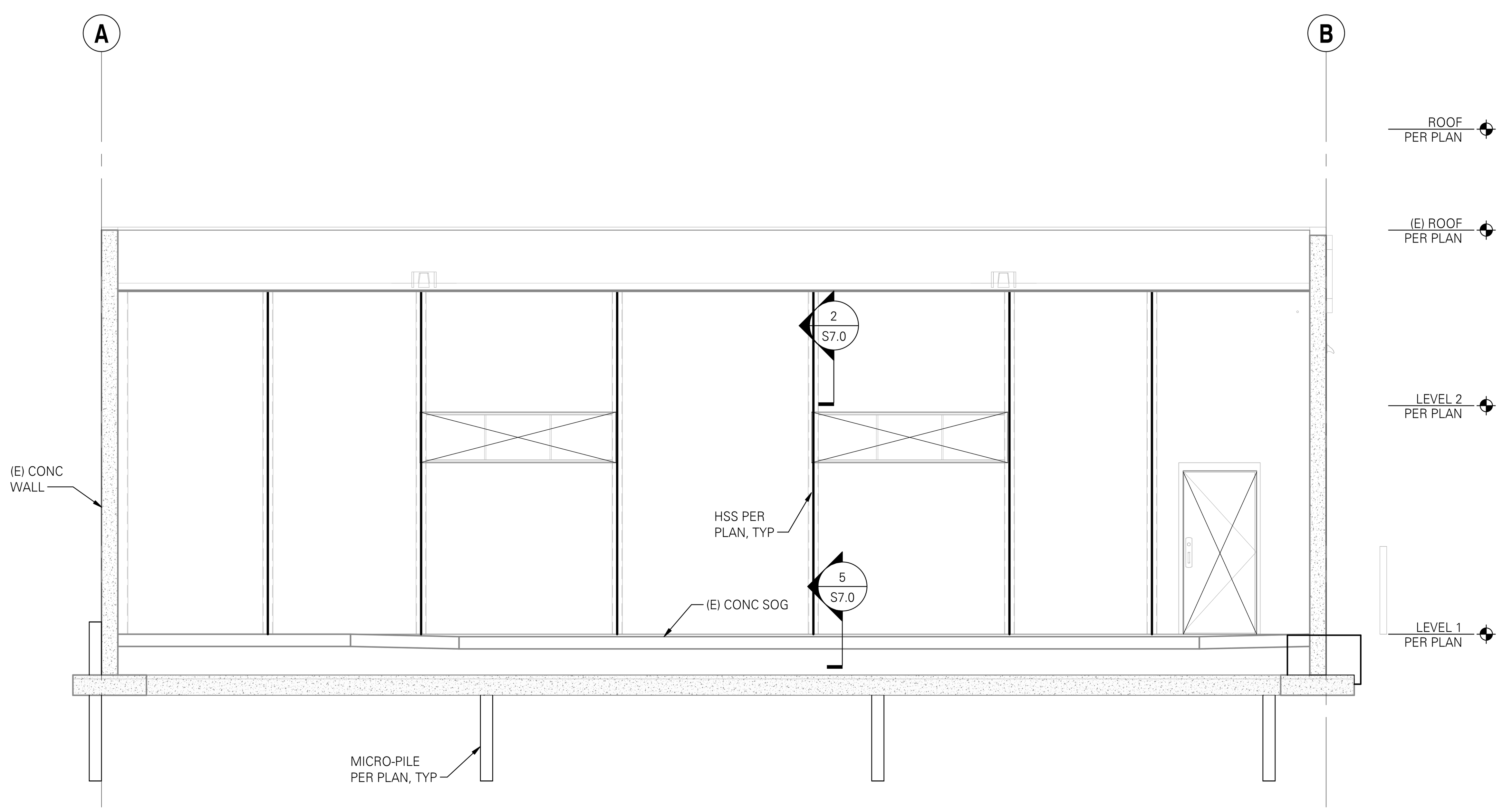
SHEET TITLE:
STRUCTURAL ROOF FRAMING PLAN

S2.3



8 **EXTERIOR SOUTH ELEVATION**

SCALE: 1/4" = 1'-0"



9 **EAST ELEVATION**

SCALE: 1/4" = 1'-0"

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#	DATE	DESCRIPTION

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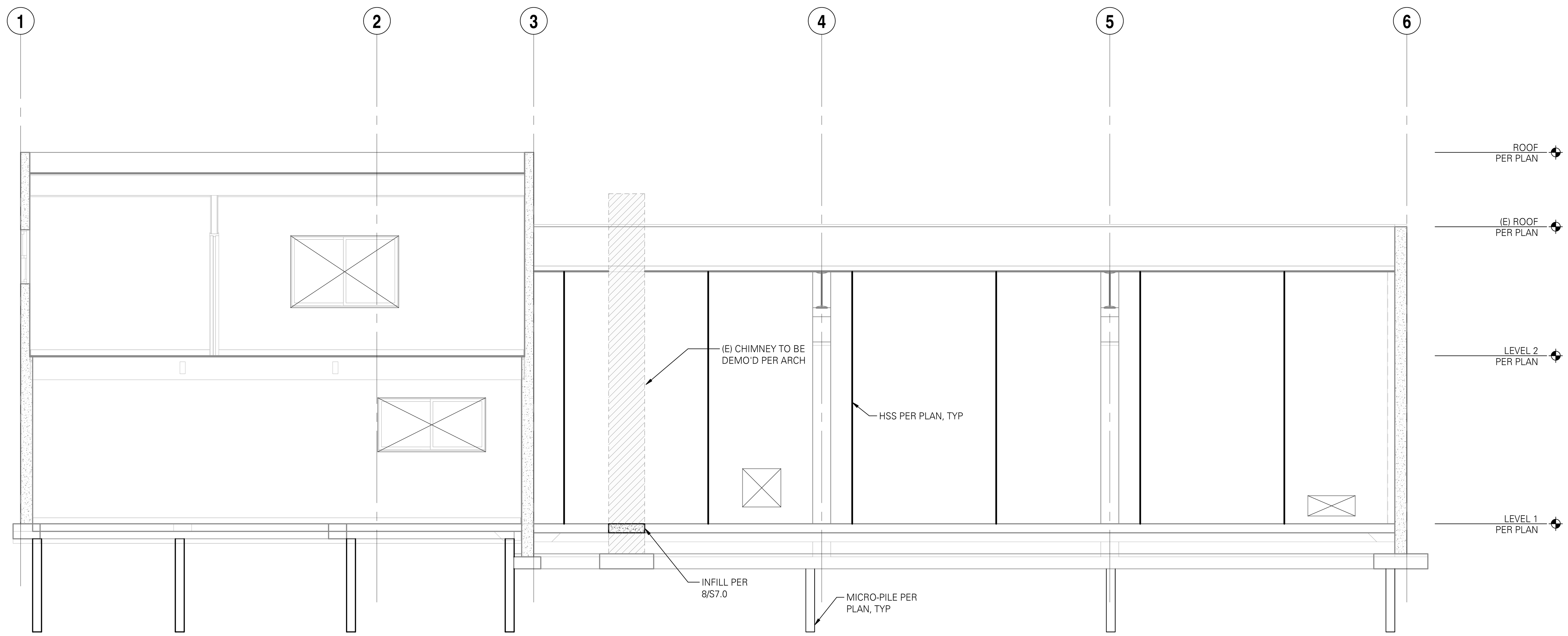
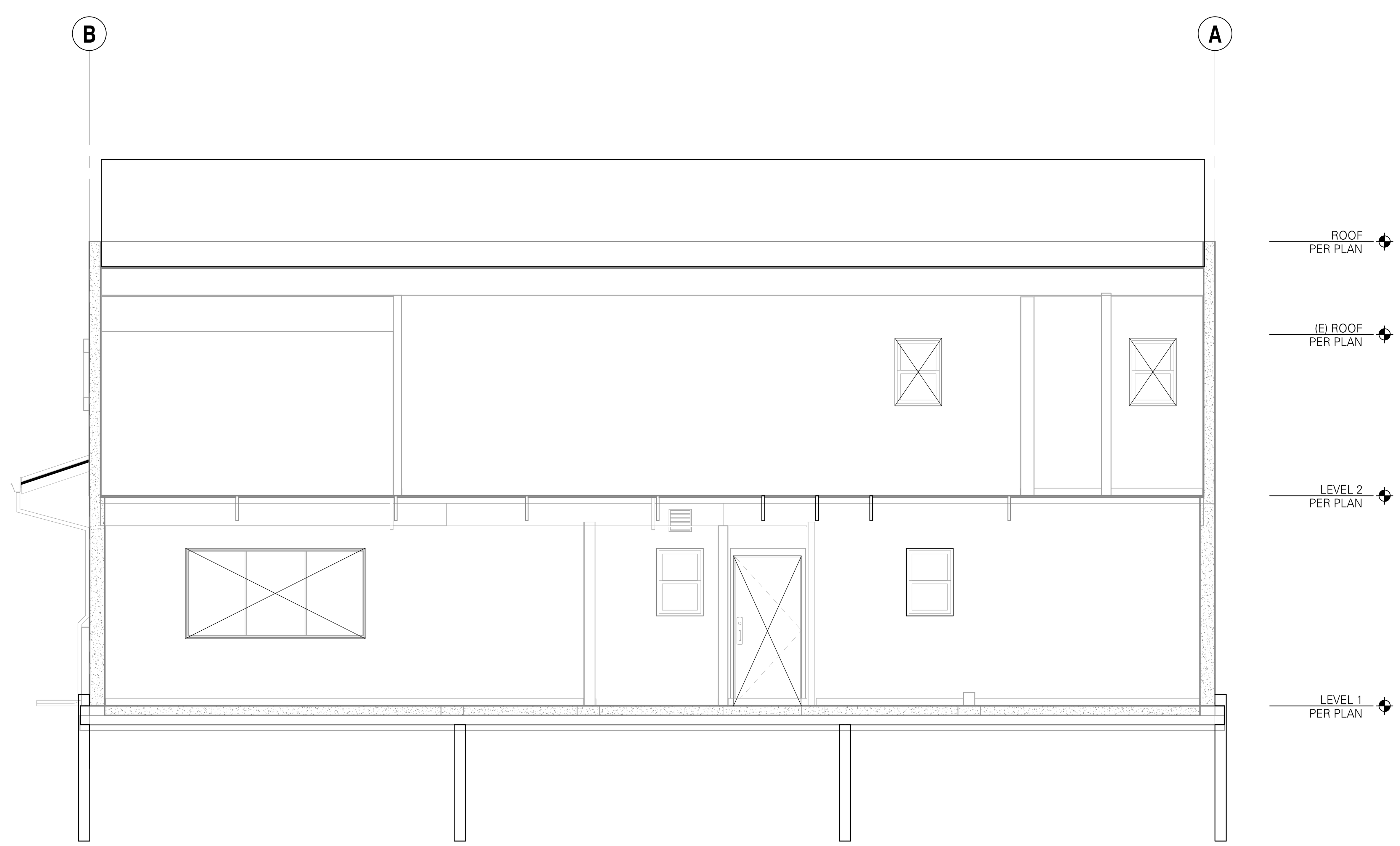
SHEET TITLE:
STRUCTURAL ELEVATIONS

S3.1



5 **NORTH ELEVATION**

SCALE: 1/4" = 1'-0"



9 **EAST ELEVATION**

SCALE: 1/4" = 1'-0"

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SHEET TITLE:
STRUCTURAL ELEVATIONS

S3.2

01400	GRADE 60 REINFORCING				
BAR SIZE	MISCELLANEOUS BARS		TOP BARS (see note #5)		HOOKED BARS
	Ld	Splice	Ld	Splice	Ldh
f'c = 4000psi					
#3	15	19	19	25	7
#4	19	25	25	33	11
#5	24	31	31	41	15
#6	29	37	37	49	20
#7	42	54	54	71	25
#8	48	62	62	81	30

NOTES:

1. ALL TABULATED VALUES ARE IN INCHES.

2. VALUES FOR UNCOATED REINFORCING AND NORMAL WEIGHT CONCRETE CALCULATED PER ACI 318-19 SECTION 25.4.2.4. CALCULATIONS ASSUME THAT (Cb+Ktr)/db = 1.5, WITH CLEAR SPACING > db, CLEAR COVER > db AND MINIMUM STIRRUPS OR TIES THROUGHOUT Ld OR CLEAR SPACING > 2db AND CLEAR COVER > db.

3. DEVELOP ALL REINFORCING IN STRUCTURAL SLABS WITH MINIMUM DEVELOPMENT LENGTH Ld.

4. Ldh = DEVELOPMENT LENGTH OF BAR WITH STANDARD HOOK.

5. TOP BAR = HORIZONTAL BAR WITH MORE THAN 12" OF FRESH CONCRETE BELOW OR AS NOTED ON DOCUMENTS AS "TOP BAR".

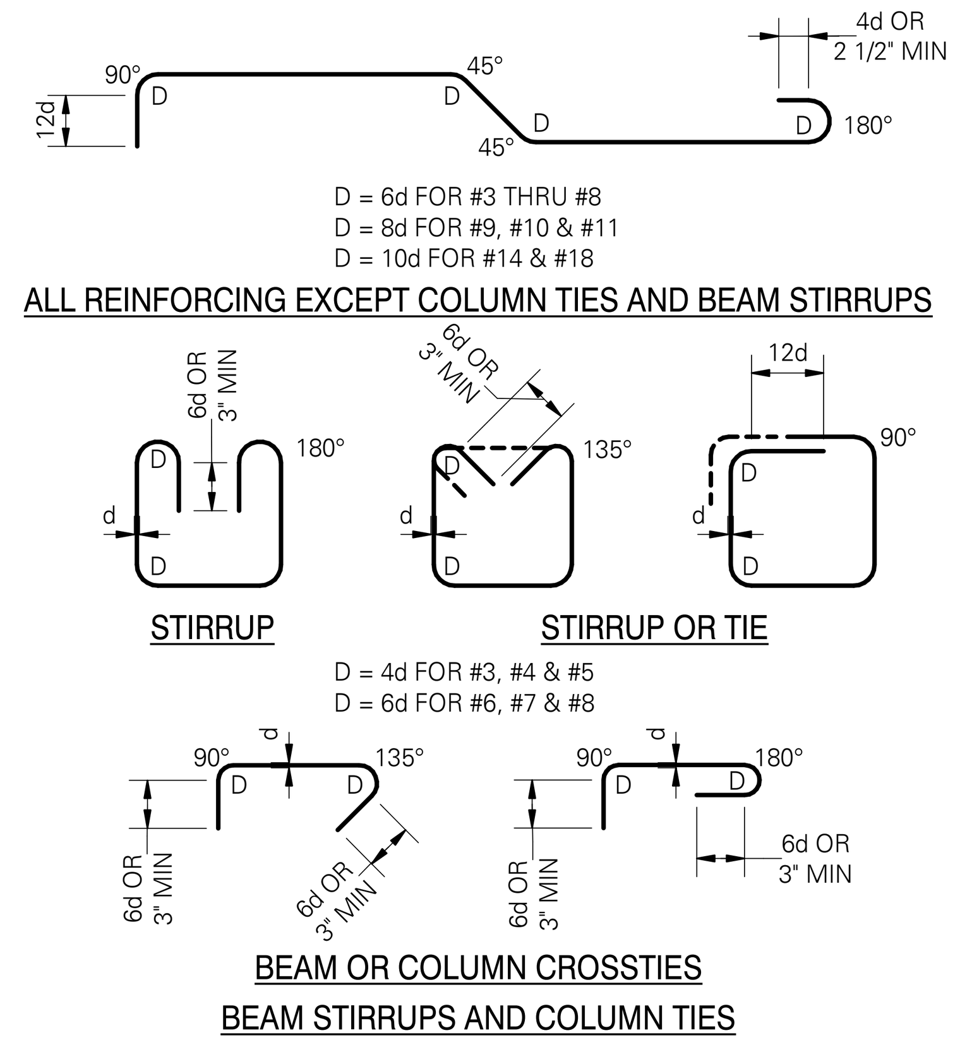
6. LAP SPLICE OF DIFFERENT SIZED BARS TO BE THE LARGER OF Ld OF THE LARGER BAR OR SPLICE LENGTH OF THE SMALLER BAR.

8. LAP SPLICE OF DIFFERENT GRADES OF REINFORCING TO BE THE LARGER OF Ld OF THE HIGHER GRADE BAR OR SPLICE LENGTH OF THE LOWER GRADE BAR.

10. SHEAR WALL REINFORCING LAP SPLICE SCHEDULE PER 8/S5.1.

1 TYPICAL LAP SPLICE AND DEVELOPMENT LENGTH SCHEDULE

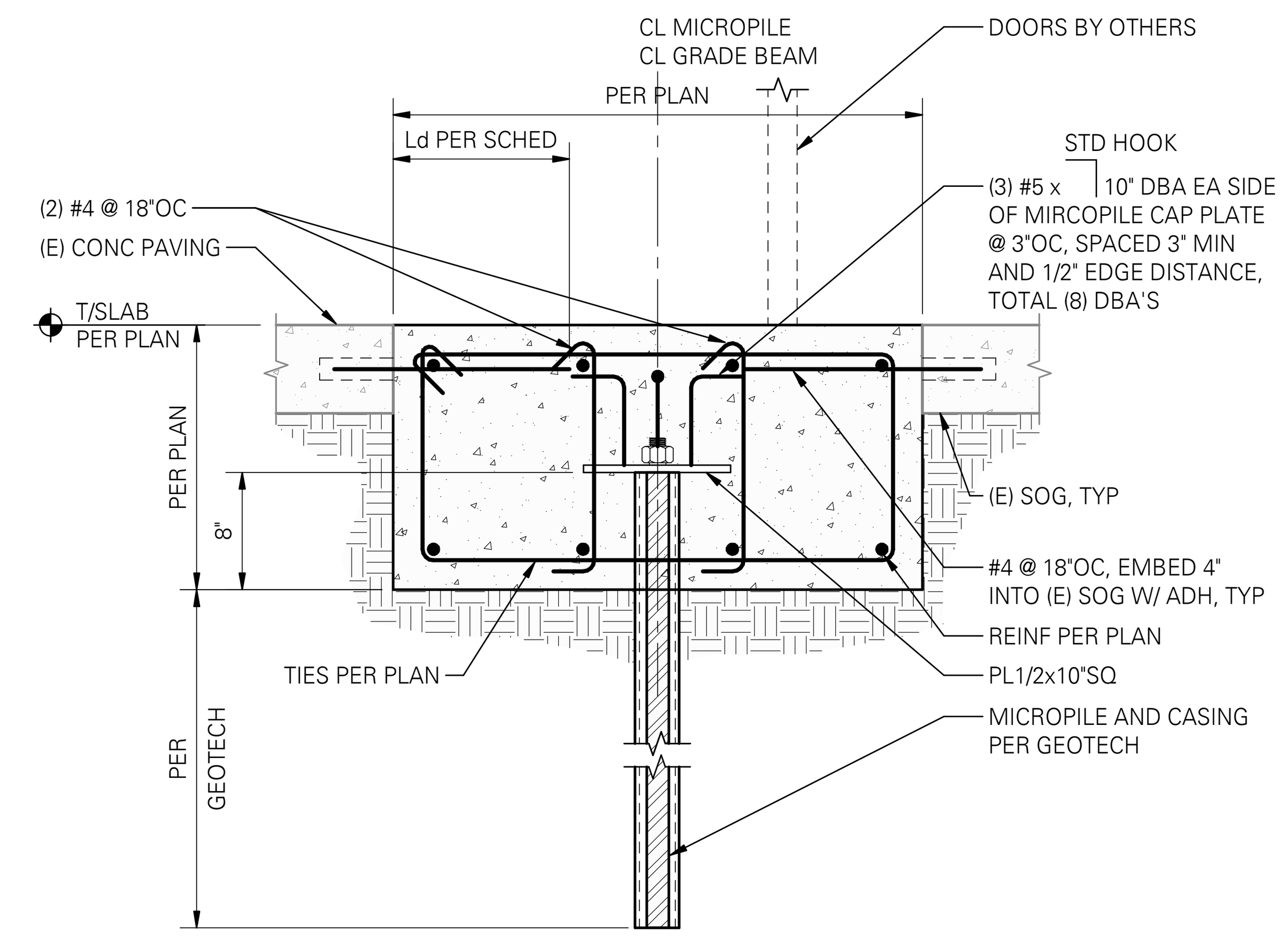
SCALE: 3/4" = 1'-0" (01400)



NOTE:
TIES AND CROSSTIES FOR SHEAR WALL BOUNDARY ELEMENTS SHALL BE DETAILED AS COLUMN TIES/CROSSTIES.

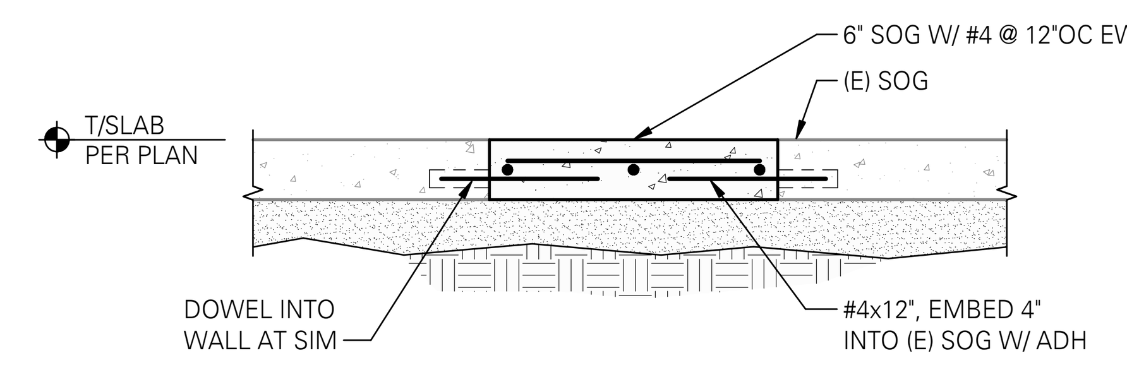
2 STANDARD HOOKS AND BENDS

SCALE: 3/4" = 1'-0" (03400)



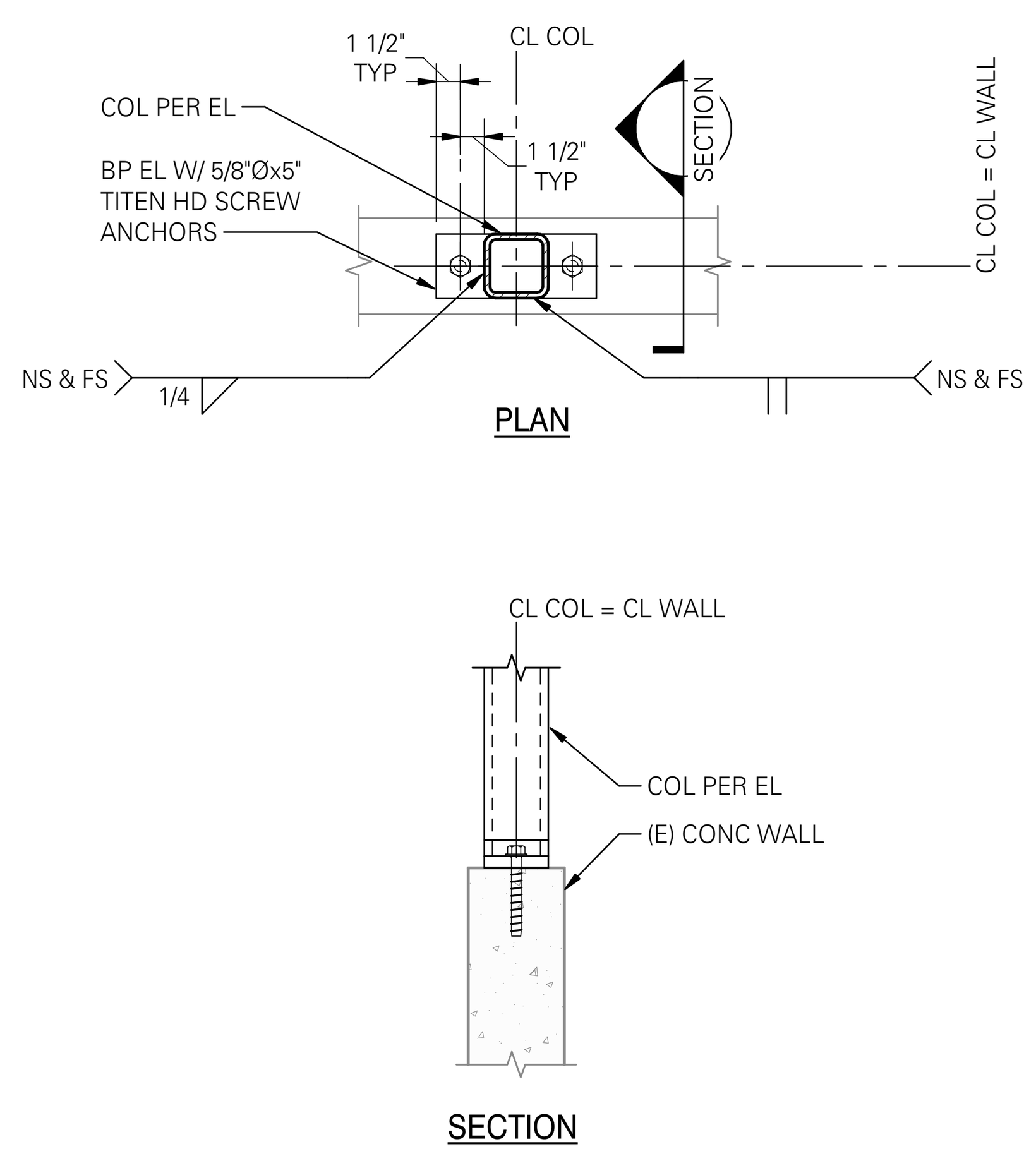
3 TYPICAL MICROPILE TO CONCRETE GRADE BEAM

SCALE: 1" = 1'-0"



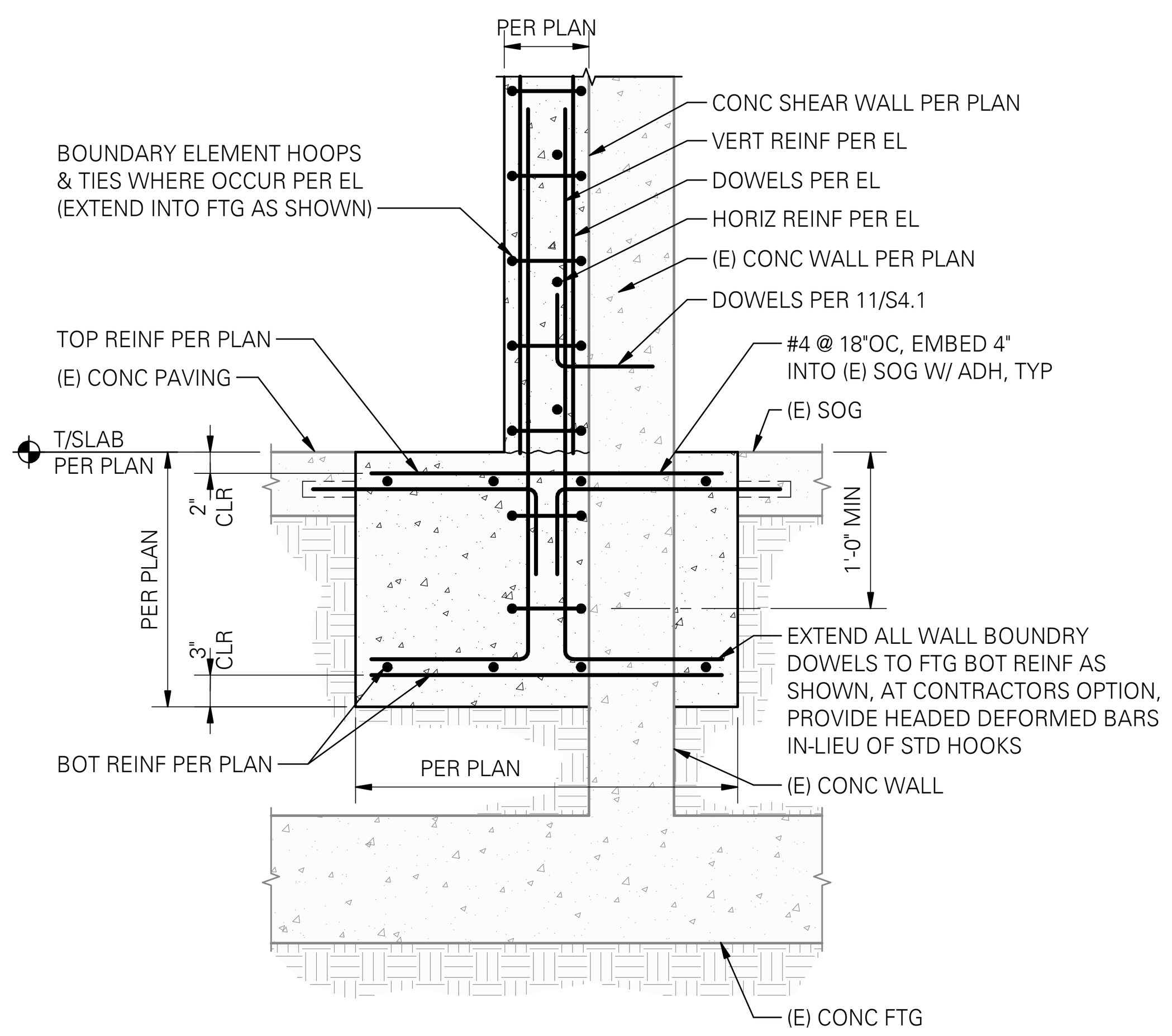
4 SLAB ON GRADE INFILL

SCALE: 3/4" = 1'-0"



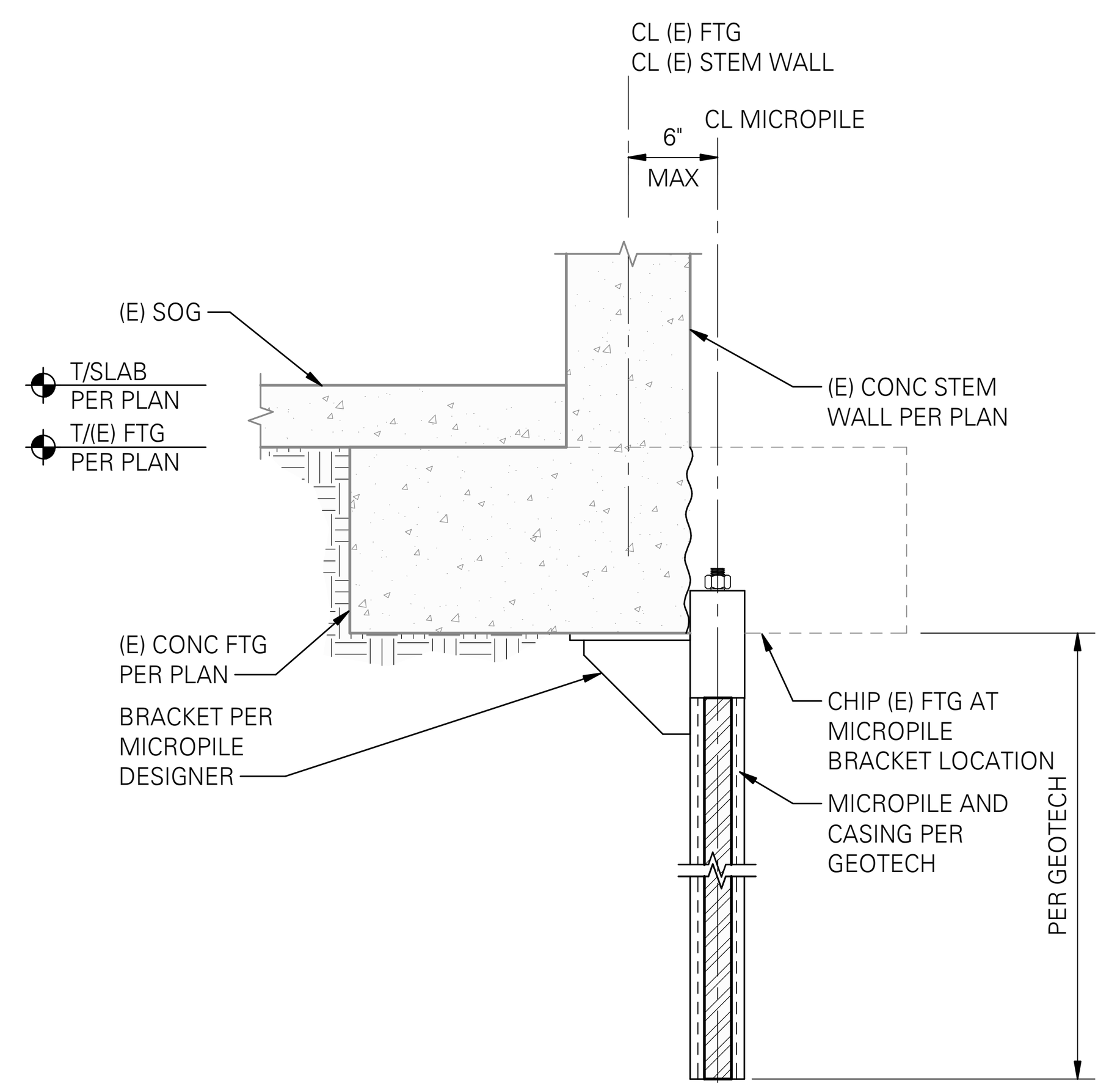
5 HSS CONNECTION TO EXISTING WALL

SCALE: 1" = 1'-0" (05032)



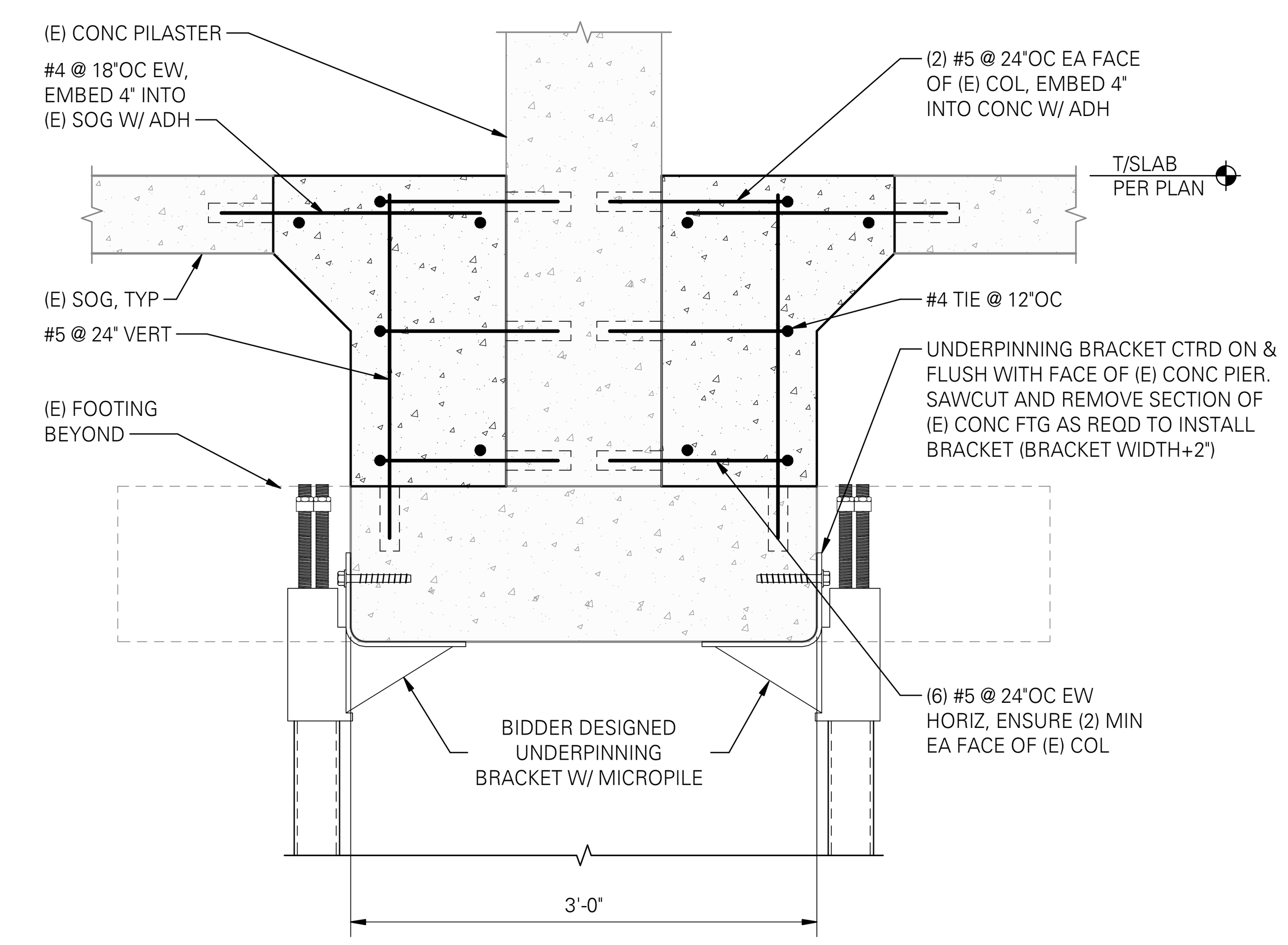
6 FOOTING AT CONCRETE SHEAR WALL

SCALE: 3/4" = 1'-0" (03136)



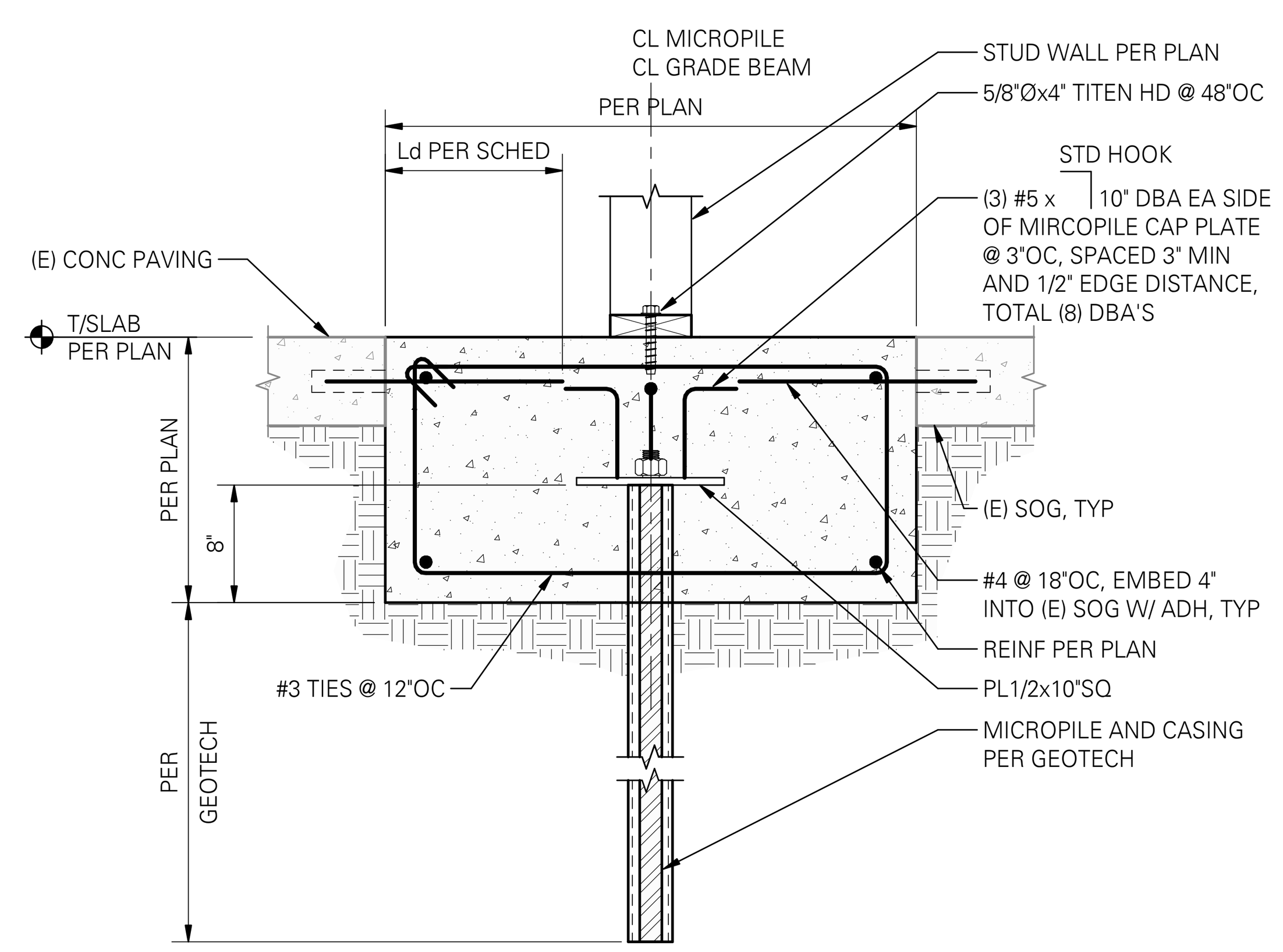
7 TYPICAL MICROPILE TO EXISTING CONCRETE FOOTING

SCALE: 1" = 1'-0"



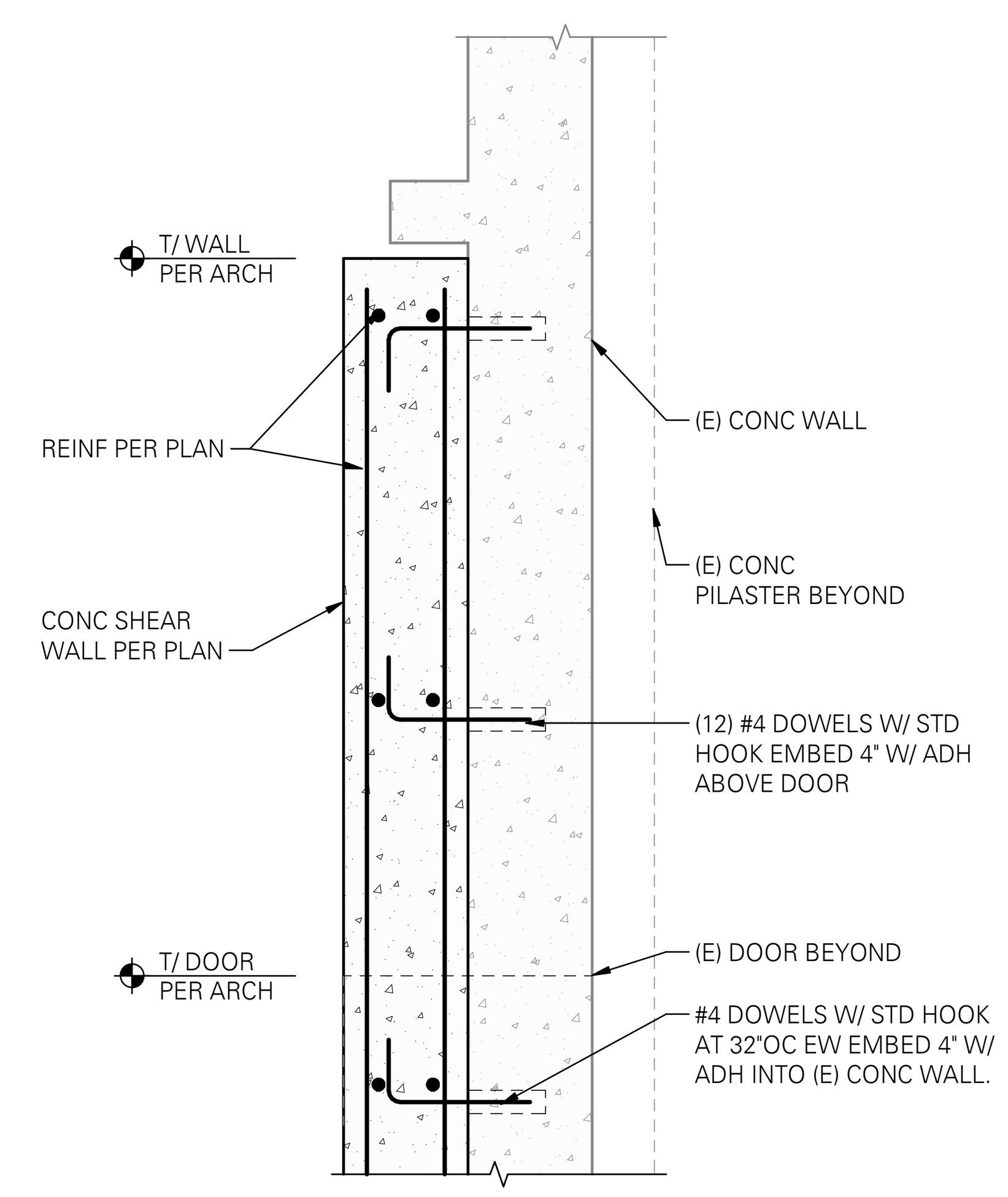
8 MICROPILE AT EXISTING CONCRETE PIER FOOTING

SCALE: 1" = 1'-0"



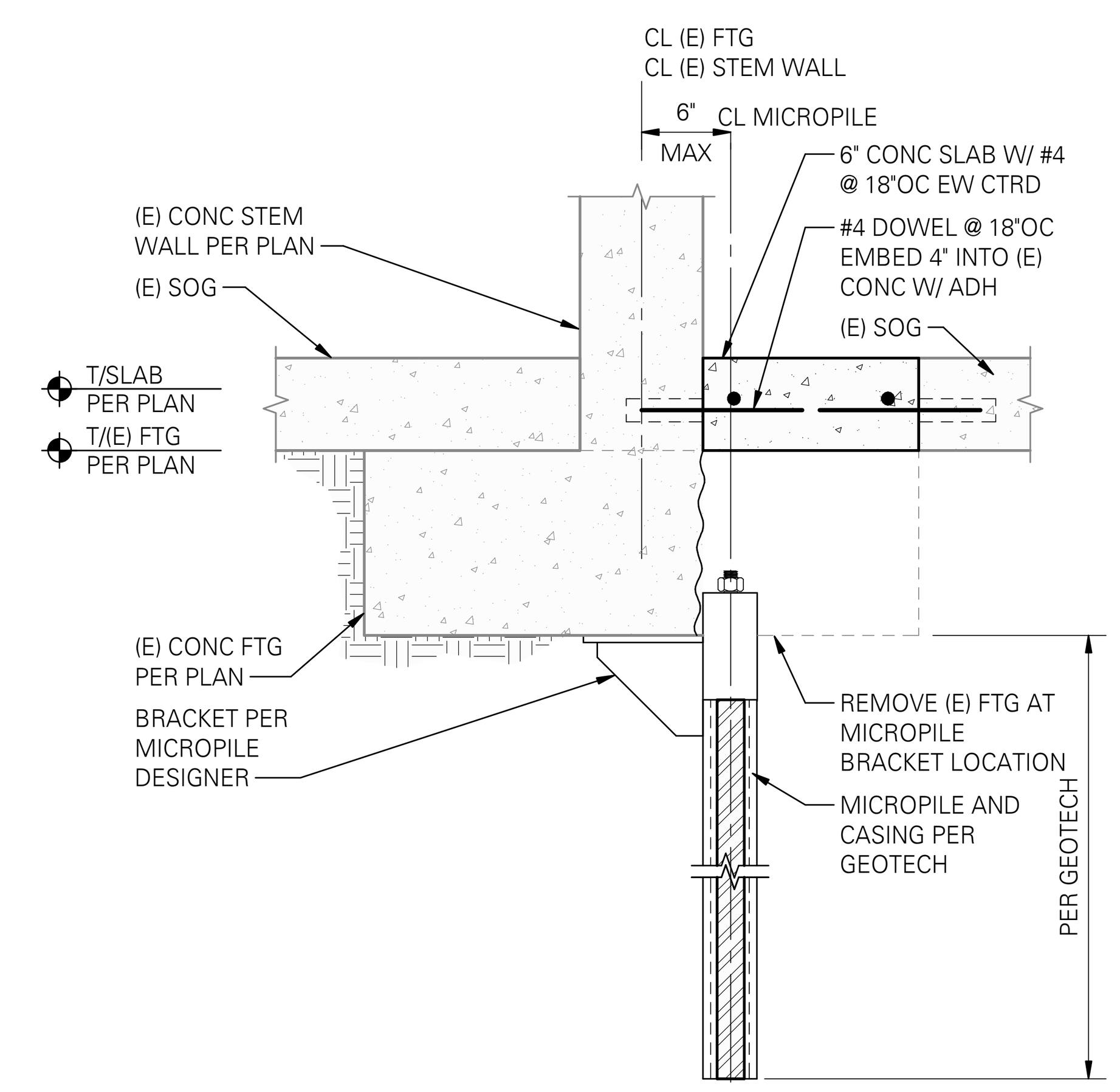
10 GRADE BEAM AT INTERIOR WALL

SCALE: 1" = 1'-0"



11 CONCRETE SHEAR WALL TO EXISTING CONCRETE WALL CONNECTION

SCALE: 1" = 1'-0"



12 MICROPILE AT EXISTING INTERIOR CONCRETE WALL

SCALE: 1" = 1'-0"

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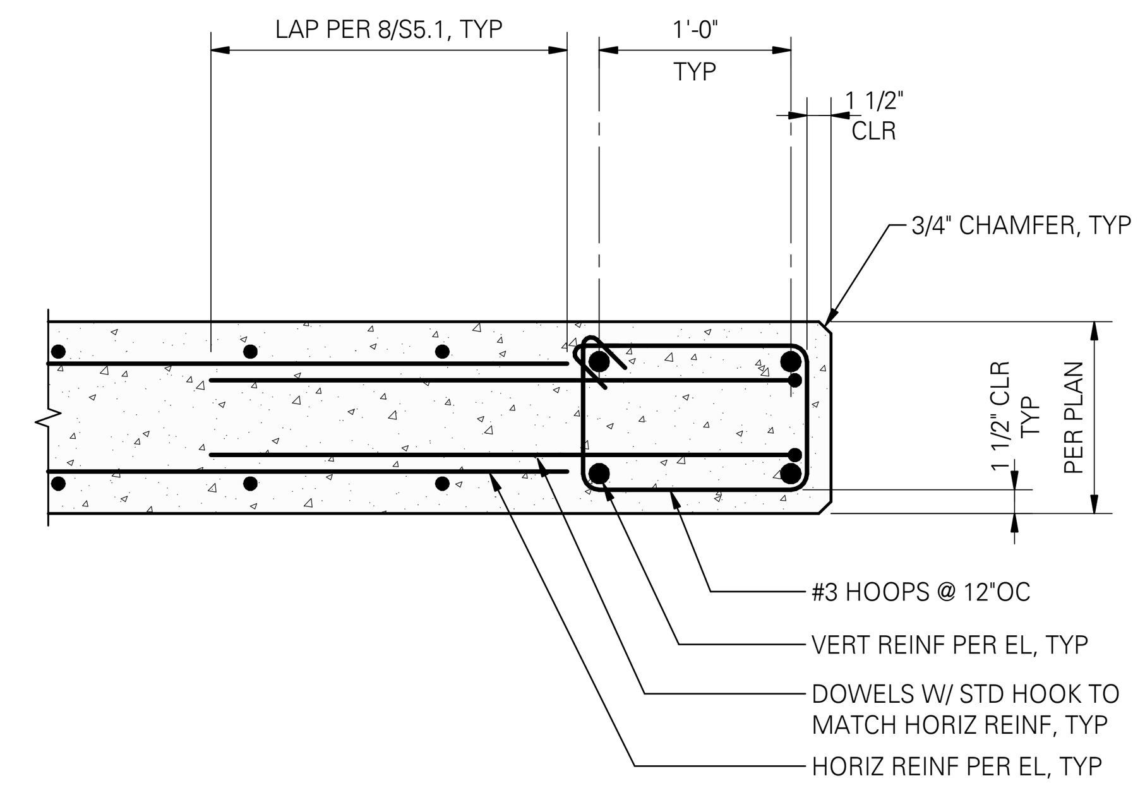
CONSTRUCTION

REVISIONS:
DATE DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
STRUCTURAL FOUNDATION DETAILS

S4.1

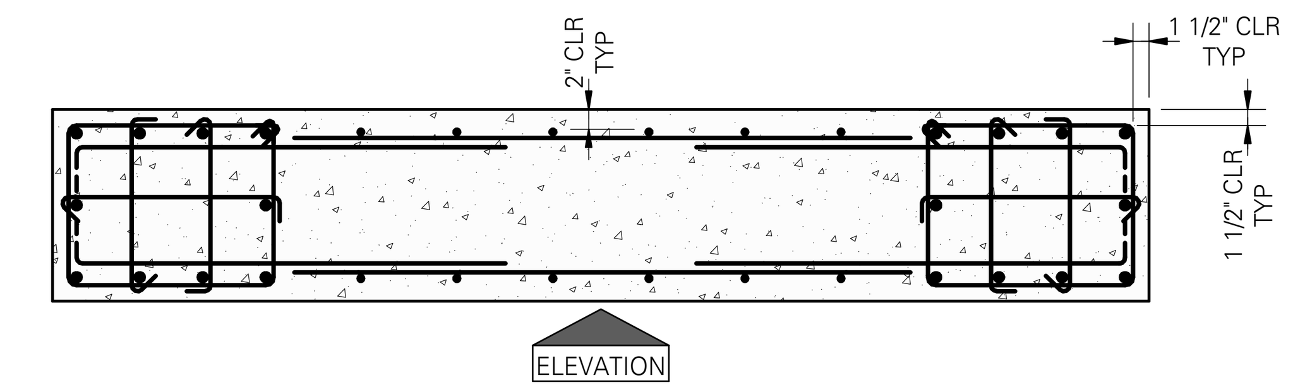


4 PLAN - CONCRETE SHEAR WALL BOUNDARY (6 BARS)

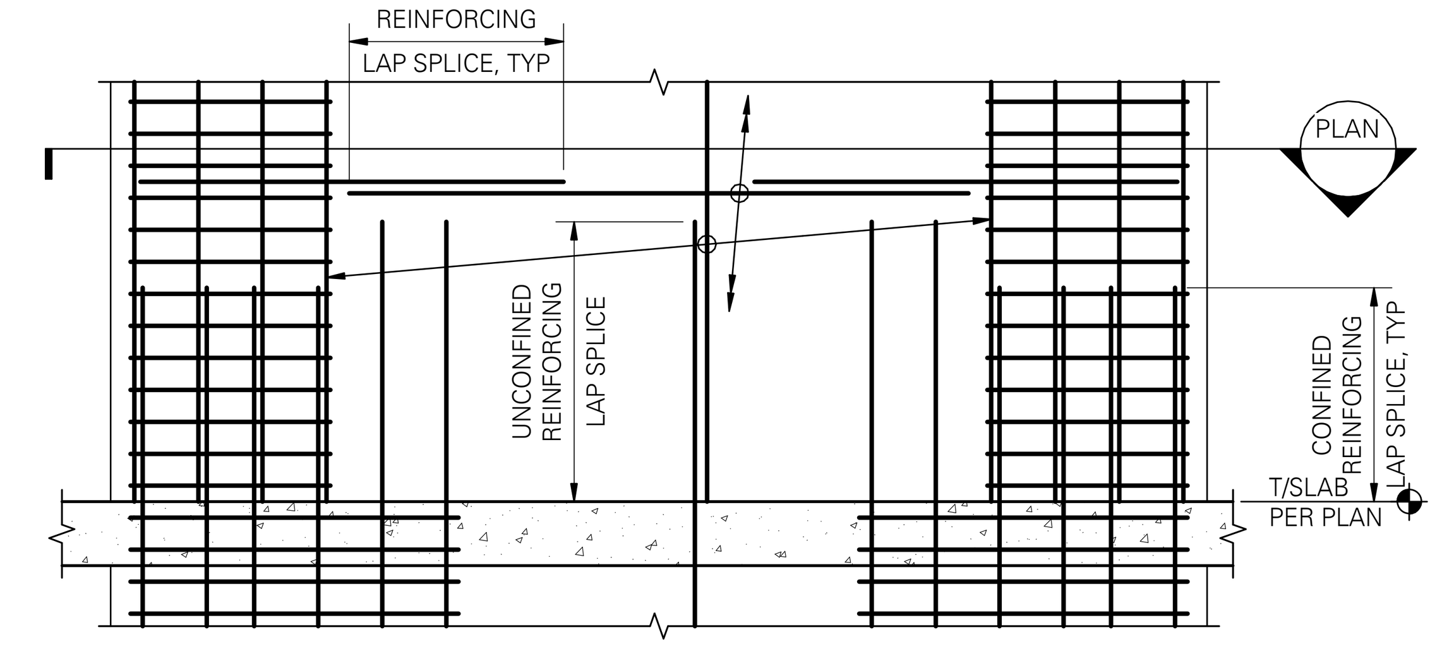
SCALE: 1" = 1'-0" (03421)

CONFINED REINFORCING								
f'c (psi)	#4	#5	#6	#7	#8	#9	#10	#11
6000	16	19	23	34	38	43	49	59

UNCONFINED REINFORCING								
f'c (psi)	#4	#5	#6	#7	#8	#9	#10	#11
6000	16	20	24	35	40	49	61	73



PLAN - TYPICAL SHEAR WALL



ELEVATION

NOTES:
1. ALL TABULATED VALUES ARE IN INCHES.

8 SHEAR WALL REINFORCING LAP SPLICE SCHEDULE

SCALE: 1/2" = 1'-0" (01404)

FOR PERMIT
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CIVIL / STRUCTURAL

STRUCTURAL REGISTERED PROFESSIONAL ENGINEER 74858PE OREGON MARCH 23, 2011 SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 22.22.2
EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID
CITY OF COOS BAY
365 D ST, COOS BAY, OR 97420

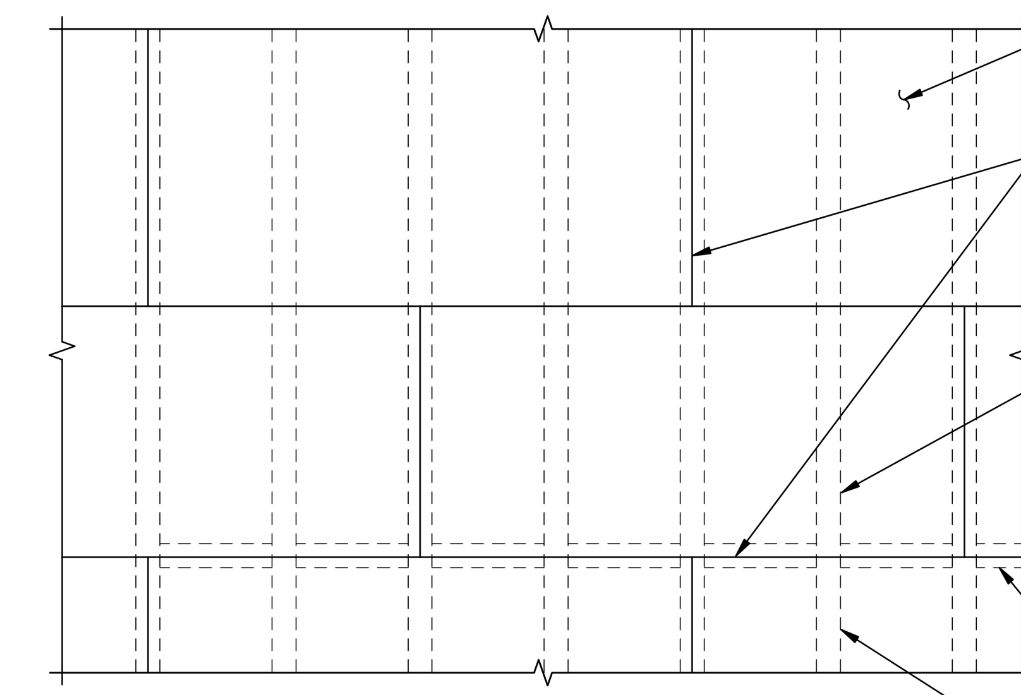
CONSTRUCTION

REVISIONS:
\# DATE DESCRIPTION

DATE: SEPTEMBER 2025

SHEET TITLE:
FOUNDATION DETAILS

S4.2

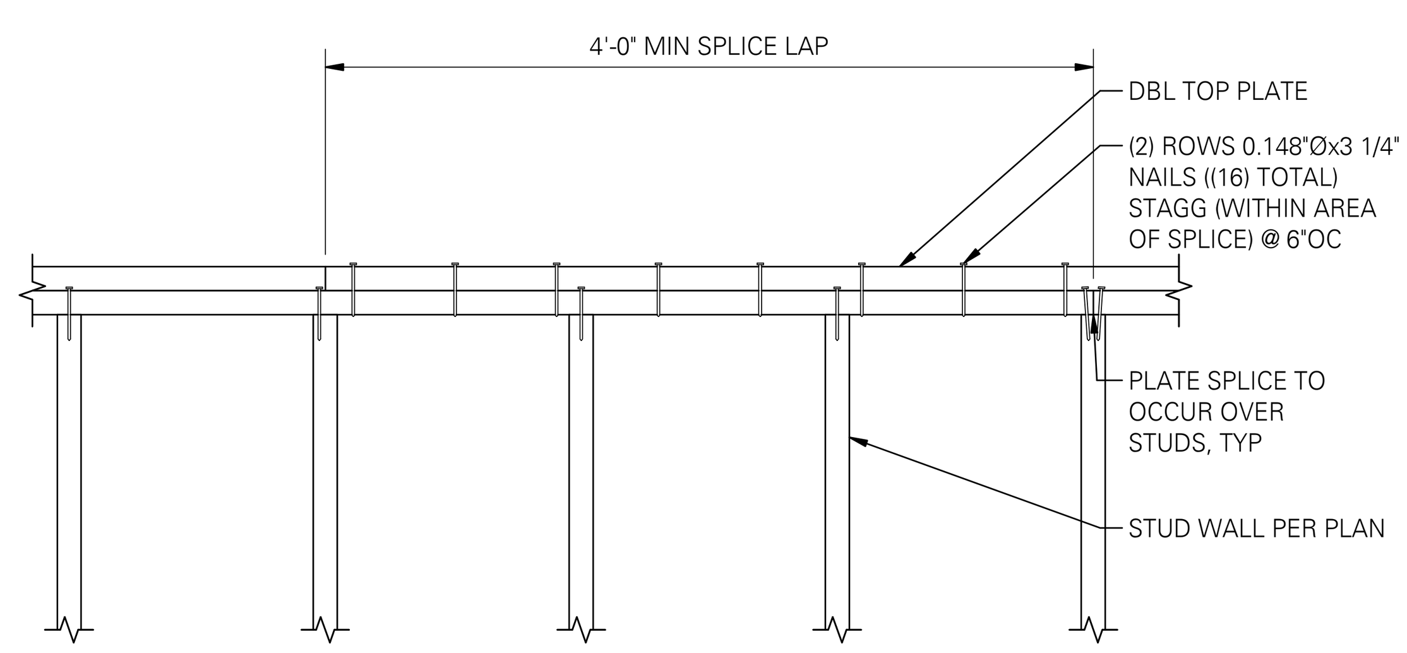


NOTES:

1. MINIMUM EDGE DISTANCE FOR NAILS SHALL BE 3/8".
2. MINIMUM SHEATHING SHEET SIZE SHALL BE 24"x48".
3. NAILS SHALL NOT BE OVERDRIVEN.
4. NAILS SHALL BE COMMON WIRE TYPE OR APPROVED EQUAL.
5. ADDITIONAL INFORMATION PER STRUCTURAL GENERAL NOTES.

4 PLAN - TYPICAL FLOOR AND ROOF SHEATHING ATTACHMENT

SCALE: 1" = 1'-0" (06230)

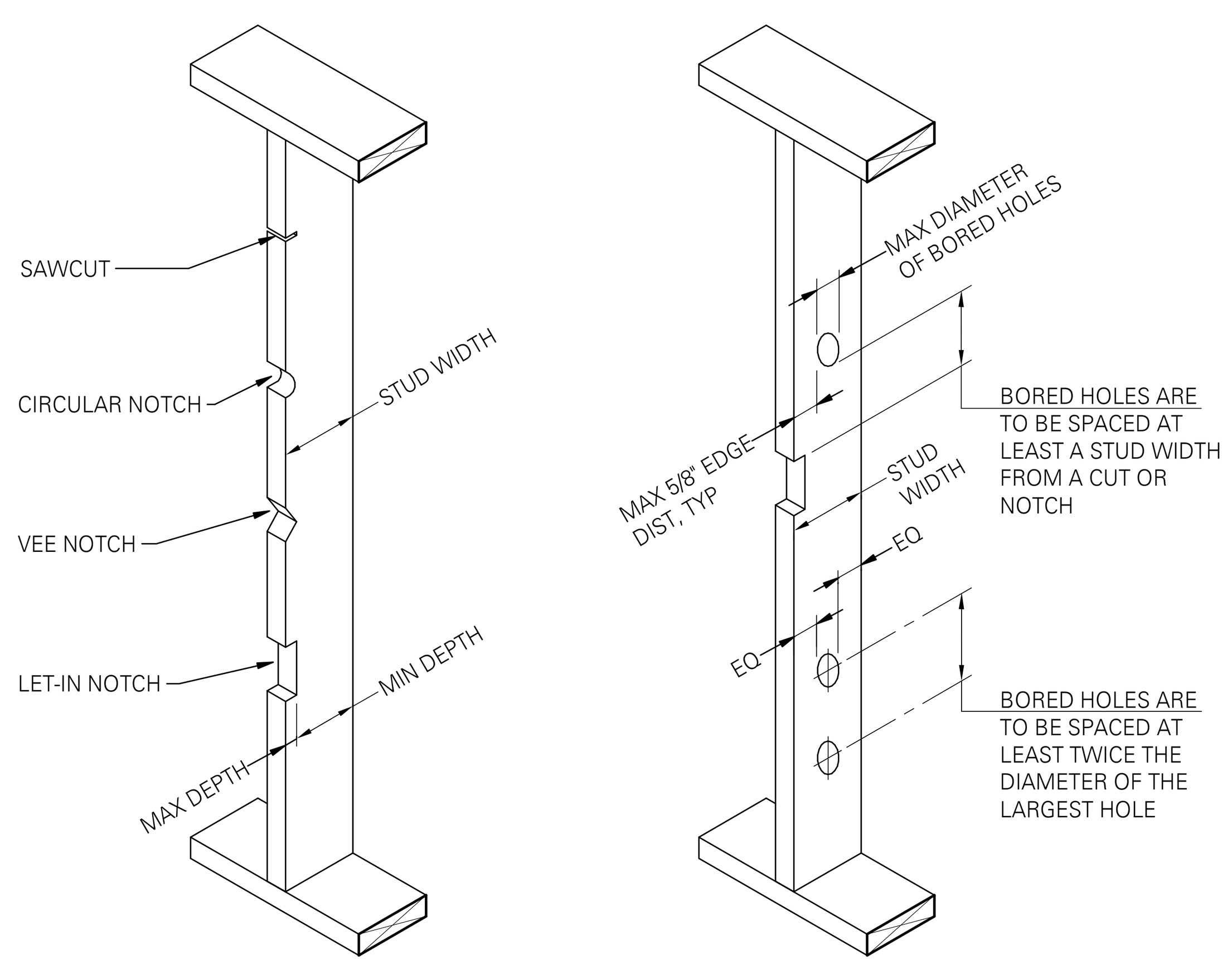


NOTE:

FLOOR/ROOF JOISTS NOT SHOWN FOR CLARITY.

6 TYPICAL PLATE SPLICE DETAIL

SCALE: 1" = 1'-0" (06904)



BEARING WALL STUDS		
STUD SIZE	MAX DEPTH OF EDGE CUT OR NOTCH	MIN DEPTH REMAINING AFTER CUT OR NOTCH
2x4	7/8"	2 5/8"
2x6	1 3/8"	4 1/8"

BEARING WALL STUDS		
STUD SIZE	MAX DIAMETER OF BORED HOLE	MIN DEPTH REMAINING AFTER BORED HOLE
2x4	1 3/8"	5/8" EA SIDE OF HOLE
2x6	2 3/16"	5/8" EA SIDE OF HOLE

NOTE:
STUDS MAY NOT BE BORED IN EXCESS OF 40% OF THE STUD, IF STUDS ARE DOUBLED, BORINGS MAY BE INCREASED TO 60% OF STUD WIDTH PROVIDED NOT MORE THAN (2) SUCCESSIVE STUDS ARE BORED. BORINGS SHALL NOT BE MADE AT THE SAME SECTION WHERE CUT OR NOTCH HAS BEEN MADE.

NON-BEARING WALL STUDS		
STUD SIZE	MAX DEPTH OF EDGE CUT OR NOTCH	MIN DEPTH REMAINING AFTER CUT OR NOTCH
2x4	1 3/8"	2 1/8"
2x6	2 3/16"	3 3/8"

NON-BEARING WALL STUDS		
STUD SIZE	MAX DIAMETER OF BORED HOLE	MIN DEPTH REMAINING AFTER BORED HOLE
2x4	2 1/16"	5/8" EA SIDE OF HOLE
2x6	3 1/4"	5/8" EA SIDE OF HOLE

NOTE:
STUDS MAY NOT BE BORED IN EXCESS OF 60% OF THE STUD. BORINGS SHALL NOT BE MADE AT THE SAME SECTION WHERE CUT OR NOTCH HAS BEEN MADE.

CUTTING AND NOTCHING WOOD STUDS

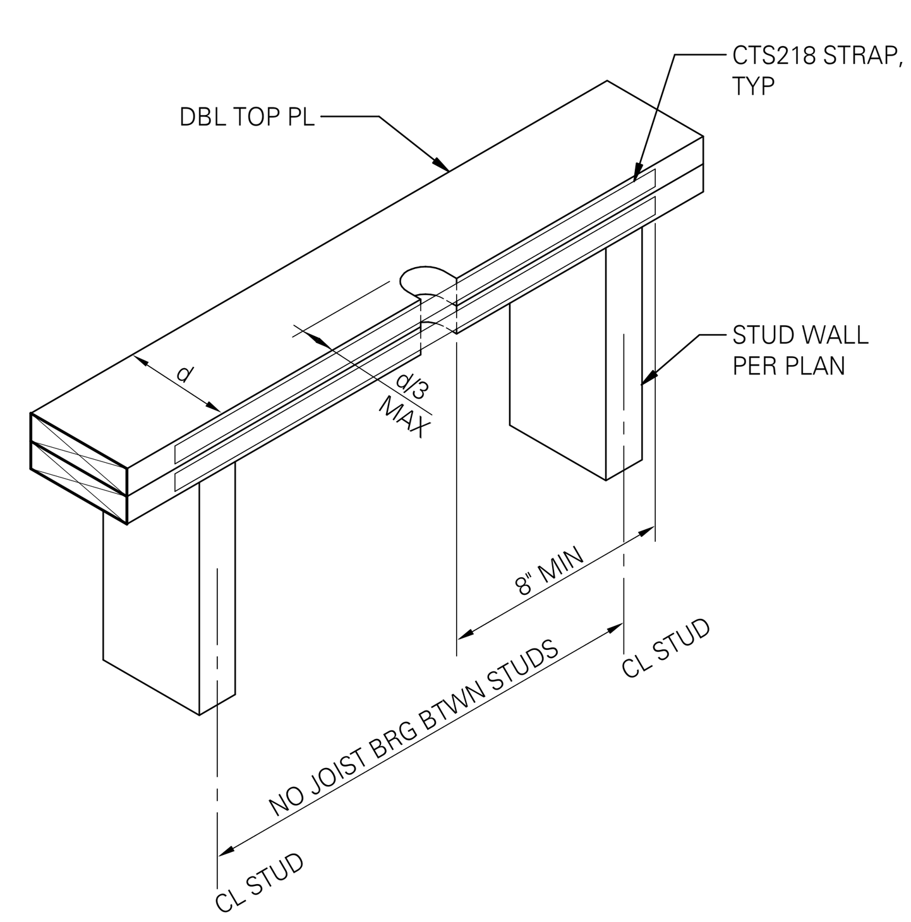
NOTE:
DO NOT NOTCH MORE THAN THREE ADJACENT STUDS WITHOUT REVIEW BY ENGINEER.

BORED HOLES IN WOOD STUDS

NOTE:
BORED HOLE NOT PERMITTED IN MORE THAN THREE ADJACENT STUDS WITHOUT REVIEW BY ENGINEER.

9 TYPICAL HOLES & NOTCHES IN WOOD STUDS

SCALE: 1" = 1'-0" (06908)



NOTE:

TYPICAL HOLES AND NOTCHES IN WOOD STUDS PER 9/S5.1.

10 TYPICAL NOTCHES AT TOP PLATES

SCALE: 1" = 1'-0" (06908A)

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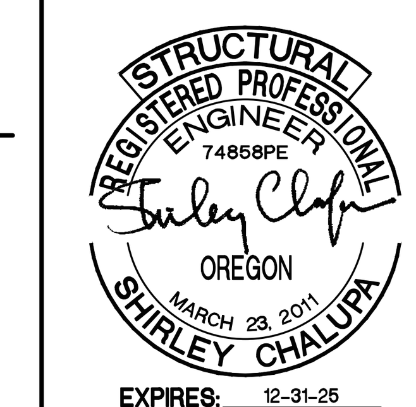

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EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

CITY OF COOS BAY
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CONSTRUCTION

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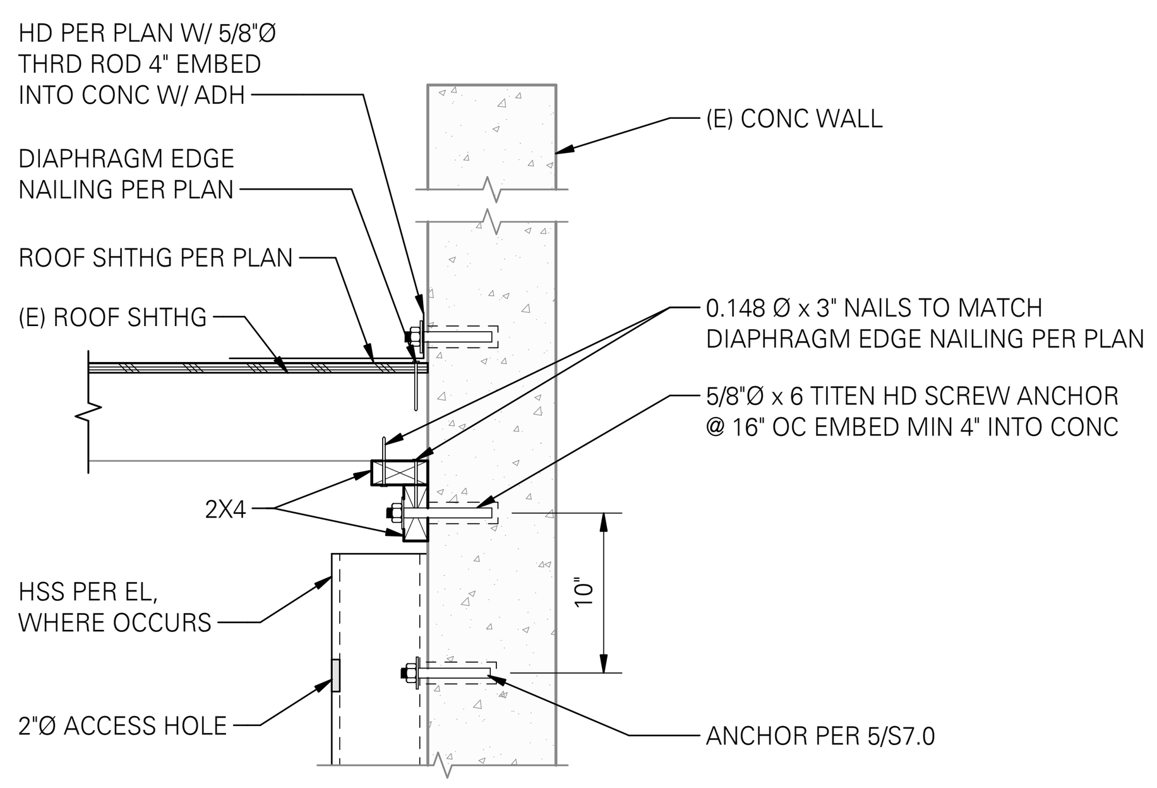
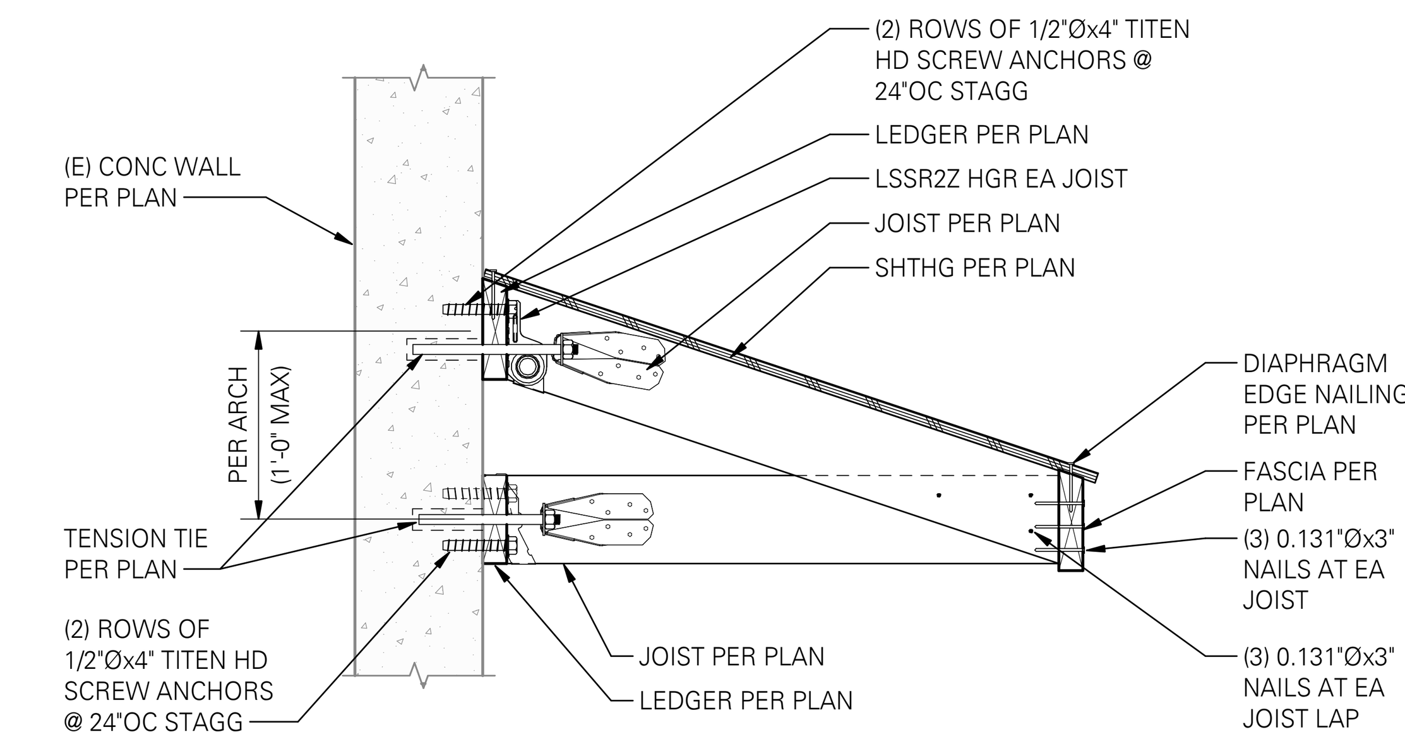
STRUCTURAL FRAMING DETAILS

S5.1



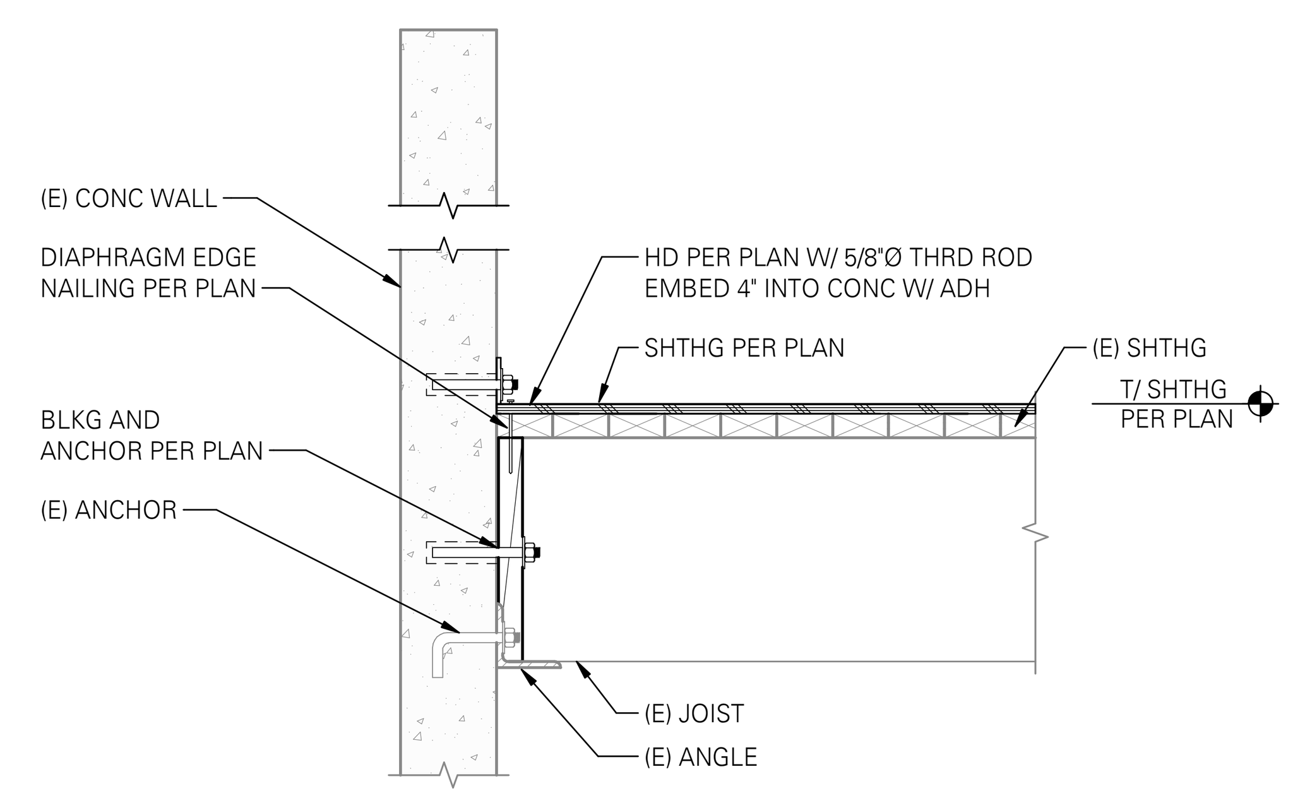
1 OUT OF PLANE CONNECTION AT EAST WALL

SCALE: 1" = 1'-0"



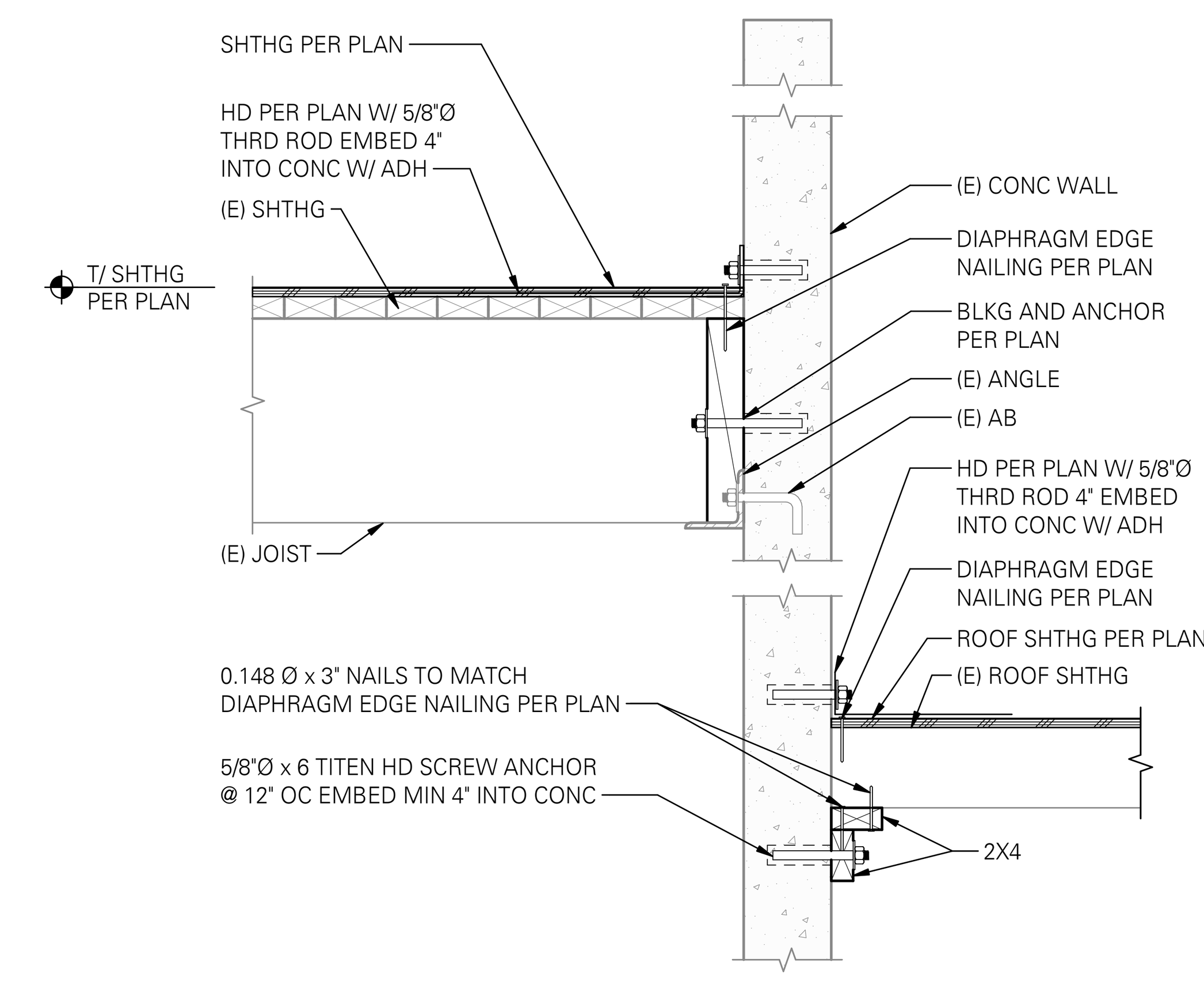
4 CANOPY CONNECTION TO EXTERIOR CONCRETE

SCALE: 1" = 1'-0"



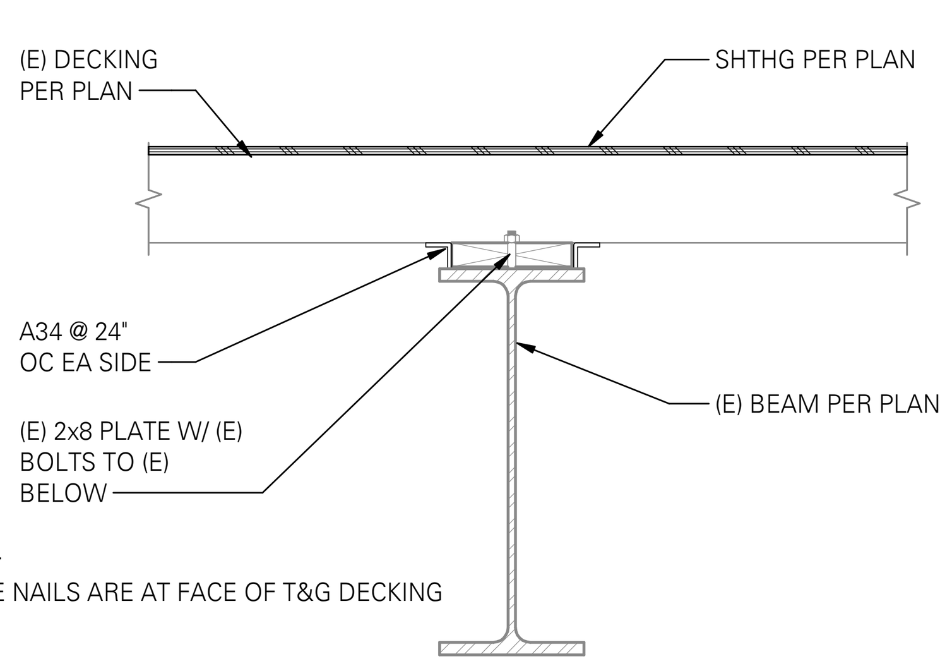
7 OUT OF PLANE CONNECTION AT EXTERIOR CONCRETE WALL

SCALE: 1" = 1'-0"



8 OUT OF PLANE CONNECTION AT INTERIOR CONCRETE WALL

SCALE: 1" = 1'-0"

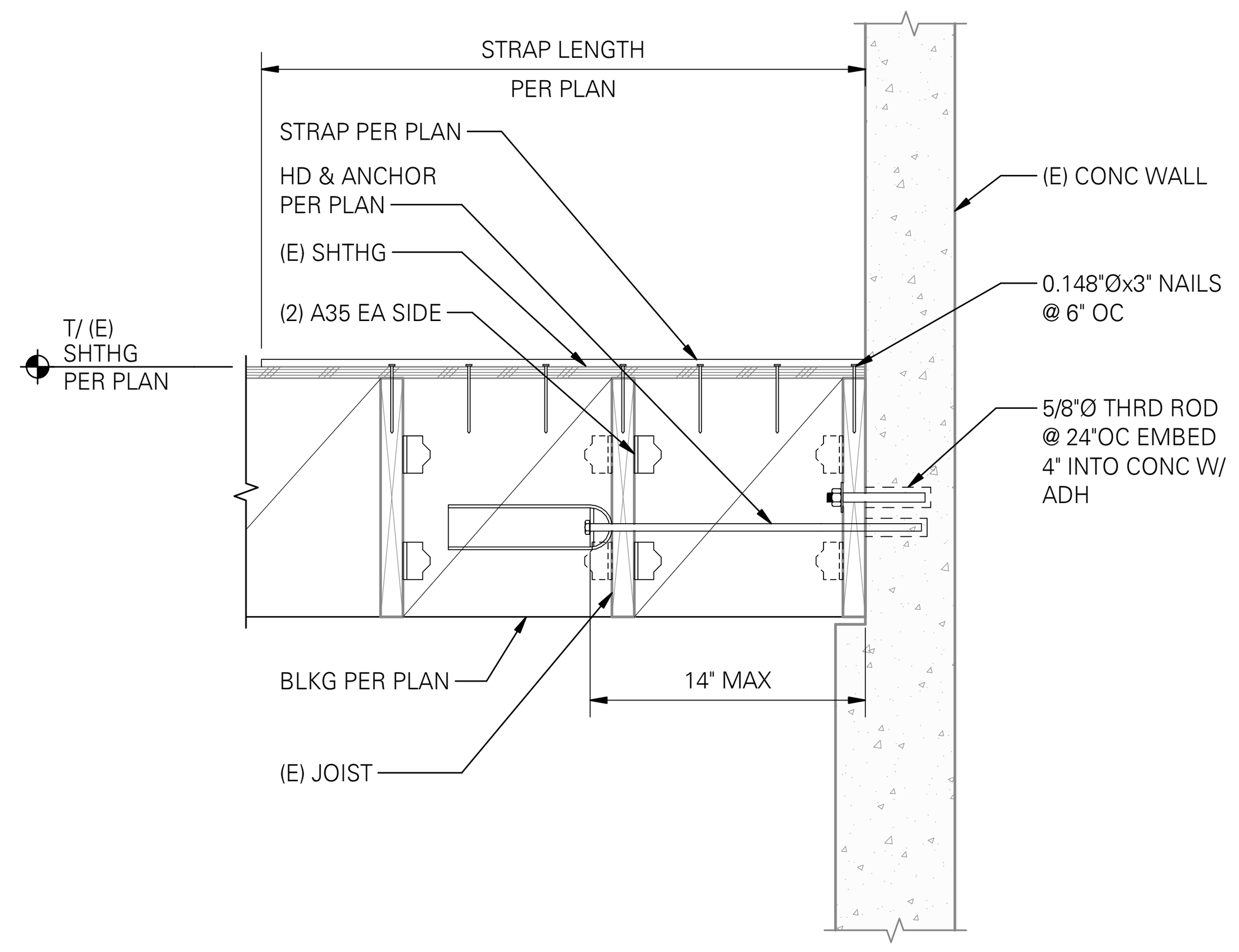


NOTE:

ENSURE NAILS ARE AT FACE OF T&G DECKING

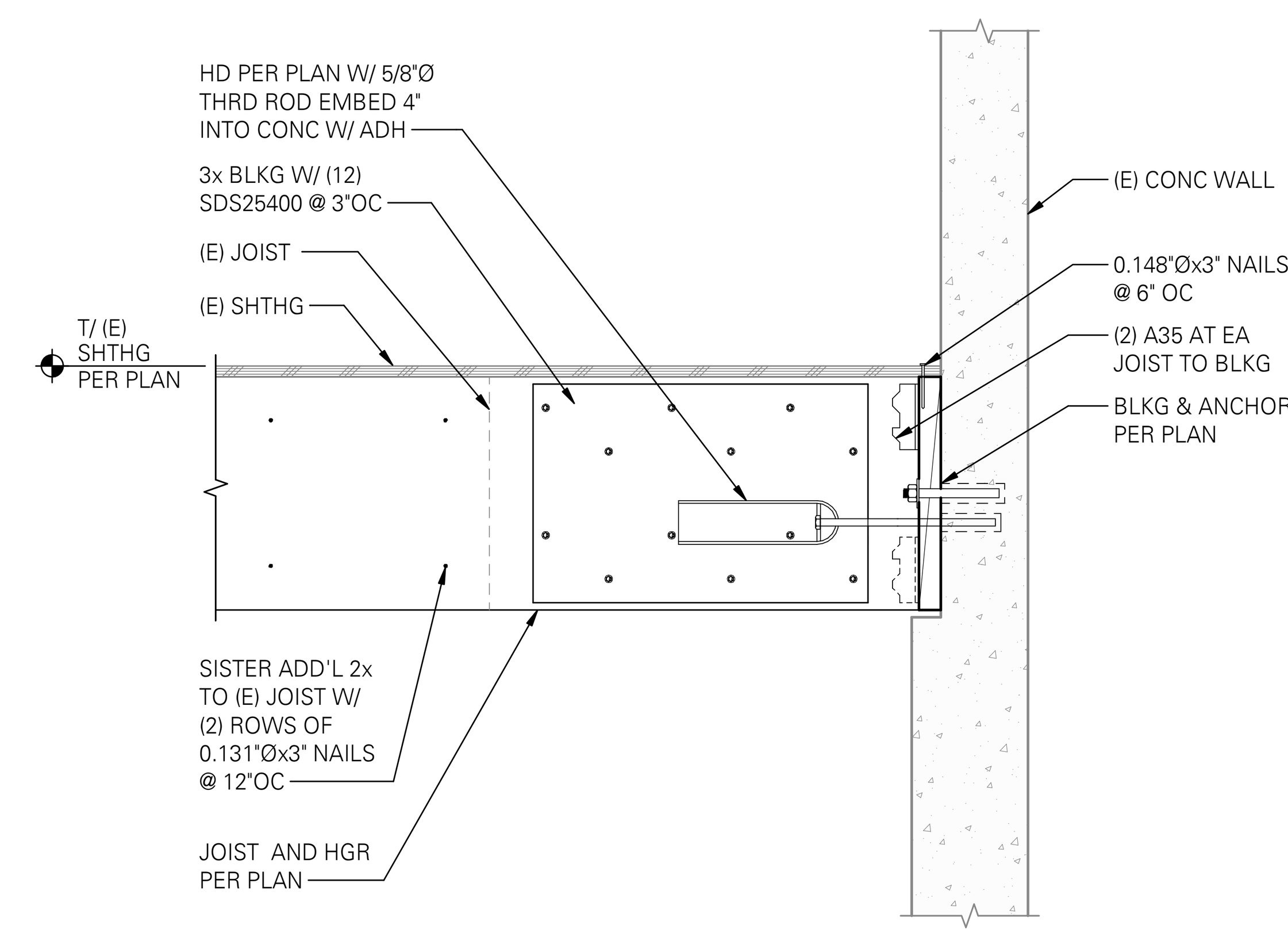
9 (E) STEEL BEAM PEPENDICULAR TO ROOF DECKING

SCALE: 1" = 1'-0" (06021M)



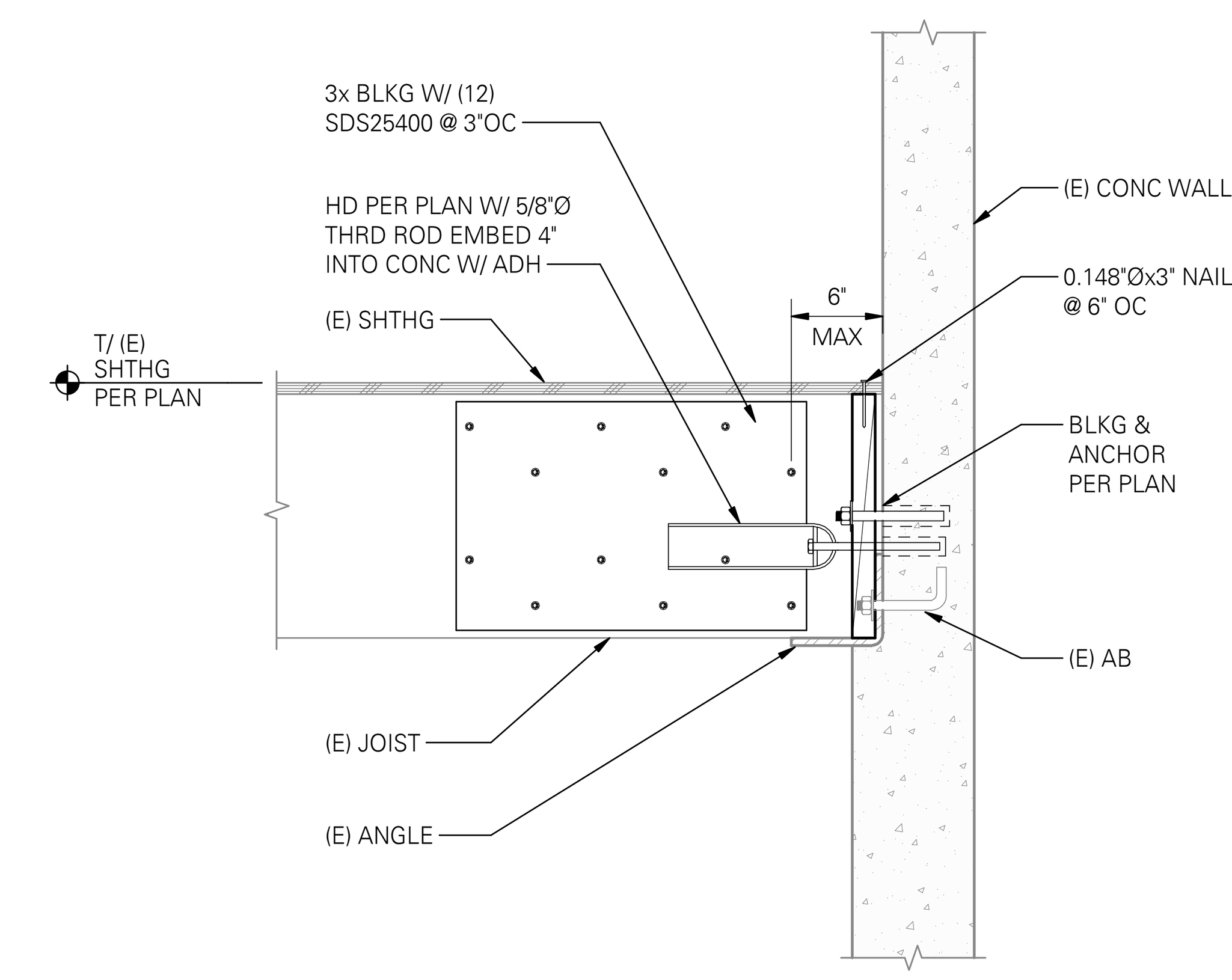
10 SECOND FLOOR OUT OF PLANE CONNECTION AT JOIST PARALLEL TO CONCRETE

SCALE: 1" = 1'-0"



11 SECOND FLOOR OUT OF PLANE CONNECTION AT JOIST PERPENDICULAR TO CONCRETE

SCALE: 1" = 1'-0"



12 SECOND FLOOR OUT OF PLANE CONNECTION AT JOIST PERPENDICULAR TO CONCRETE

SCALE: 1" = 1'-0"

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EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE REBID

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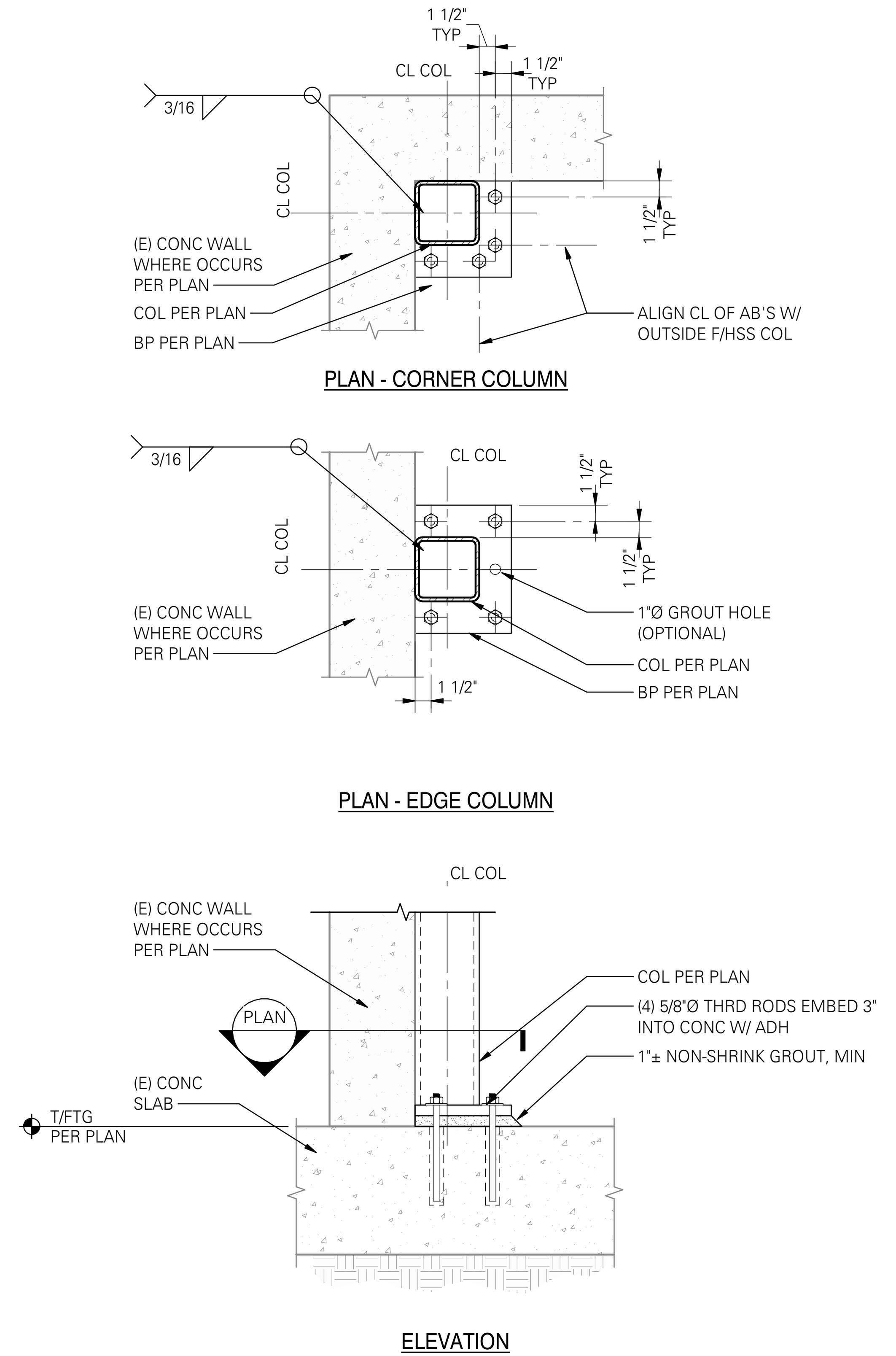
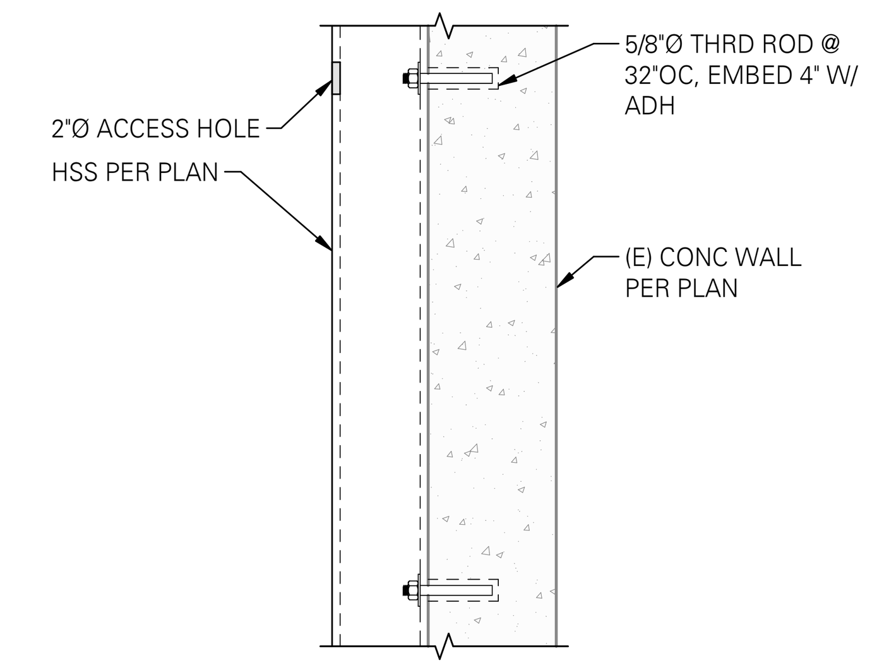
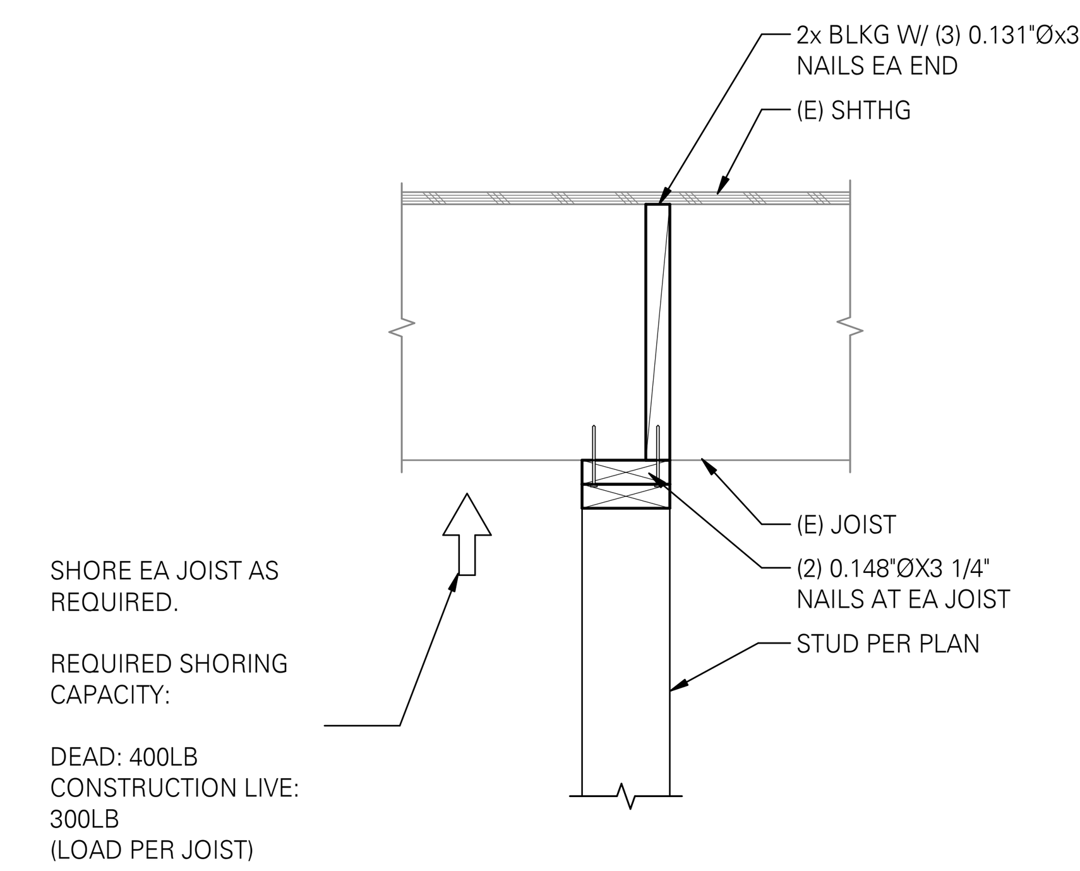
S6.0



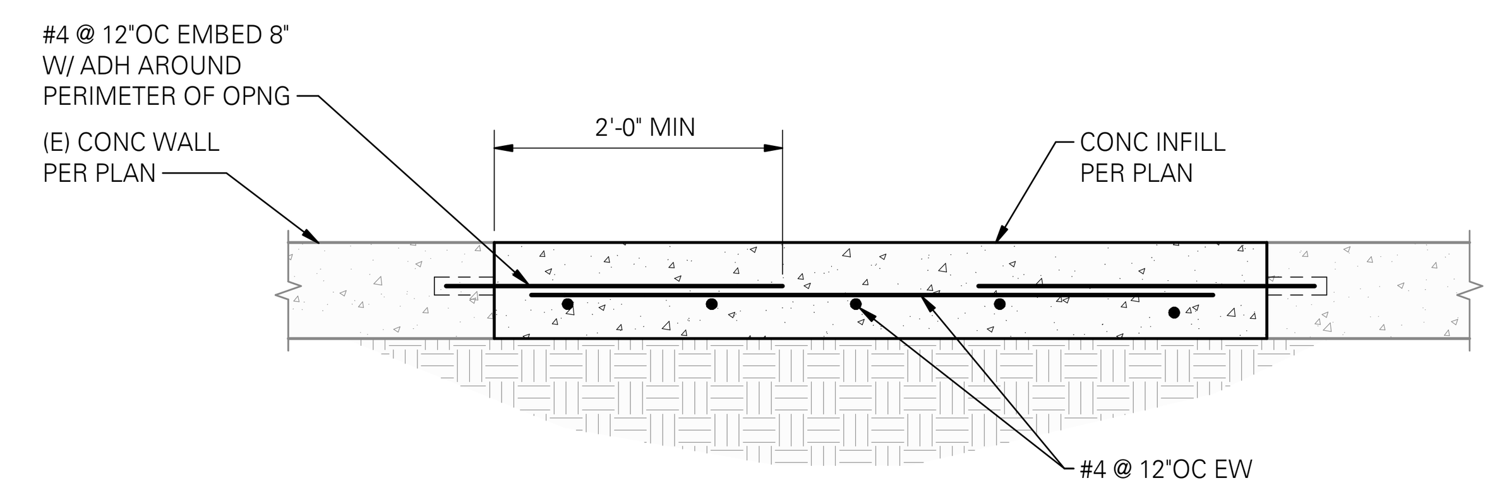
5 **TYPICAL BASEPLATE TO FOUNDATION CONNECTION - HSS COLUMN**
SCALE: 1" = 1'-0" (05030)



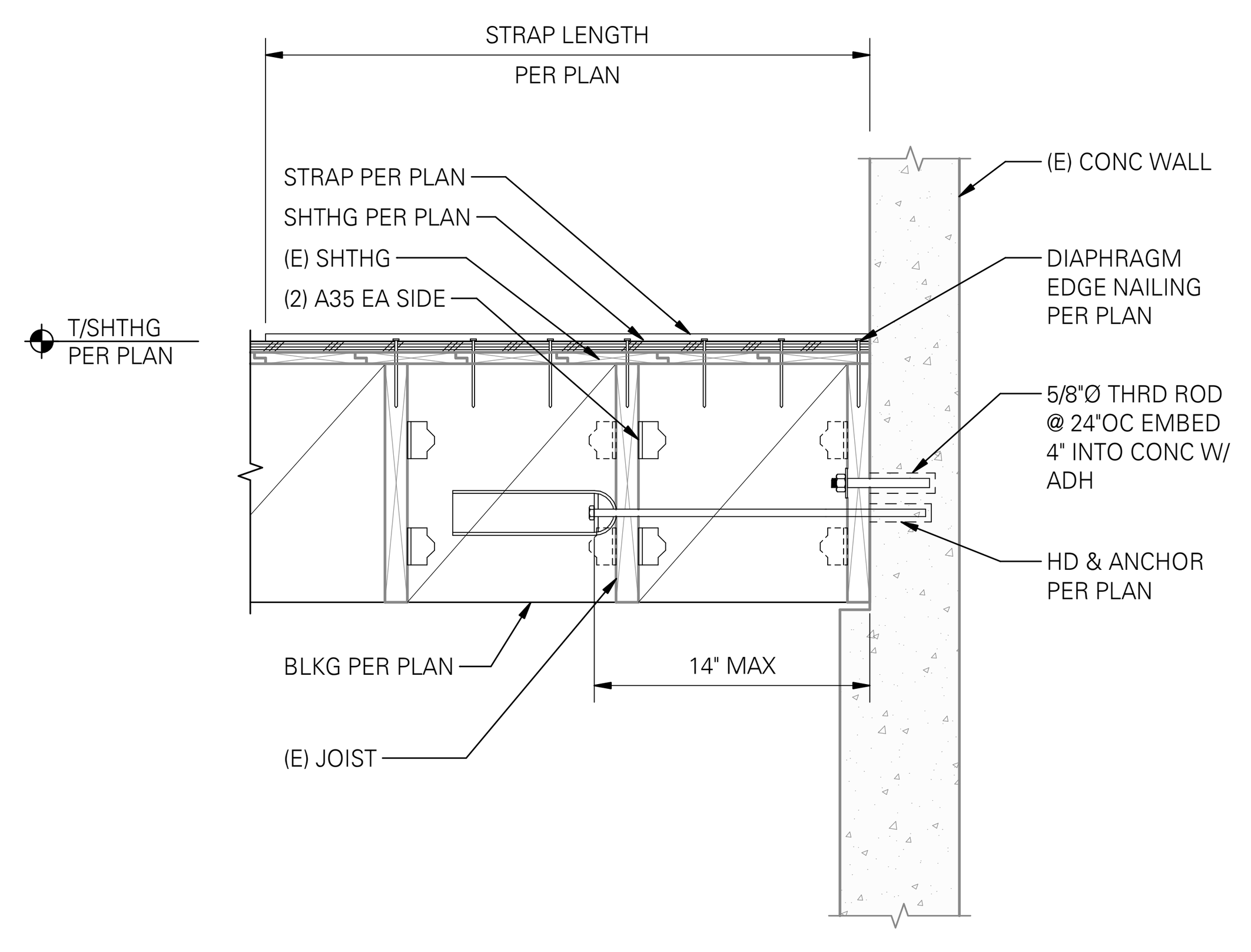
2 **HSS STONGBACK TO EXISTING CONCRETE**
SCALE: 1" = 1'-0" (06909)



4 **EXISTING BEARING WALL SHORING**
SCALE: 1" = 1'-0"



8 **EXISTING CONCRETE SLAB INFILL**
SCALE: 3/4" = 1'-0"



11 **OUT OF PLANE ANCHORAGE PARALLEL TO EXISTING CONCRETE WALL**
SCALE: 1" = 1'-0"

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STRUCTURAL FRAMING DETAILS

S7.0

ELECTRICAL SYMBOL SCHEDULE

SYMBOLS	ONELINE DIAGRAM	NOTES
	MOLDED CASE CIRCUIT BREAKER	
	TRANSFORMER	
	CURRENT TRANSFORMER(S)	
M KW	METER, TYPE AS NOTED	
	GROUND	
N	NEUTRAL BUS	
2	MOTOR WITH MOTOR NUMBER (SEE EQUIPMENT SCHEDULE)	
D	COMBINATION FIRE SMOKE DAMPER	2
XX X	EQUIPMENT NUMBER (SEE EQUIPMENT SCHEDULE)	
	NON-FUSED DISCONNECT SWITCH	
	FUSED DISCONNECT SWITCH (FUSES SIZED PER EQUIPMENT MANUFACTURERS RECOMMENDATIONS UNO.)	
	COMBINATION MOTOR STARTER / FUSED DISCONNECT SWITCH	
	SUB-DISTRIBUTION PANELBOARD OR SWITCHBOARD	
	BRANCH CIRCUIT PANELBOARD	
	MISCELLANEOUS PANEL AS NOTED	
	MAIN DISTRIBUTION PANELBOARD	
T	TRANSFORMER	

SYMBOLS	RACEWAYS	NOTES
	BRANCH CIRCUIT INSTALLED CONCEALED FROM FINISH SPACES . PROVIDE GROUND CONDUCTOR AS INDICATED IN PANEL SCHEDULE . GROUND CONDUCTOR NOT INCLUDED IN HASH MARK INDICATION .	
	BRANCH CIRCUIT INSTALLED IN OR BELOW FLOOR . PROVIDE GROUND CONDUCTOR AS INDICATED IN PANEL SCHEDULE . GROUND CONDUCTOR NOT INCLUDED IN HASH MARK INDICATION .	
LA-1,3,5	BRANCH CIRCUIT HOME RUN TO PANEL , HASH MARKS INDICATES NUMBER OF CONDUCTORS . PROVIDE GROUND CONDUCTOR AS INDICATED IN PANEL SCHEDULE . GROUND CONDUCTOR NOT INCLUDED IN HASH MARK INDICATION .	
	LOW VOLTAGE EMPTY CONDUIT WITH PULL STRING - ¾" UNO	
PB	PULL BOX , 6" x 6" x 4" UNLESS NOTED OTHERWISE	
J J	JUNCTION BOX , 4" SQUARE UNLESS OTHERWISE NOTED	
	4" CONDUIT SLEEVE WITH BUSHINGS AT BOTH ENDS . LOCATE AT 6" ABOVE ACCESSIBLE CEILING . FIRESTOP WITH UL APPROVED SYSTEM .	
	CONDUIT STUB-OUT, CAP & MARK WITH APPROVED MARKER	
	CONDUIT , UP	
	CONDUIT , DOWN	

SYMBOLS	RECEPTACLES	NOTES
•	WHEN ADDED TO A SYMBOL , INDICATES OUTLET MOUNTED WITH BOTTOM OF OUTLET AT 2" ABOVE COUNTER TOP OR BACK SPLASH UNO.	
	DUPLEX CONVENIENCE OUTLET	+ 18"
	GFI DUPLEX CONVENIENCE OUTLET	+ 18"
	DUPLEX OUTLET WITH USB OUTLETS	+ 18"
	DOUBLE DUPLEX CONVENIENCE OUTLET	+ 18"
	SINGLE PHASE SPECIAL PURPOSE OUTLETS, AS NOTED	+ 18" UNO
	THREE PHASE SPECIAL PURPOSE OUTLETS, AS NOTED	
	FLUSH FLOOR OUTLET AS SHOWN	

SYMBOLS	TELEPHONE / DATA	NOTES
•	WHEN ADDED TO SYMBOL , INDICATES OUTLET MOUNTED WITH BOTTOM OF OUTLET AT 2" ABOVE COUNTER TOP OR BACKSPLASH UNO	
	TELE./DATA. PROVIDE (2) CAT6 CABLES UNO	+ 18" 1
W	'W' ADDED TO SYMBOL INDICATES WALL MOUNTED	+ 60" 1
	FLUSH FLOOR OUTLET AS SHOWN	1
TTB	TELEPHONE TERMINAL BOARD , 8" HIGH (WIDTH AS SHOWN) , ¾" FIRE RESISTIVE PLYWOOD WITH # 6 CU GND	
WIFI	WIRELESS ACCESS PORT. PROVIDE (1) CAT6 CABLES	1
	TELE./DATA. PROVIDE (1) CAT6 CABLES UNO	+ 18" 1

NOTES

1. ALL SYMBOLS MAY NOT APPLY DIRECTLY TO THIS JOB.
2. ALL MOUNTING HEIGHTS SHOWN ARE TO CENTERLINE OF DEVICE.
3. ALL MOUNTING HEIGHTS ARE TYPICAL ON PLANS,

KEYED NOTES

1. PROVIDE 1" CONDUIT FROM OUTLET BOX TO ACCESSIBLE LOCATION ABOVE CEILING, UNLESS NOTED OTHERWISE. TERMINATE CONDUITS WITH BLUE INSULATED BOX CONNECTORS AND LABEL SYSTEM, SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION . ROUTE CAT6 or 6a CABLE(S) TO DATA RACK

SYMBOLS	LIGHT FIXTURES	NOTES
	WHEN ADDED TO LIGHT FIXTURE SYMBOL- INDICATES WALL OR BRACKET MOUNTED LIGHT FIXTURE	
FD 2. a	SURFACE OR PENDANT MOUNTED LIGHT FIXTURE OUTLET . (NUMBER INDICATES CIRCUIT , CAPITAL LETTER INDICATES FIXTURE TYPE , LOWER CASE LETTER INDICATES SWITCHING CONTROL , TYPICAL FOR ALL LIGHT FIXTURES)	
	RECESSED CEILING LIGHT FIXTURE	
	RECESSED WALL WASHER , UNSHADED SIDE INDICATES DIRECTION OF WALL WASHING	
	FLUORESCENT LIGHT FIXTURE	
	FLUORESCENT STRIP LIGHT FIXTURE	
	SINGLE FACE EXIT SIGN WITH NUMBER OF DIRECTIONAL ARROWS AS SHOWN , CEILING MOUNTED . SOLID QUADRANT INDICATES FACE.	

SYMBOLS	SWITCHES	NOTES
$	SINGLE POLE LIGHT SWITCH	+ 46"
$3	THREE WAY LIGHT SWITCH	+ 46"
$M	MOTOR RATED SWITCH	+ 46"
MC MW	OCCUPANCY SENSOR - C=CEILING W=WALL MOUNTED	
P	PHOTOELECTRIC SWITCH	

SYMBOLS	SECURITY	NOTES
	SECURITY CAMERA. PROVIDE J-BOX WITH CAT 6 CABLE	1
L	ELECTRONICALLY CONTROLLED LOCK	
	DOOR POSITION SWITCH	
M	MOTION DETECTOR (OMNI DIRECTIONAL)	
CR	CARDREADER	+ 44"
SK	KEYPAD	+ 44"

SYMBOLS	AUDIO / VISUAL	NOTES
S	CEILING SPEAKER	
S	WALL MOUNTED SPEAKER	+ 80"
S	WALL MOUNTED SPEAKER HORN	+ 80"
TV	TELEVISION (VIDEO) OUTLET	+ 18"
IR	INTERCOM REQUEST STATION (SPEAKER & PUSH BUTTON)	+ 44"

SYMBOLS	FIRE ALARM	NOTES
M	MANUAL PULL STATION	+ 44"
V	COMBINATION VISUAL / AUDIBLE ALARM	+ 80" AFTB
V	VISUAL STROBE ALARM	+ 80" AFTB
P	PHOTOELECTRIC SMOKE DETECTOR (CEILING NOUNTED UNO)	
I	IONIZATION SMOKE DETECTOR (CEILING MOUNTED UNO)	
D	MAGNETIC DOOR HOLDER	
H	HEAT DETECTOR (CEILING MOUNTED, 135° UNO)	

SYMBOLS	ABBREVIATIONS	NOTES
AIC	AMPERE INTERRUPTING CAPACITY	
AMP	AMPERE	
C	CONDUIT	
EC	EMPTY CONDUIT (WITH PULL-IN LINE)	
ELEC	ELECTRICAL	
FAAP	FIRE ALARM ANNUNCIATOR PANEL	
FACP	FIRE ALARM CONTROL PANEL	
G, GND	GROUND	
GEN	GENERATOR	
GFI	GROUND FAULT CIRCUIT INTERRUPTER TYPE	
HP	HORSEPOWER	
IG	ISOLATED GROUND	
MECH	MECHANICAL	
MFGR	MANUFACTURER	
NEC	NATIONAL ELECTRIC CODE	
NL	NIGHT LIGHT	24 HOUR 'ON'
OFCI	OWNER FURNISHED CONTRACTOR INSTALLED	
OFOI	OWNER FURNISHED OWNER INSTALLED	
PB	PULL BOX	
ϕ PH	PHASE	
PNL	PANEL	
PWR	POWER	
SYS	SYSTEM	
T	TELEPHONE	
TTB	TELEPHONE TERMINAL BOARD	
TYP	TYPICAL	
UNO	UNLESS NOTED OTHERWISE	
V	VOLT	
VP	VANDAL PROOF	
W	WATT	
WP	WEATHERPROOF TYPE	

PROJECT NOTES

1. THIS PROJECT IS A SEISMIC UPGRADE TO AN EXISTING FIRE STATION. FIELD COORDINATE REQUIREMENTS TO ACCOMMODATE INSTALLATION OF NEW SEISMIC SUPPORTS AND TO RE-ENERGIZE EXISTING EQUIPMENT-TO-REMAIN. IT IS THE ELECTRICIAN'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH STRUCTURAL AND ARCHITECTURAL PLANS TO FULLY UNDERSTAND THE REQUIREMENTS OF THIS PROJECT.
2. 400 AMP SERVICE: REPLACE EXISTING SYSTEM OF PANELS AND DISCONNECTS WITH NEW 400 AMP SERVICE (SEE NEW 1-LINE). PROVIDE SERVICE ENTRANCE DISCONNECT TO FEED ATS AND PANEL 'A'. PROVIDE NEW 150 AMP PANEL AND FEEDER TO UPSTAIRS DORMITORY AREA. FIELD COORDINATE ALL OTHER BREAKER REQUIREMENTS WITH EXISTING EQUIPMENT TO REMAIN. PROVIDE TYPE WRITTEN PANEL SCHEDULE WITH PLASTIC COVER. PROVIDE NEW ENGRAVED PHYNOLIC PANEL LABELS WITH NAME, VOLTAGE AND AMP RATING.
3. WORK SHOWN ON PLAN IS BASED ON AVAILABLE INFORMATION AT THE TIME OF DESIGN. FIELD COORDINATE SEISMIC UPGRADE REQUIREMENTS WITH ARCHITECTURAL AND STRUCTURAL. CONTRACTOR IS TO FIELD VERIFY EXISTING CIRCUITING AND INSTALLATION.
4. CONTRACTOR SHALL COORDINATE AND PERFORM NECESSARY ELECTRICAL DEMOLITION WORK ASSOCIATED WITH ALL ITEMS AND EQUIPMENT TO BE REMOVED.
5. CONFIRM THAT ALL EXISTING DEVICES AND EQUIPMENT PLANNED FOR REUSE ARE IN GOOD OPERATING CONDITION. UNSUITABLE ITEMS SHALL NOT BE REUSED. RETURN ALL OTHER ITEMS SUITABLE FOR REUSE TO OWNER.
6. WIRING WHICH SERVES USABLE EXISTING OUTLETS SHALL BE REROUTED AND RESTORED CLEAR OF CONSTRUCTION. MAINTAIN ELECTRICAL CONTINUITY OF EXISTING SYSTEM. REPAIR AND RECONDITION ASSOCIATED SURFACES TO MATCH ADJACENT SURFACES. VERIFY EXACT LOCATIONS IN THE FIELD.

PANEL ' A '

FAULT CURRENT = 12,136
400 AMP MAIN BREAKER 120 / 240 VOLTS 1-PHASE, 3-WIRE
FEEDER SIZE COPPER: 2 SETS - 2" C, 3 #3/0 PH, #1/0 GRD FLUSH MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	25%
CONNECTED VA	2303	42338	9344	0	0	4500	1200	31454	28231	59685	262	74606 VA	328
DIVERSITY FACTOR	125%	62%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	2879	26169	9344	0	0	4500	1200	22799	21292	44092	190	55115 VA	237

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR		PH		BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1	P	PANEL B	10177					200	2	A	2	150					4800	PANEL C	P	2
3	P	PANEL B	9123							B							7419	PANEL C	P	4
5		SPACE								A								SPACE		6
7		SPACE								B								SPACE		8
9	O	FIRE ALARM PANEL	1200		12	12	1/2	20	1	A	1	20	1/2	12	12		180	REC: OUTDOOR	R	10
11	R	REC: CONF RM / HALL	900		12	12	1/2	20	1	B	1	20	1/2	12	12		360	REC: W. APP BAY	R	12
13	R	REC: SW CONF RM	540		12	12	1/2	20	1	A	1	20	1/2	12	12		720	REC: N. APP BAY	R	14
15	R	REC: CONF RM TV	360		12	12	1/2	20	1	B	1	20	1/2	12	12		720	REC: E. APP BAY	R	16
17	R	REC: NW CONF / HALL	540		12	12	1/2	20	1	A	1	30	1/2	10	12	1	1656	WEST DOOR	M	18
19	R	REC: STORAGE 6	900		12	12	1/2	20	1	B	1	30	1/2	10	12	1	1656	CENTER DOOR	M	20
21	R	REC: DATA	360		12	12	1/2	20	1	A	1	30	1/2	10	12	1	1656	EAST DOOR	M	22
23	R	REC: DATA	360		12	12	1/2	20	1	B	1	20	1/2	12	12		180	REC: W. TRUCK	R	24
25	R	REC: TOILET 5	180		12	12	1/2	20	1	A	1	20	1/2	12	12		180	REC: C. TRUCK	R	26
27	R	REC: TOILET 4	180		12	12	1/2	20	1	B	1	20	1/2	12	12		180	REC: E. TRUCK	R	28
29		SPARE	0				1/2	20	1	A	1	20	1/2	12	12		360	REC: CEILING DROP	R	30
31		SPARE	0				1/2	20	1	B	1	20	1/2	12	12	1/2	1176	PROPANE UNIT HTR	M	32
33		SPARE	0				1/2	20	1	A	1	20	1/2	12	12		276	ATTIC REC & LTS	R	34
35		SPARE	0				1/2	20	1	B	1	20	1/2	12	12		880	LTS: LOWER ADMIN	L	36
37		SPARE	0				1/2	20	1	A	1	20	1/2	12	12		260	LTS: EXTERIOR	L	38
39		SPARE	0				1/2	20	1	B	1	20	1/2	12	12		704	LTS: APP BAY	L	40

A 5/27/2025 FED FROM PANEL 'ATS'
ALL CIRCUIT CONDUCTORS SIZED FOR COPPER 25.05 Schedules

PANEL ' B '

FAULT CURRENT = 10,908
200 AMP MAIN BREAKER 120 / 240 VOLTS 1-PHASE, 3-WIRE
FEEDER SIZE COPPER: 2" C, 3 #3/0 PH, #6 GRD FLUSH MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	25%
CONNECTED VA	0	13200	3200	0	0	4500	0	11050	9850	20900	92	26125 VA	115
DIVERSITY FACTOR	125%	88%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	0	11600	3200	0	0	4500	0	10177	9123	19300	85	24125 VA	106

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR		PH		BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1	R	50-AMP RECEPTACLE	4800		6	10	3/4	50	2	A	1	20	1/2	12	12		1200	GEN: BATTERY	R	2
3	R		4800							B	1	20	1/2	12	12		1200	GEN: HEATER	R	4
5	H	WATER HEATER	2250		10	10	1/2	30	2	A	1	20	1/2	12	12		1200	GEN: ANNUNCIATOR	R	6
7	H	WATER HEATER	2250							B	1	20	1/2				0	SPARE		8
9	M	MINI-SPLIT	1600		10	10	1/2	30	2	A	1	20	1/2				0	SPARE		10
11	M		1600							B	1	20	1/2				0	SPARE		12
13		SPARE	0				1/2	20	1	A	1	20	1/2				0	SPARE		14
15		SPARE	0				1/2	20	1	B	1	20	1/2				0	SPARE		16
17										A										18
19										B										20
21										A										22
23										B										24
25										A										26
27										B										28
29										A										30
31										B										32
33										A										34
35										B										36
37										A										38
39										B										40

B 5/27/2025 FED FROM PANEL 'A'
ALL CIRCUIT CONDUCTORS SIZED FOR COPPER 25.05 Schedules

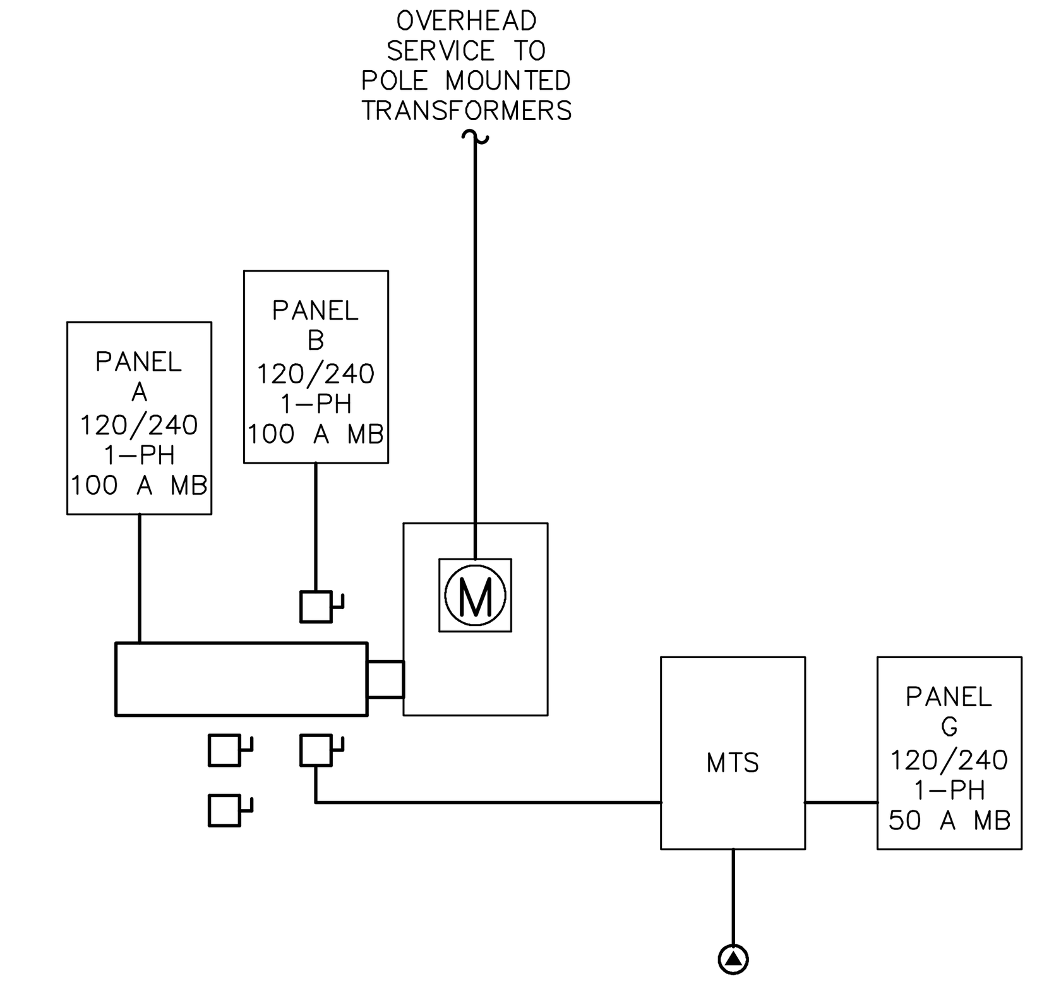
PANEL ' C '

FAULT CURRENT = 7,716
150 AMP MAIN BREAKER 120 / 240 VOLTS 1-PHASE, 3-WIRE
FEEDER SIZE ALUMINUM: 2" C, 3 #3/0 PH, #4 GRD FLUSH MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	25%
CONNECTED VA	459	21662	0	0	0	0	0	12296	9825	22121	102	27651 VA	128
DIVERSITY FACTOR	125%	73%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	574	15831	0	0	0	0	0	8986	7419	16405	75	20506 VA	94

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR		PH		BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1	R	STOVE	4000		6	10	3/4	50	2	A	1	20	1/2	12	12		1600	REC: BATHROOM	R	2
3	R		4000							B	1	20	1/2	12	12		64	BATHROOM LTS / FAN	L	4
5	R	REFRIGERATOR	1200		12	12	1/2	20	1	A	1	20	1/2	12	12		720	REC: EXISTING DAY RM	R	6
7	R	DISHWASHER	1200		12	12	1/2	20	1	B	1	20	1/2	12	12		1200	SLEEPING AREAS	R	8
9	R	GARBAGE DISP.	1656	3/4	12	12	1/2	20	1	A	1	20	1/2				0	SPARE		10
11	R	REC: COUNTER / FAN	1200		12	12	1/2	20	1	B	1	20	1/2				0	SPARE		12
13	R	REC: COUNTER	1200		12	12	1/2	20	1	A	1	20	1/2				0	SPARE		14
15	R	REC: COUNTER	1200		12	12	1/2	20	1	B	1	20	1/2				0	SPARE		16
17	R	REC: ISLAND	1200		12	12	1/2	20	1	A	1	20	1/2				0	SPARE		18
19	R	REC: RADIO COUNTER	360		12	12	1/2	20	1	B	1	20	1/2				0	SPARE		20
21	R	REC: DINING ROOM	720		12	12	1/2	20	1	A	1	20	1/2				0	SPARE		22
23	R	REC / LT: ATTIC	206		12	12	1/2	20	1	B	1	20	1/2	12	12		395	LIGHTS	L	24

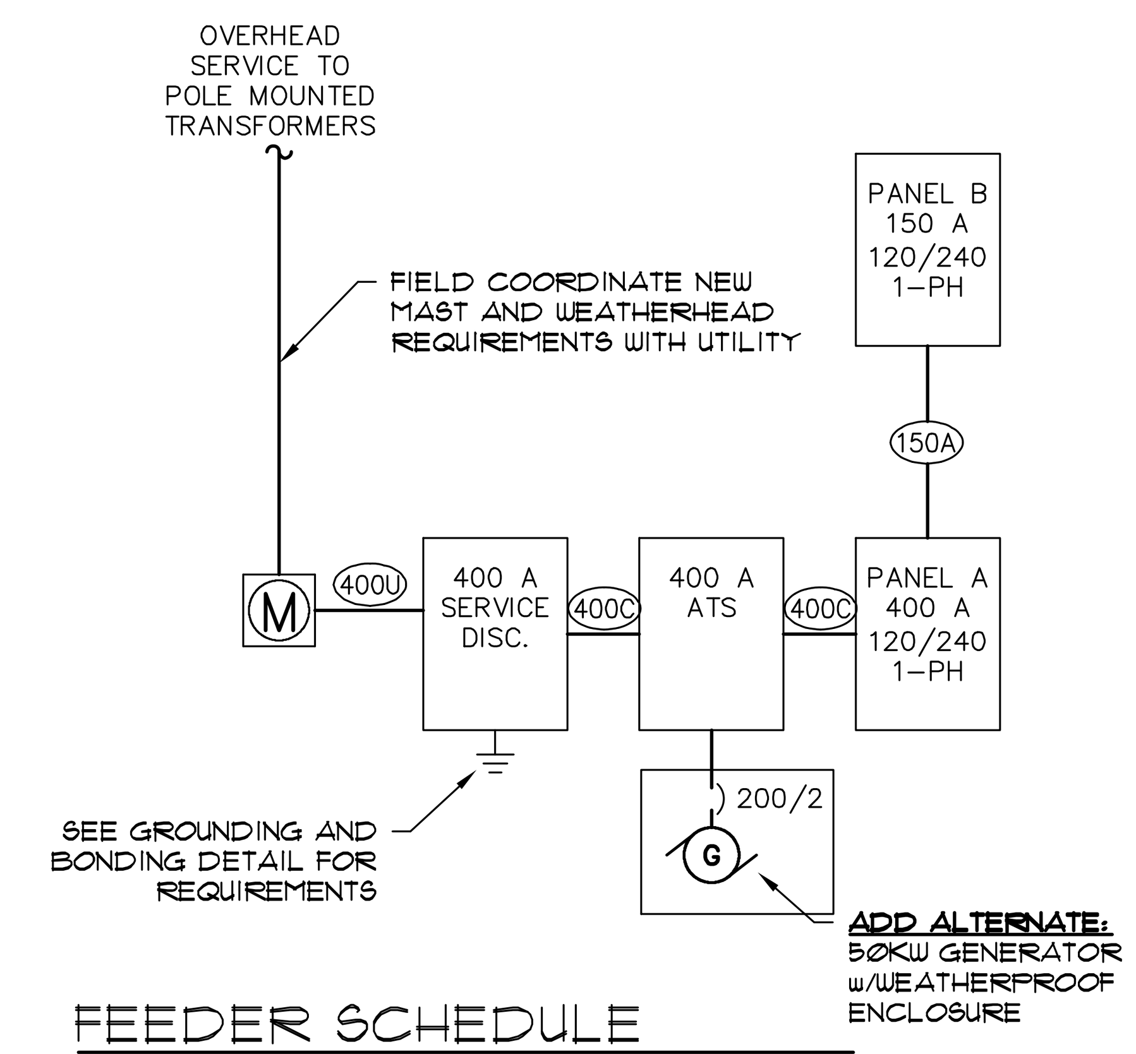
C 5/27/2025 FED FROM PANEL 'A'
ALL CIRCUIT CONDUCTORS SIZED FOR COPPER 25.05 Schedules



APPROXIMATION OF EXISTING 1-LINE DIAGRAM FOR REFERENCE ONLY. ELECTRICAL CONTRACTOR IS RESPONSIBLE TO FAMILIARIZE THEMSELVES WITH EXISTING CONDITIONS.

1 E1.0 EXISTING 1-LINE DIAGRAM

120/240 VOLT, 1-PHASE

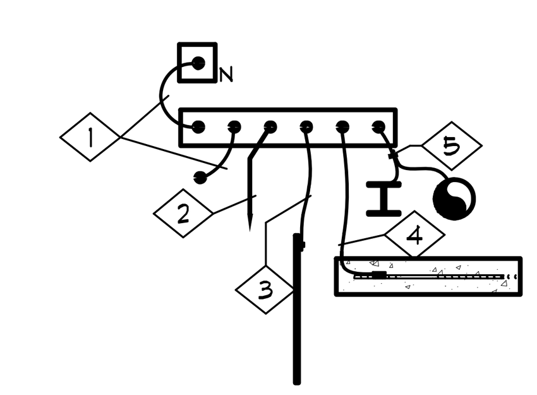


FEEDER SCHEDULE

- (150A) 3 #3/0 AL PH, #4 AL GRD, 2" C
- (200A) 3 #250 AL PH, #4 AL GRD, 2" C
- (400C) 2 SETS – 3 #3/0 CU PH, #1/0 CU GRD, 2" C
- (400U) 2 SETS – 3 #3/0 CU PH, 2" C

2 E1.0 NEW 1-LINE DIAGRAM

120/240 VOLT, 1-PHASE



DETAIL NOTES

1. #1/0 KCMIL CU MAIN BONDING JUMPER AND EQUIPMENT BONDING JUMPER PER NEC 250.28(D) AND 250.102(C)
2. #1/0 KCMIL CU GROUNDING ELECTRODE SYSTEM JUMPER PER NEC 250.52(A)(1,3 AND 5).
3. #6 CU TO ROD, PIPE, OR PLATE ELECTRODES PER NEC 250.66(A)
4. #4 CU TO CONCRETE ENCASED ELECTRODE PER NEC 250.52(A)(3) AND 250.66(B)
5. WHERE REQUIRED, BOND PIPING SYSTEMS AND EXPOSED STRUCTURAL STEEL PER NEC 250.104

3 E1.0 GROUNDING DETAIL

TYPICAL

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REGISTERED PROFESSIONAL ENGINEER
65580
OREGON
JULY 13, 2004
GREGORY L. PRIDE
EXPIRES 12-31-26

PROJECT NO.: 22.22.2

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE

CITY OF COOS BAY
365 D ST,
COOS BAY, OR 97420

CONSTRUCTION

REVISIONS:

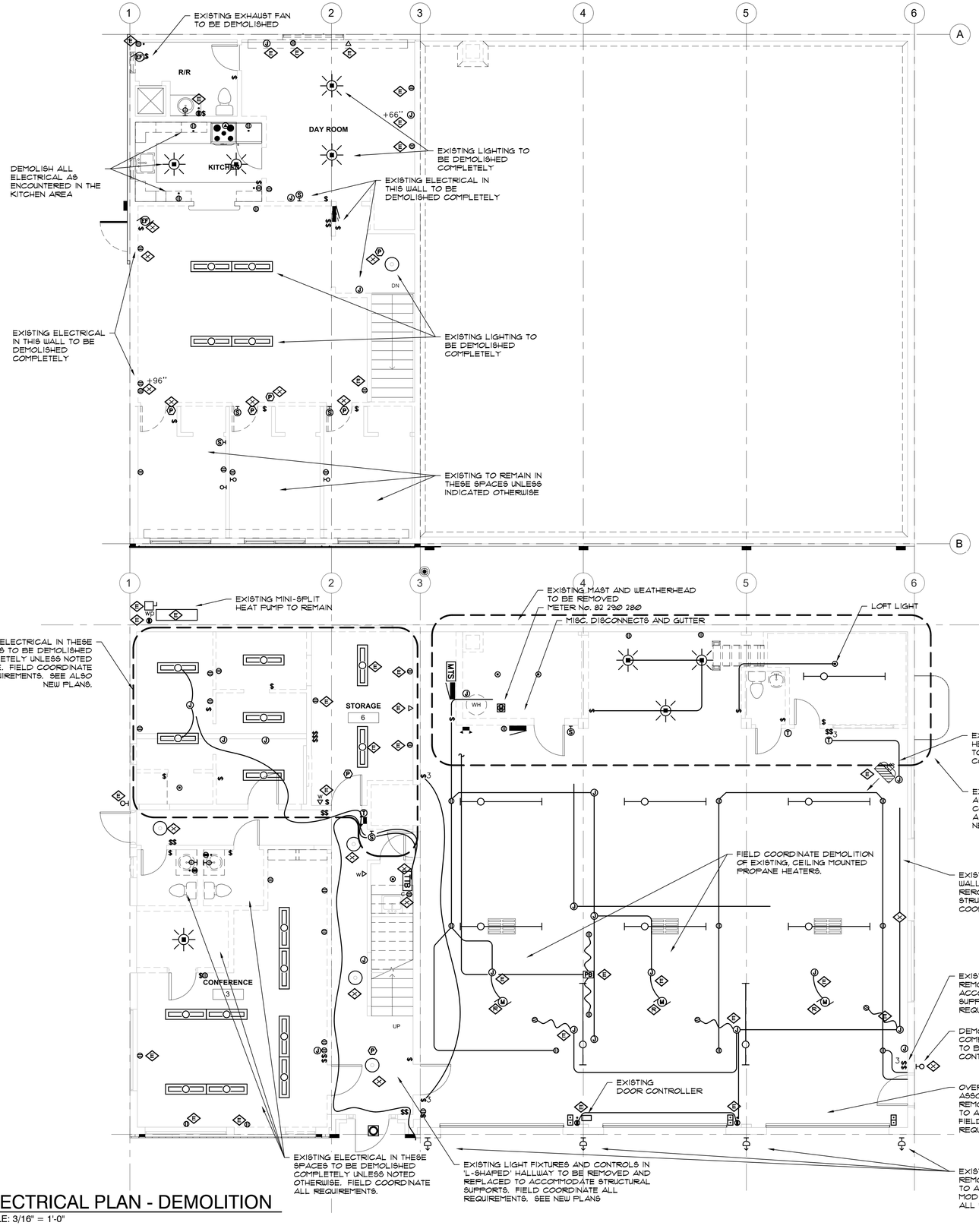
#	DATE	DESCRIPTION

DATE: MAY 2025

SHEET TITLE:
ELECTRICAL PLAN SYMBOLS & SCHEDULES

E1.0

FILE No. D:\Double 'E' Engineering_Projects\25.05 Eastside Fire Station\Drawings\25.05 Eastside Fire Electrical Plans.dwg 05/27/25 07:59 – greg



1 / E1.1 ELECTRICAL PLAN - DEMOLITION
SCALE: 3/16" = 1'-0"

PROJECT NOTES

1. WORK SHOWN ON PLAN IS BASED ON AVAILABLE INFORMATION AT THE TIME OF DESIGN. FIELD COORDINATE SEISMIC UPGRADE REQUIREMENTS WITH ARCHITECTURAL AND STRUCTURAL. CONTRACTOR IS TO FIELD VERIFY EXISTING CIRCUITING AND INSTALLATION.
2. CONTRACTOR SHALL COORDINATE AND PERFORM NECESSARY ELECTRICAL DEMOLITION WORK ASSOCIATED WITH ALL ITEMS AND EQUIPMENT TO BE REMOVED.
3. CONFIRM THAT ALL EXISTING DEVICES AND EQUIPMENT PLANNED FOR REUSE ARE IN GOOD OPERATING CONDITION. UNSUITABLE ITEMS SHALL NOT BE REUSED. RETURN ALL OTHER ITEMS SUITABLE FOR REUSE TO OWNER.
4. WIRING WHICH SERVES USABLE EXISTING OUTLETS SHALL BE REROUTED AND RESTORED CLEAR OF CONSTRUCTION. MAINTAIN ELECTRICAL CONTINUITY OF EXISTING SYSTEM. REPAIR AND RECONDITION ASSOCIATED SURFACES TO MATCH ADJACENT SURFACES. VERIFY EXACT LOCATIONS IN THE FIELD.

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REGISTERED PROFESSIONAL ENGINEER
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OREGON
JULY 19, 2006
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EXPIRES 12-31-26

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EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE

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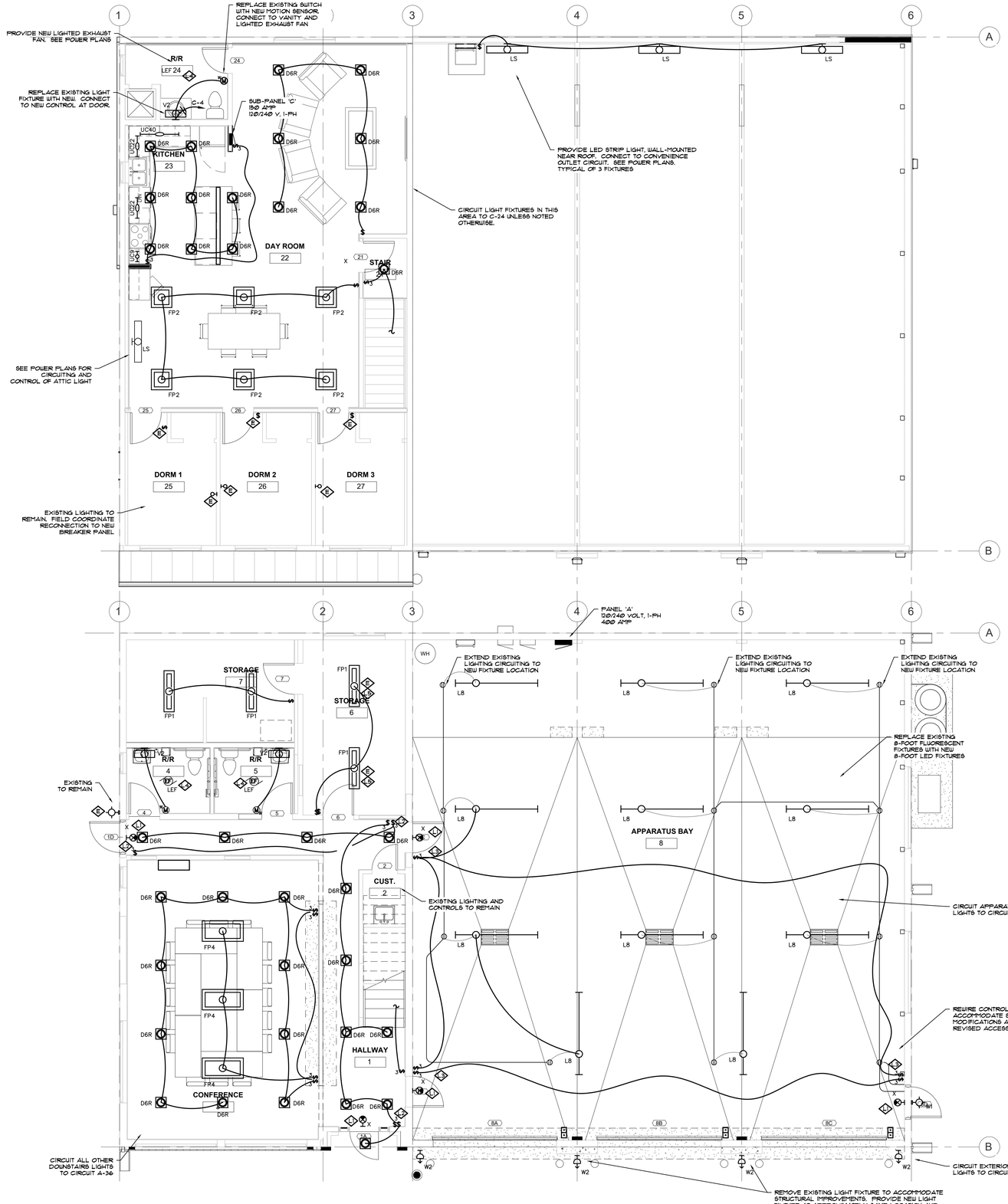
SHEET TITLE:
ELECTRICAL PLAN DEMOLITION

E1.1



1 E2.0 ELECTRICAL PLAN - LIGHTING
SCALE: 3/16" = 1'-0"

LIGHT FIXTURE SCHEDULE

NAME	MANUFACTURER	DESCRIPTION	
D6R / D6RE	LITHONIA LDN6 35/2000 L06 AR LD MVOLT	TYPE: TRIM: MOUNTING: D6E: LAMPS:	6" LED RECESSED DOWNLIGHT SELF FLANGED, CLEAR DIFFUSE RECESSED - VERIFY CEILING TYPE ADD EMERGENCY BATTERY PACK LED 3500K, 2000 LUMENS, 23 WATTS
D6W	LITHONIA WF6 LL LED 27K30K35K 90CRI MW	TYPE: TRIM: MOUNTING: LAMPS:	6" LED CANLESS DOWNLIGHT MATTE WHITE RECESSED - VERIFY CEILING TYPE LED 3000K, 900 LUMENS, 10 WATTS
L8	LITHONIA CSS L96 ALO4 MVOLT SWW3 80CRI CORD & PLUG	TYPE: HOUSING: FINISH: MOUNTING: FINISH: LAMPS:	8-FOOT LED STRIP DIE-FORMED 22 GAUGE, PRIMED COLD ROLLED STEEL WHITE POLYESTER ENAMEL w/POLYCARBONATE LENS SURFACE WHITE POLYESTER ENAMEL LED 3500K, 8000 LUMENS, 64 WATTS
FP1	LITHONIA CPANL 1X4 24/33/44LM 35K M4 2X2CRMK	TYPE: HOUSING: FINISH: MOUNTING: LAMPS:	1 X 4 LED FLAT PANEL ALUMINUM FRAME WHITE POLYESTER COATING SURFACE - CEILING LED, 82 CRI, 3,500K, 3,300 LUMEN (20/28/39 WATTS)
FP2	LITHONIA CPANL 2X2 ALO1 SWWT M4	TYPE: HOUSING: FINISH: MOUNTING: LAMPS:	2 X 2 LED FLAT PANEL ALUMINUM FRAME WHITE POLYESTER COATING SURFACE - CEILING LED, 82 CRI, 3,500K, 3,300 LUMEN (31 WATTS)
FP4	LITHONIA CPANL 2X4 40/50/60LM 35K M2 2X4CRMK	TYPE: HOUSING: FINISH: MOUNTING: LAMPS:	2 X 4 LED FLAT PANEL ALUMINUM FRAME WHITE POLYESTER COATING SURFACE - CEILING LED, 82 CRI, 3,500K, 5,000 LUMEN (32/42/52 WATTS)
LEF	PANASONIC FV-11IBVKL2 FV-V5IBVK1 MULTI SPEED MODULE FV-M8VK1 MOTION SENSOR	TYPE: OPERATION: FINISH: MOUNTING: LAMPS:	LIGHTED EXHAUST FAN 50 CFM CONTINUOUS / 130 CFM OCCUPIED WHITE POWDER COAT SURFACE - WALL LED, 3,000K, 700 LUMEN (10+30 WATTS)
LS	LITHONIA - Z SERIES ZLID L48 SMR 5000LM FST MVOLT 35K 80CRI WH	TYPE: HOUSING: FINISH: MOUNTING: LAMPS:	LED STRIP LIGHT COLD-ROLLED STEEL, ACRYLIC LENS WHITE POWDER COAT SURFACE - WALL LED, 3,500K, 5,000 LUMEN (41 WATTS)
UCxx	KELVIX - SEE PLANS FOR MODELS UC9-3040-010V-120277-WH UC22-3040-010V-120277-WH UC40-3040-010V-120277-WH	TYPE: MOUNTING: FINISH: CONTROL: LAMPS:	UNDER CABINET LIGHT SURFACE - UNDER CABINET WHITE CONTROL w/SEPARATE WALL SWITCH LED , 3,000 K
V2	LITHONIA FMVTSL 24IN MVOLT 30K 90CRI BN M4	TYPE: MOUNTING: FINISH: LENS: LAMPS:	24" VANITY LIGHT SURFACE - WALL BRUSHED NICKEL WHITE ACRYLIC DIFFUSER LED 3,000K, 1300 LUMEN, 9.5 WATTS
W1	LITHONIA WDGE2 LED P13 30K 80CRI VF MVOLT SRM DDBXD	TYPE: HOUSING: FINISH: MOUNTING: LAMPS:	LED WALL PACK DIE-CAST ALUMINUM DARK BRONZE SURFACE - WALL LED, 3,000K, 1,200 LUMEN (10 WATTS)
W2	LITHONIA WDGE2 LED P5 30K 80CRI VF MVOLT SRM DDBXD	TYPE: HOUSING: FINISH: MOUNTING: LAMPS:	LED WALL PACK DIE-CAST ALUMINUM DARK BRONZE SURFACE - WALL LED, 3,000K, 6,000 LUMEN (48 WATTS)
X	LITHONIA - ECBR LED M6	TYPE: HOUSING: FINISH: MOUNTING: LAMPS: BATTERY: NOTE:	LED EXIT SIGN WITH EMERGENCY LIGHT BAR THERMOPLASTIC OR POLYCARBONATE WHITE HOUSING WITH RED LETTERS FIELD VERIFY MOUNTING LED NI-CAD BATTERY DOUBLE FACE AS NECESSARY

EASTSIDE FIRE STATION — ALL FIXTURES ARE 120 VOLT UNLESS NOTED OTHERWISE

SHEET NOTES

1. UNLESS NOTED OTHERWISE, PROVIDE NEW LIGHT FIXTURES, CONTROLS AND CIRCUITING FOR ALL LIGHTING IN RENOVATED SPACES.
2. APPARATUS BAY LIGHTING: EXISTING APPARATUS BAY LIGHTING CONSIST OF 8-FOOT CORD & PLUG FLUORESCENT FIXTURES. CONDUIT AND RECEPTACLES ARE SURFACE-MOUNTED. REPLACE EXISTING FLUORESCENT FIXTURES WITH NEW LED FIXTURES AS INDICATED. MODIFY CONTROLS AND CONTROL CIRCUITING AS INDICATED. EXTEND LIGHTING INTO RENOVATED AREA AND PROVIDE NEW FIXTURES.
3. EXTERIOR LIGHTING: REPLACE ALL EXTERIOR LIGHTS WITH NEW. PROVIDE NEW PHOTOCELL CONTROL. CONNECT EXTERIOR LIGHTS TO CIRCUIT A-38 IN NEW PANEL. SOME FIXTURES REQUIRE REMOVAL AND CONDUIT, CONDUCTOR AND J-BOX MODIFICATIONS TO ACCOMMODATE STRUCTURAL MODIFICATIONS. FIELD COORDINATE WITH ARCHITECTURAL AND STRUCTURAL PLANS.

KEYED NOTES

1. EXIT SIGNS AND BATTERY-PACK NIGHT LIGHTS: CONNECT TO UNSWITCHED LIGHTING CIRCUIT SERVING THIS AREA.
2. FIELD COORDINATE REQUIREMENTS TO PROVIDE 3-WAY SWITCHING OF NEW LIGHT FIXTURES IN EXISTING HALLWAYS.
3. MODIFY APPARATUS BAY LIGHTING CONTROL AS REQUIRED TO ACCOMMODATE NEW 3-WAY CONTROL LOCATIONS AND GROUPS.
4. NEW VARIABLE SPEED, LIGHTED EXHAUST FAN WITH MOTION CONTROL. FAN TO RUN CONTINUOUSLY ON LOW AND INCREASE SPEED WITH MOTION CONTROL. LIGHT TO BE MOTION CONTROLLED. CONNECT TO UNSWITCHED LIGHTING CIRCUIT SERVING THIS AREA.
5. REPLACE EXISTING STORAGE LIGHT FIXTURE WITH NEW LED FIXTURE AS INDICATED.

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REGISTERED PROFESSIONAL ENGINEER 65580 OREGON JULY 19, 2006 GREGORY L. PRIDE
EXPIRES 12-31-26

PROJECT NO.: 22.22.2

EASTSIDE FIRE STATION SEISMIC GRANT UPGRADE

CITY OF COOS BAY
365 D ST,
COOS BAY, OR 97420

CONSTRUCTION

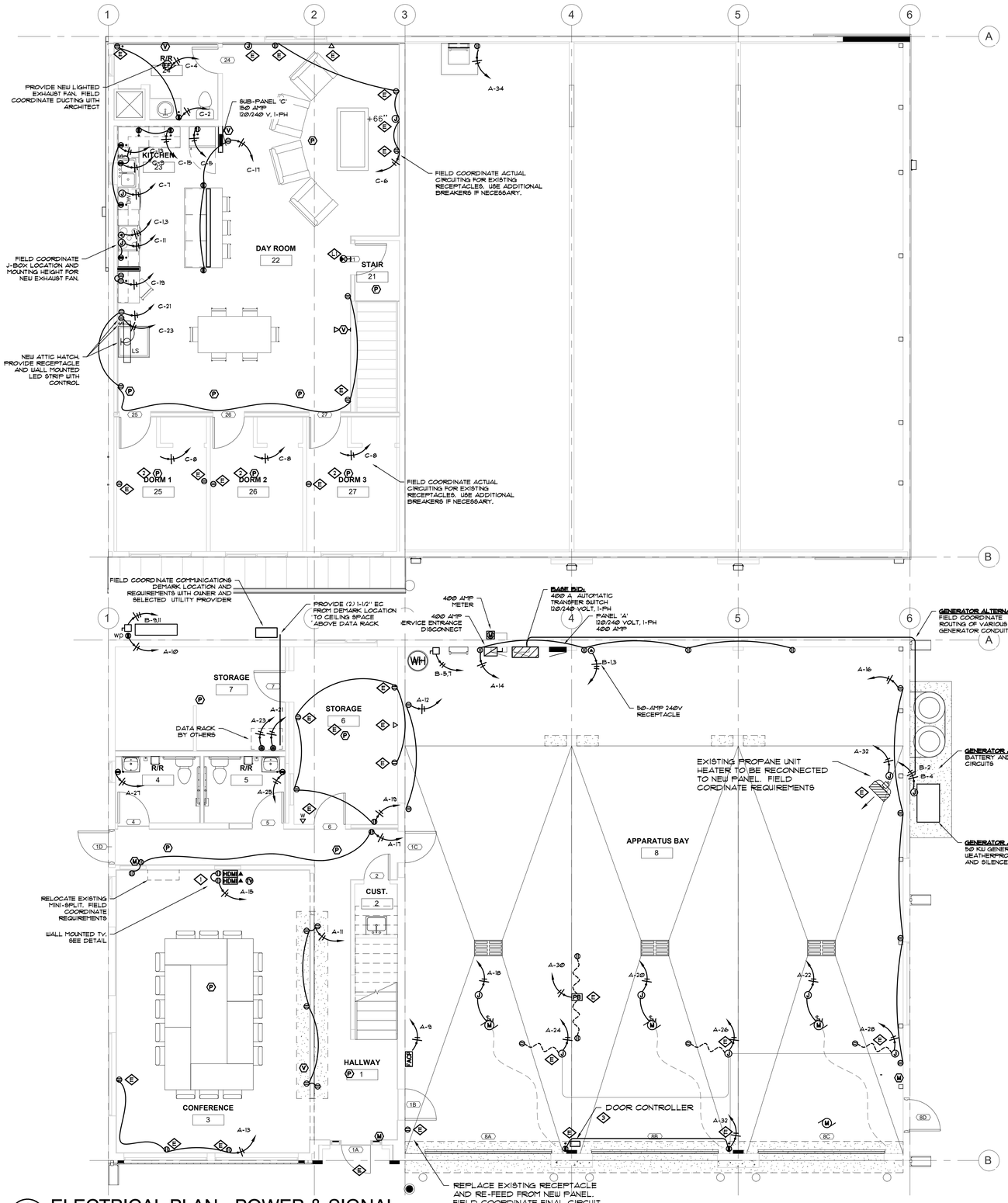
REVISIONS:

#	DATE	DESCRIPTION

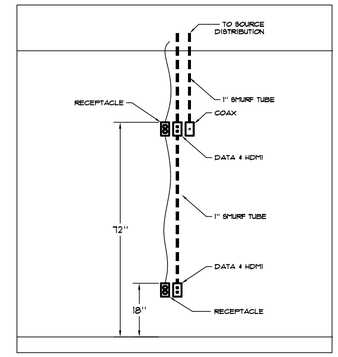
DATE: MAY 2025

SHEET TITLE: ELECTRICAL PLAN LIGHTING

E2.0



1 E2.0 ELECTRICAL PLAN - POWER & SIGNAL
SCALE: 3/16" = 1'-0"



2 E2.0 A/V DETAIL - WALL MOUNTED TV
DAYROOM & TRAINING ROOM

SHEET NOTES

1. UNLESS SPECIFICALLY NOTED AS EXISTING, ASSUME THAT ALL DEVICES SHOWN ARE NEW.
2. FIELD COORDINATE REQUIREMENTS TO RECONNECT EXISTING DEVICES TO NEW ELECTRICAL PANEL - CIRCUIT AS INDICATED.
3. DATA: EXTEND DATA CABLING FROM DATA RACK IN STORAGE TO EACH LOCATION INDICATED BY DATA TRIANGLE.

KEYED NOTES

1. VIDEO WALL: SEE A/V DETAIL FOR WALL MOUNTED TV. PROVIDE RECEPTACLES, COAX TELEVISION, DATA AND HDMI AT VIDEO WALL. PROVIDE RECESSED BOX AT UPPER LOCATION TO ACCOMMODATE LOW-PROFILE WALL MOUNTED TELEVISION.
2. PROVIDE CODE COMPLIANT SMOKE DETECTOR WITH NOTIFICATION FOR SLEEPING AREAS.
3. EMERGENCY GARAGE DOOR OPENER CONTROL: REMOVE AND REINSTALL COMPLETELY TO ACCOMMODATE SEISMIC SUPPORT INSTALLATION. FIELD COORDINATE REQUIREMENTS WITH NEW GARAGE DOORS.

FIRE ALARM

1. PROVIDE A COMPLETE DESIGN-BUILD, CODE COMPLIANT, ADDRESSABLE FIRE ALARM SYSTEM FOR THE ENTIRE FIRE STATION. REPLACE EXISTING DEVICES AND CABLING.
2. DEVICES SHOWN SHOWN ON THE PLANS ARE IN ADDITION TO THE MINIMUM CODE COMPLIANT SYSTEM. CONTRACTOR MAY MODIFY FINAL LOCATIONS SO LONG AS THE INTENT IS MAINTAINED.
3. SLEEPING AREAS DETECTION AND NOTIFICATION SHALL BE PROVIDED WITH CARBON MONOXIDE DETECTION AS PROPANE MAY BE USED IN THE FACILITY FROM TIME TO TIME.

GENERATOR ALTERNATE

BASE BID:
1. PROVIDE ATS-ONLY AS A PART OF THE UTILITY SERVICE AND POWER DISTRIBUTION SYSTEM.

ADD ALTERNATE:
1. PROVIDE 50 KW PROPANE GENERATOR WITH WEATHERPROOF ENCLOSURE AND ALL ASSOCIATED APPURTENANCES (INCLUDING BATTERY CHARGER, WARMER, FREEZE PROTECTION, ETC.)
2. PROVIDE REMOTE ANNUNCIATOR
3. SEE SPECIFICATIONS

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EXPIRES 12-31-26

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DATE: MAY 2025

SHEET TITLE:
ELECTRICAL PLAN
POWER & DATA

E3.0

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