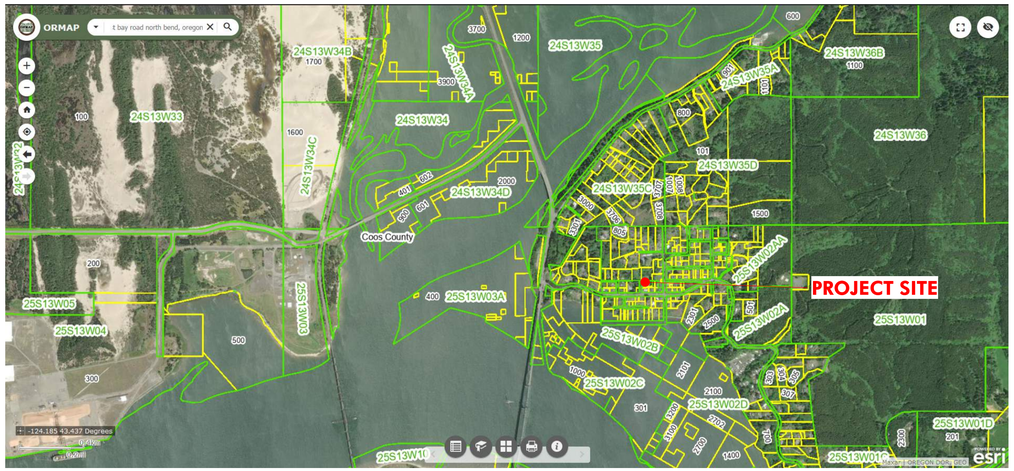
NORTH BAY FIRE STATION
SEISMIC GRANT UPGRADE & ADDITION



VICINITY MAP OVERALL
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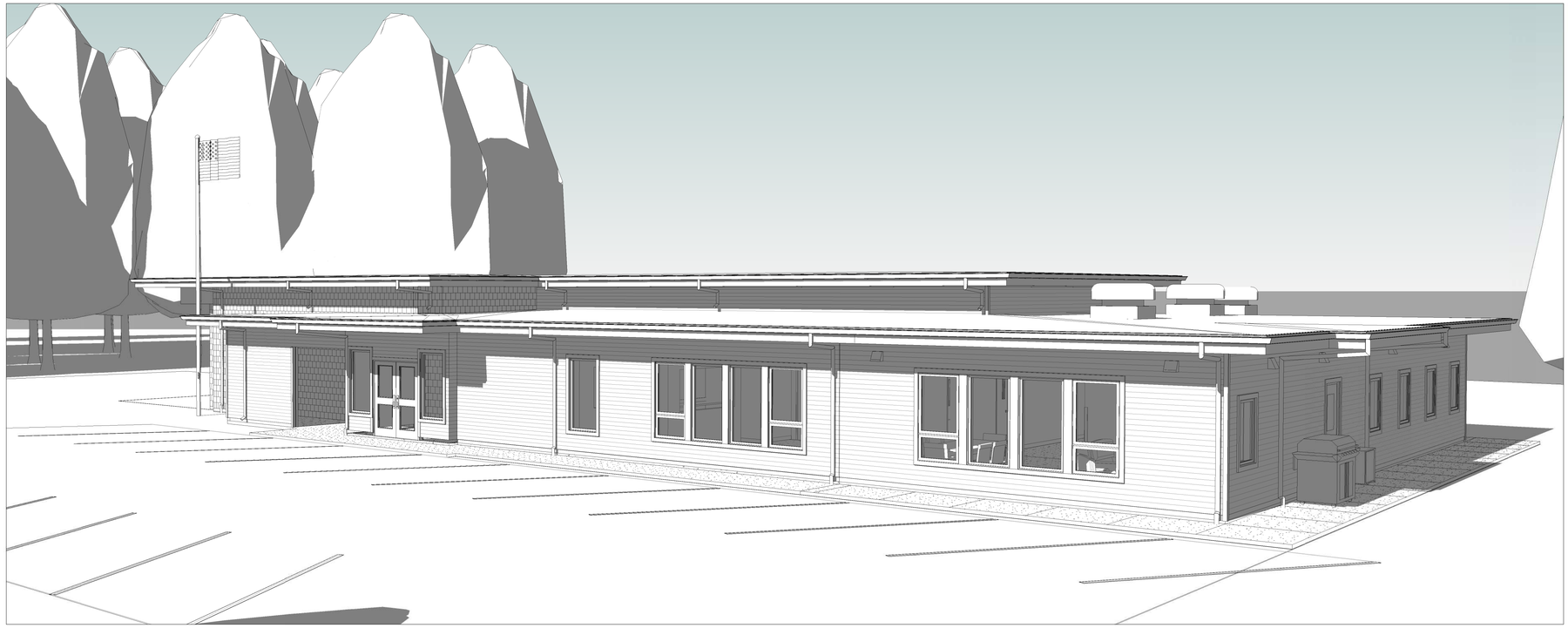

BUILDING ADDITION - VIEW FROM SOUTHEAST

NORTH BAY FIRE DISTRICT

SHEET INDEX

PROJECT TEAM

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REGISTERED ARCHITECT
2840
JOSEPH A. SLACK
COOS BAY, OREGON
STATE OF OREGON

PROJECT NO.: **21.59**

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION
NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: JANUARY 2024

SHEET TITLE:
COVER SHEET

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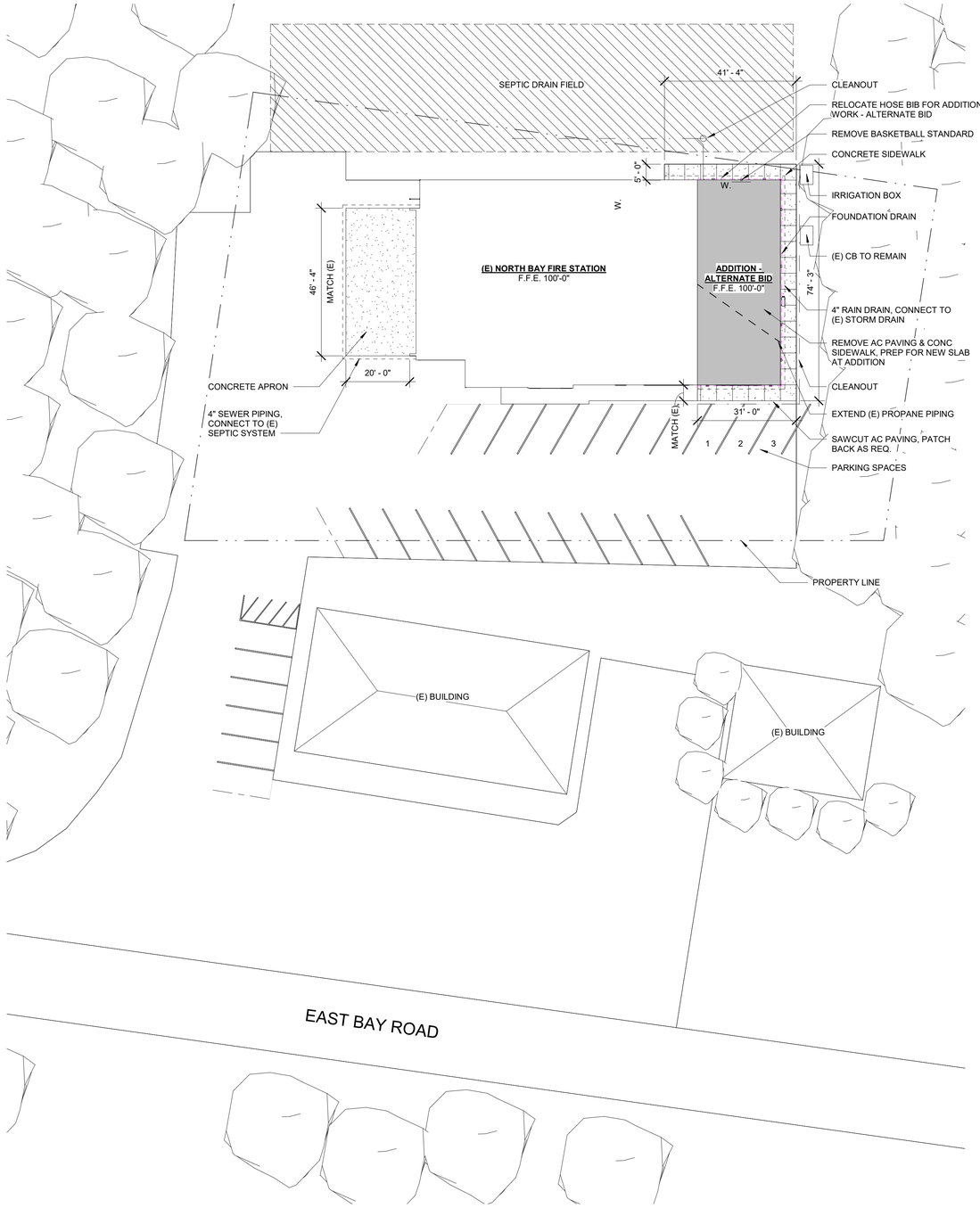


TRUE NORTH PLAN NORTH

2 SITE PLAN - BASE BID

1" = 20'-0"

CODE SUMMARY

CODE: Oregon Structural Specialty Code – 2022

USE & OCCUPANCY: Section 304 Group B, Business
Section 310 Group R-2 - Residential
Section 311 Group S-1, Moderate hazard storage

TABLE 1004.5- MAX FLOOR AREA ALLOWANCE PER OCCUPANT
Business: 1977 sf/150 = 14 occs
Storage - Vehicle Support: 3830 sf/200 = 20 occs
Residential: 332 sf/200 = 2 occs

CONSTRUCTION TYPE: (E) Fire Station - Section 602.5, Type V-B, Non-Protected, Non-Sprinklered

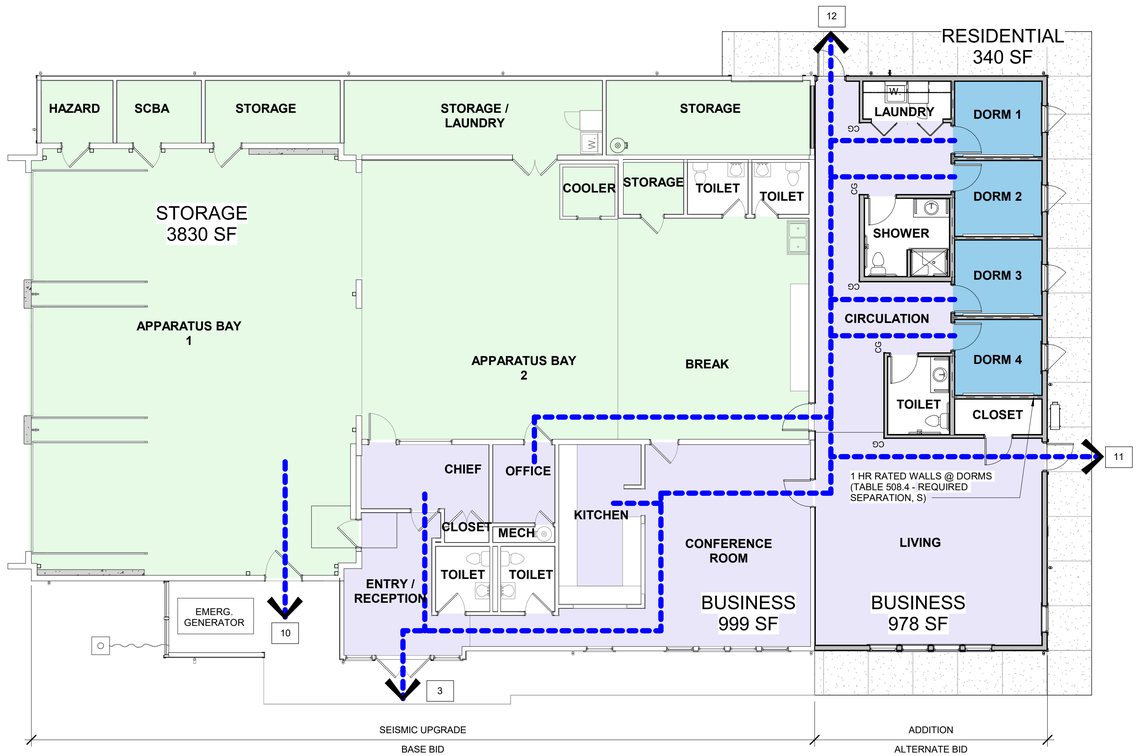
ALLOWABLE BUILDING HEIGHTS & AREAS: Tables 504.3, 504.4, 506.2
(E): Group B, Type V-B, Non-Sprinklered – 9,000 sf, Two-Story, 40 ft Ht.

Actual Area: 5,017 sf (E), 1,534 sf Addition

903.3.1.2 NFPA 13R Sprinkler systems @ R-2 occupancies (dorm rooms) - 1 hr rated walls.

OCCUPANT LOAD - ALTERNATE BID

Number	Name	Area	Occupancy	Code_Room Exits	Code_Room OCC	Code_Room OLF
1	HAZARD	56 SF	STORAGE	1	0.5	200
2	SCBA	66 SF	STORAGE	1	0.5	200
3	STORAGE	106 SF	STORAGE	1	0.5	200
4	STORAGE / LAUNDRY	240 SF	STORAGE	1	1	200
5	STORAGE	189 SF	STORAGE	1	0.5	200
6	APPARATUS BAY 1	1722 SF	STORAGE	1	9	200
7	APPARATUS BAY 2	851 SF	STORAGE	1	4	200
8	BREAK	528 SF	STORAGE	1	3	200
9	COOLER	32 SF	STORAGE	1	0.5	200
10	STORAGE	39 SF	STORAGE	1	0.5	200
11	TOILET	40 SF		0	0	0
12	TOILET	37 SF		0	0	0
13	ENTRY / RECEPTION	166 SF	BUSINESS	1	1	150
14	CHIEF	111 SF	BUSINESS	1	1	150
15	OFFICE	61 SF	BUSINESS	1	1	150
16	KITCHEN	166 SF	BUSINESS	1	1	150
17	CONFERENCE ROOM	495 SF	BUSINESS	1	4	150
18	CLOSET	9 SF		0	0	0
19	MECH	13 SF		0	0	0
20	TOILET	45 SF		0	0	0
21	TOILET	45 SF		0	0	0
22	LIVING	594 SF	BUSINESS	1	3	150
23	CIRCULATION	384 SF	BUSINESS	1	3	150
24	LAUNDRY	42 SF		0	0	0
25	DORM 1	83 SF	RESIDENT DORMS	1	0.5	200
26	DORM 2	83 SF	RESIDENT DORMS	1	0.5	200
27	DORM 3	83 SF	RESIDENT DORMS	1	0.5	200
32	DORM 4	83 SF	RESIDENT DORMS	1	0.5	200
33	SHOWER	82 SF		0	0	0
34	TOILET	61 SF		0	0	0
35	CLOSET	39 SF		0	0	0
		6551 SF			36	



TRUE NORTH PLAN NORTH

1 CODE PLAN

1/8" = 1'-0"

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REGISTERED ARCHITECT 2840 JOSEPH A. SLACK COOS BAY, OREGON STATE OF OREGON

PROJECT NO.: **21.59**

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

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SITE PLAN & CODE INFORMATION - BASE BID

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ABBREVIATIONS

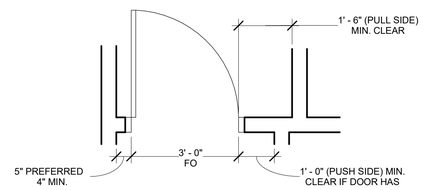
∠	ANGLE
@	AT
AB	ANCHOR BOLT
AC	ACOUSTIC
ACC	ACCESS
ACT	ACOUSTIC CEILING TILE
ACP	ACOUSTIC CEILING PANEL
AD	AREA DRAIN
ADD	ADDITIONAL
ADJ	ADJUSTABLE
AFF	ABOVE FINISH FLOOR
AHU	AIR HANDLING UNIT
AL	ALUMINUM
ALT	ALTERNATE
AP	ACCESS PANEL
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL
ASPH	ASPHALT
BB	BOND BEAM
BD	BOARD
BF	BOTH FACES
BFC	BELOW FINISH CEILING
BG	BUMPER GUARD
BIT	BITUMINOUS
BLDG	BUILDING
BLKG	BLOCKING
BLKT	BLANKET
BM	BEAM/BENCH MARK
BLK	BLOCK
BOT	BOTTOM
BRG	BEARING
BRKR	BREAKER
BRK	BRICK
BRKT	BRACKET
BS	BACK SPLASH
BSMT	BASEMENT
BTWN	BETWEEN
CAB	CABINET
CER	CERAMIC
CFCI	CONTRACTOR FURNISHED, CONTRACTOR INSTALLED
CG	CORNER GUARD
CH	COAT HOOK
CIP	CAST IN PLACE
CJ	CONTROL JOINT/CONSTRUCTION JOINT
CLG	CEILING
CLO	CLOSET/CLOSURE
CLR	CLEAR
COL	COLUMN
COMB	COMBINATION
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CONF	CONFERENCE
CONN	CONNECTION/CONNECT
CONST	CONSTRUCTION
CONT	CONTINUOUS
CONTR	CONTRACTOR
CORR	CORRIDOR
CPT	CARPET
CR	COAT RACK/CURTAIN ROD
CSG	CASING
CT	CERAMIC TILE
CTR	CENTER/COUNTER
CTSK	COUNTERSUNK
CUH	CABINET UNIT HEATER
CW	COLD WATER
D	DEPTH
DBL	DOUBLE
DET	DETAIL
DF	DRINKING FOUNTAIN
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIR	DIRECTION
DIV	DIVISION
DM	DE-MOUNTABLE PARTITION
DN	DOWN
DO	DITTO
DR	DOOR
DRWR	DRAWER
DS	DOWNSPOUT
DWG	DRAWING
DWL	DOWEL
DWS	DEFORMED WELDED STUD
EA	EACH
EC	ELECTRICAL CONTRACTOR
EF	EACH FACE
EH	ELECTRICAL HEATER/EXHAUST HOOD
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR/ELEVATION
EMBED	EMBEDDED
EMER	EMERGENCY
ENT	ENTRANCE
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EMERGENCY SHOWER
ESR	ELASTOMERIC SHEET ROOFING
ETR	(E) TO REMAIN
EVC	ELASTIC VINYL COATING
EW	EACH WAY
EWC	ELECTRIC WATER COOLER
EXC	EXCAVATE
EXP	EXPANSION
EXPD	EXPOSED
EXPF	EXPLOSION PROOF
EXT	EXTERIOR
FA	FIELD ADJUSTABLE
FV	FIELD VERIFY
FD	FLOOR DRAIN
FDN	FOUNDATION
FE	FIRE EXTINGUISHER
FEC	FIRE EXTINGUISHER CABINET
FHC	FIRE HOSE CABINET
FIN	FINISH
FIX	FIXTURE
FLEX	FLEXIBLE
FLR	FLOOR
FLRG	FLOORING
FOS	FACE OF STUD
FP	FIREPROOF/FIRE PROTECTION
FR	FIRE RETARDANT
FS	FULL SIZE/FULL SCALE
FT	FEET
FTG	FOOTING
FURG	FURRING
GA	GAUGE
GAL	GALLON
GALV	GALVANIZED
GB	GRAB BAR
GC	GENERAL CONTRACTOR
GEN	GENERAL
GFCI	GOVERNMENT FURNISHED, CONTRACTOR INSTALLED
GFGI	GOVERNMENT FURNISHED, GOVERNMENT INSTALLED
GFRC	GLASS FIBER REINFORCED CONCRETE
GFRG	GLASS FIBER REINFORCED GYPSUM
GL	GLASS
GLB	GLUE LAM BEAM
GMU	GLAZED MASONRY UNIT
GWB	GYPSUM WALL BOARD
GYP	GYPSUM
H	HEIGHT
HDBD	HARDBOARD
HDCP	HANDICAPPED
HDWD	HARDWOOD
HDWE	HARDWARE
HK	HOOK
HM	HOLLOW METAL
HP	HIGH POINT
HR	HANDRAIL
HT	HEIGHT
HVAC	HEATING VENTILATION AND AIR CONDITIONING
HWS	HEAD WELDED STUDS
ID	INSIDE DIAMETER
IMP	INSULATED METAL PANEL
IN	INCHES
INFO	INFORMATION
INSUL	INSULATION
INT	INTERIOR
IPW	INSULATED PLENUM WALL
IRF	INSULATED ROOF FILL
JAN	JANITOR
JS	JANITOR SINK
JST	JOIST
JT	JOINT
KD	KNOCKED DOWN
KO	KNOCK-OUT / KNEE OPENING
L	LENGTH
LAB	LABORATORY
LAM	LAMINATED
LB	POUND
LBS	POUNDS
LD	LINEAR DIFFUSER
LDG	LANDING
LF	LINEAR FOOT
LG	LONG
LGT	LIGHT
LKR	LOCKER
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LP	LOW POINT
LSH	LONG SLOTTED HOLE
LTG	LIGHTING
LVR	LOUVER
LWC	LIGHTWEIGHT CONCRETE
MACH	MACHINE
MAN	MANUAL
MAR	MARBLE
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MBW	MASONRY BEARING WALL
MC	MECHANICAL CONTRACTOR
MDO	MEDIUM DENSITY OVERLAY
MECH	MECHANICAL
MEMB	MEMBRANE
MET	METAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MIN	MINIMUM
MIR	MIRROR
MISC	MISCELLANEOUS
MK	MARK
ML	METAL LATH
MLDG	MOLDING
MO	MASONRY OPENING
MP	METAL PARTITION
MS	MACHINE SCREW
MTD	MOUNTED
MTG	MOUNTING
NA	NOT APPLICABLE
NIC	NOT IN CONTRACT
NO	NUMBER
NOM	NOMINAL
NS	NON-SHRINK
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE
OA	OVERALL
OC	ON CENTER
OD	OUTSIDE DIAMETER/OVERFLOW DRAIN
OFF	OFFICE
OPNG	OPENING
OPP	OPPOSITE
OZ	OUNCE
PART	PARTITION
PC	PIECE
PCC	PRECAST CONCRETE
PCPL	PORTLAND CEMENT PLASTER
PDWR	PAPER TOWEL DISPENSER & WASTE RECEPTACLE
PH	PHILLIPS HEAD/PHASE
PL	PLATE/PROPERTY LINE
PLAM	PLASTIC LAMINATE
PLAS	PLASTER
PLBG	PLUMBING
PLYWD	PLYWOOD
PM	PROTECTED METAL
PNL	PANEL
PNLG	PANELING
POL	POLISHED
PR	PAIR
PRE FAB	PREFABRICATED
PRE FIN	PRE-FINISHED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT/PAINT
PTM	PAINT TO MATCH
PVC	POLYVINYL CHLORIDE
QT	QUARRY TILE
QTY	QUANTITY
RAD	RADIUS
RAH	ROOFTOP AIR HANDLING UNIT
RB	RUBBER BASE
RC	REINFORCED CONCRETE
RCP	RADIANT CEILING PANEL / REFLECTED CEILING PLAN
RD	ROOF DRAIN
REC	RECESSED
REF	REFERENCE
REINF	REINFORCING
REL	RELOCATE
REM	REMAINDER
REQD	REQUIRED
RES	RESILIENT
RET	RETURN
RI	ROUGH IN
RM	ROOM
RO	ROUGH OPENING
RT	RUBBER TILE
RUB	RUBBER
SAMF	SELF ADHESIVE MEMBRANE FLASHING
SAT	STANDARD AGGREGATE TOPPING
SAWRB	SELF ADHESIVE WEATHER RESTISTANT BARRIER
SB	SOIL BEARING
SC	SEAMLESS COATING
SCF	SPECIAL CONCRETE FINISH
SCHD	SCHEDULE
SD	SOAP DISPENSER
SE	SHELF EDGE
SECT	SECTION
SF	SAND FLOAT
SG	SUPPLY AIR GRILLE
SGL	SINGLE
SH	SHELF
SHD	SHOWER DOOR
SHT	SHEET
SIM	SIMILAR
SJ	STEEL JOIST
SLV	SHORT LEG VERTICAL
SM	SMOOTH
SND	SANITARY NAPKIN DISPENSER
SNV	SANITARY NAPKIN VENDER
SOG	SLAB ON GRADE
SPEC	SPECIFICATION
SPR	SPRINKLER
SQ	SQUARE
SR	SHOWER ROD
SS	STAINLESS STEEL
ST	STREET
STD	STANDARD
STL	STEEL
STO	STORAGE
STRU	STRUCTURAL/STRUCTURE
SUSP	SUSPENDED
SV	SHEET VINYL
SYM	SYMMETRICAL
T & B	TOP AND BOTTOM
TB	TACKBOARD/TOWEL BAR
TBR	TO BE REMOVED
TCP	THIN COAT PLASTER
TD	TOWEL DISPENSER
TDW	TOWEL DISPENSER AND WASTE
TEMP	TEMPERATURE/TEMPERED
TER	TERRAZZO
TEX	TEXTURE
TFC	TROWELED FLOOR COVERING
T & G	TONGUE AND GROOVE
THK	THICK
TOB	TOP OF BEAM
TOC	TOP OF CURB/TOP OF CONCRETE
TOD	TOP OF DECK/TOP OF DUCT ELEVATION
TOF	TOP OF FOOTING
TOG	TOP OF GRATE
TOJ	TOP OF JOIST
TOP	TOP OF PIPE ELEVATION
TOS	TOP OF SLAB/TOP OF STEEL
TOW	TOP OF WALL
TPG	TOPPING
TPH	TOILET PAPER HOLDER
TRAN	TRANSOM
TRANS	TRANSVERSE
TS	TUBE STEEL
TWS	THREADED WELDED STUD
TYP	TYPICAL
UG	UNDERGROUND
UNO	UNLESS NOTED OTHERWISE
UR	URINAL
V	VINYL
VB	VINYL BASE
VCT	VINYL COMPOSITION TILE
VERT	VERTICAL
VEST	VESTIBULE
VOL	VOLUME
VWC	VINYL WALL COVERING
W	WIDE FLANGE STEEL BEAM
W/	WITH
WAF	WELDED ANGLE FRAME
WC	WATER CLOSET
WD	WOOD
WDW	WINDOW
WF	WIDE FLANGE
WG	WIRE GLASS
W/O	WITHOUT
WP	WEATHERPROOF
WPFG	WATERPROOFING
WR	WASTE RECEPTACLE
WRB	WEATHER RESTISTANT BARRIER
WSCT	WAINSCOT
WSTP	WEATHERSTRIP
WTR	WATER
WWF	WELDED WIRE FABRIC
X	(E)

ABBREVIATIONS ABOVE ARE FOR ARCHITECTURAL SHEETS ONLY.

LEGEND - PLAN SYMBOLS

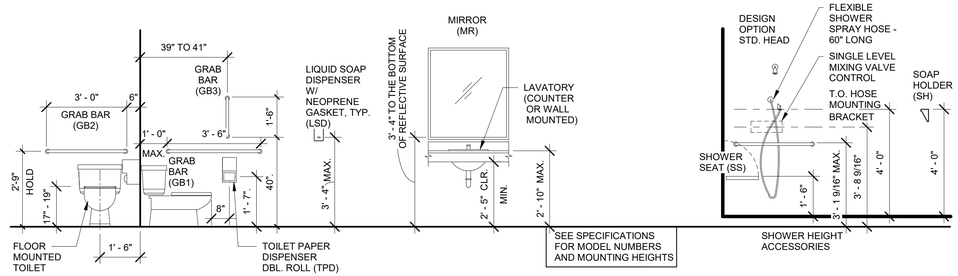
- BUILDING SECTION SYMBOL
- WALL SECTION SYMBOL
- DETAIL SYMBOL
- ENLARGED PLAN SYMBOL
- EXTERIOR ELEVATION SYMBOL
- INTERIOR ELEVATION SYMBOL
- KEYED NOTE IDENTIFICATION
- ROOM NAME AND NUMBER
- WALL TYPE IDENTIFICATION
- WINDOW IDENTIFICATION
- DOOR IDENTIFICATION
- 1 HOUR FIRE RATED WALL
- FIRE EXTINGUISHER - SURFACE MOUNT
- FIRE EXTINGUISHER CABINET AND FIRE EXTINGUISHER - SEMI-RECESSED
- FLOOR DRAIN
- (E) GRID LINES
- NEW GRID LINES
- LEVEL OR SPOT ELEVATIONS
- CEILING HEIGHT & FINISH

DOOR LOCATIONS - TYPICAL



A. DOORS SHOWN ADJACENT TO FLANKING WALL OR OTHER FIXED OBSTRUCTION, SHALL BE LOCATED AS SHOWN ABOVE

B. OTHER LOCATIONS SHALL BE ON CENTERLINE OF ROOM OR AS SPECIFICALLY DIMENSIONED



FIXTURE AND ACCESSORY MOUNTING DIAGRAMS

3/8" = 1'-0"

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REGISTERED ARCHITECT 2840 JOSEPH A. SLACK COOS BAY, OREGON STATE OF OREGON

PROJECT NO.: **21.59**

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

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GENERAL INFORMATION

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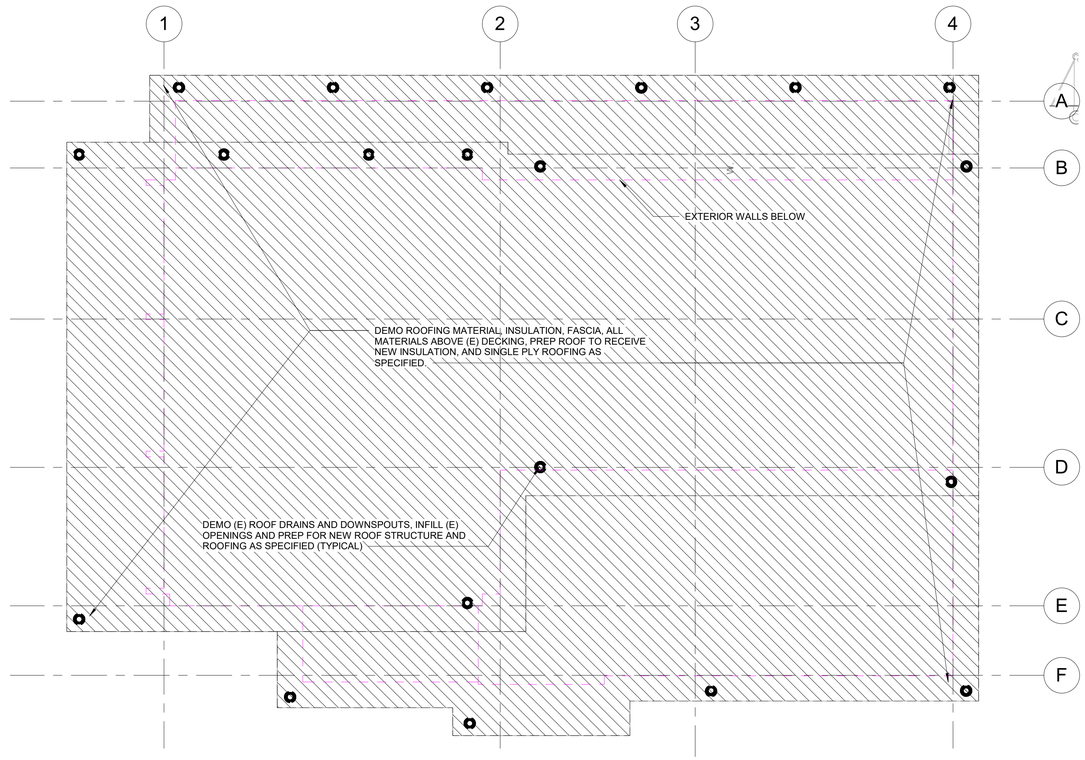


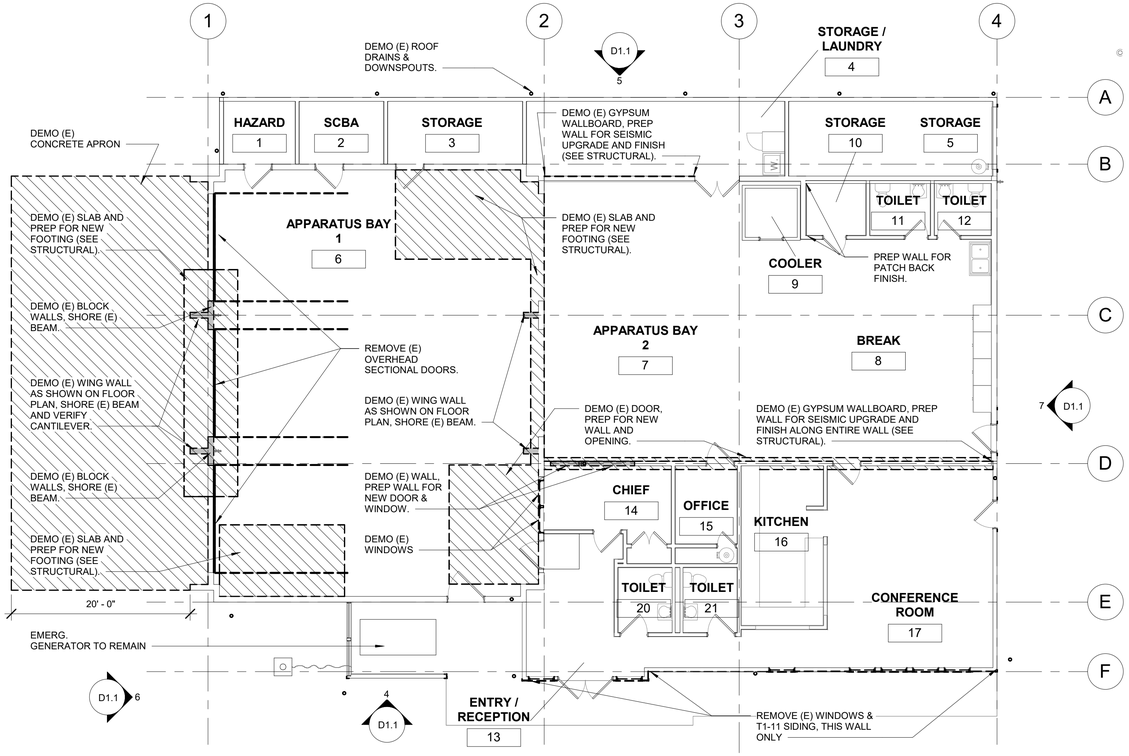
TRUE NORTH PLAN NORTH

2 ROOF PLAN - DEMO - BASE BID

1/8" = 1'-0"

DEMO LEGEND

- (E) FINISH/MATERIAL TO REMAIN
- (E) FINISH/MATERIAL TO BE REMOVED
- (E) WALL TO REMAIN
- (E) WALL TO BE REMOVED



TRUE NORTH PLAN NORTH

1 FLOOR PLAN - DEMO - BASE BID

1/8" = 1'-0"

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DEMOLITION FLOOR & ROOF PLANS - BASE BID

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DEMO LEGEND

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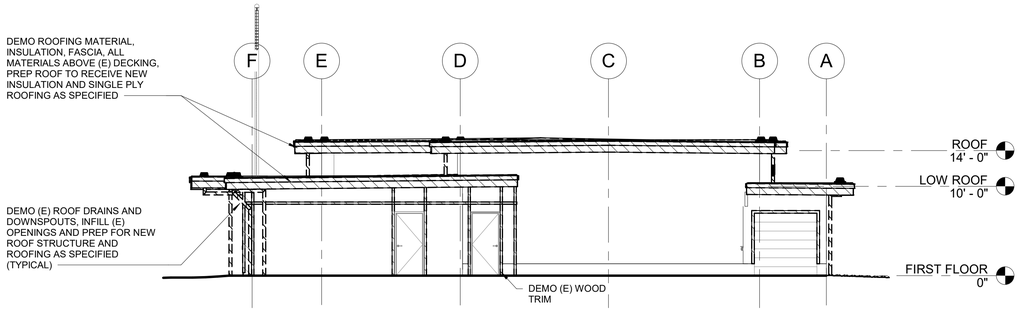
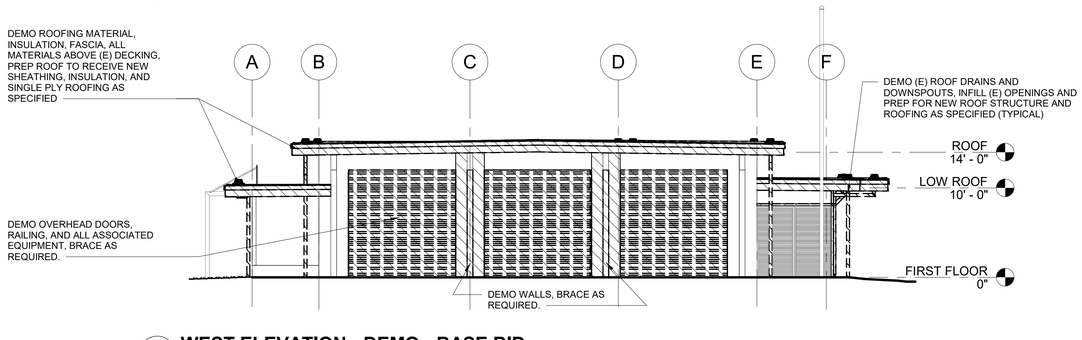
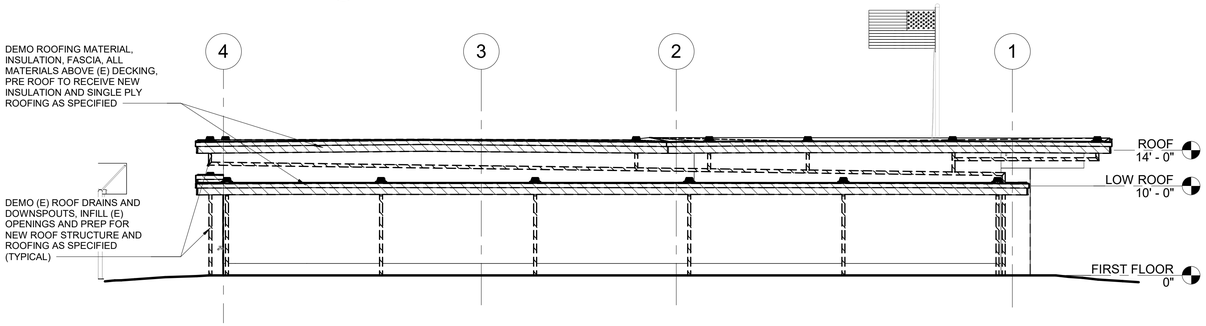
- (E) WALL TO REMAIN
- (E) WALL TO BE REMOVED



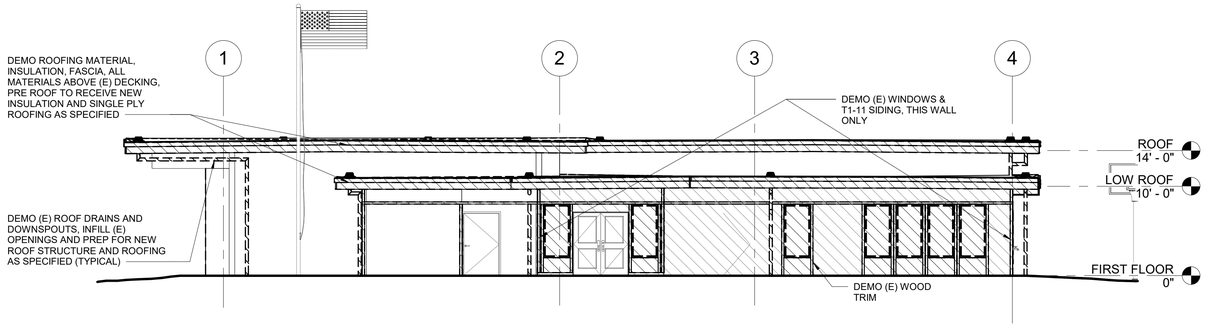
7 EAST ELEVATION - DEMO - BASE BID
1/8" = 1'-0"



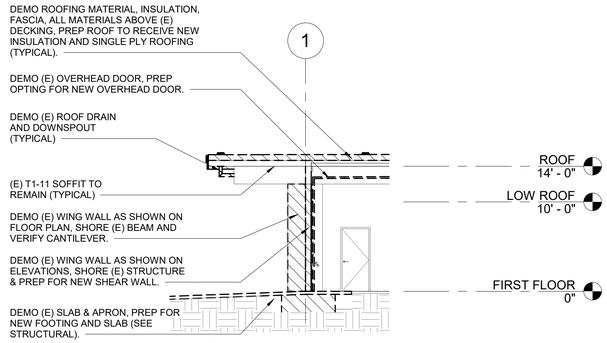
6 WEST ELEVATION - DEMO - BASE BID
1/8" = 1'-0"



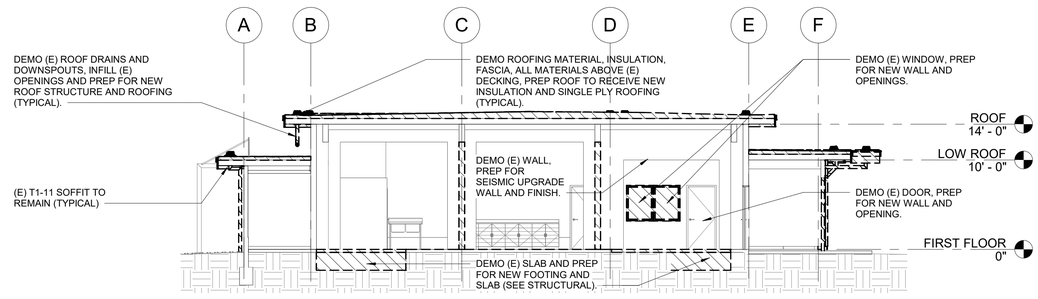
5 NORTH ELEVATION - DEMO - BASE BID
1/8" = 1'-0"



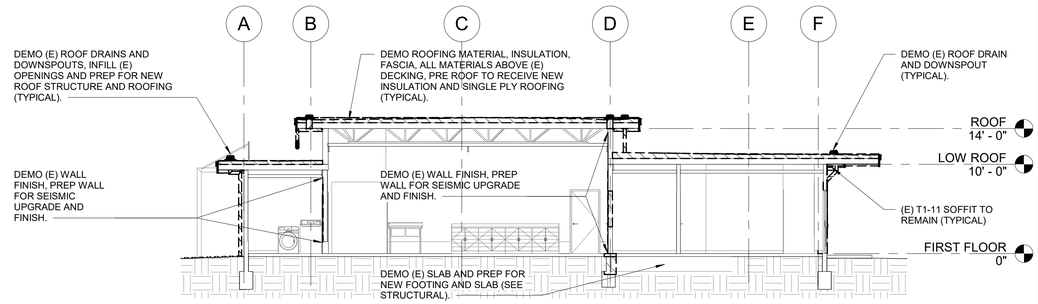
4 SOUTH ELEVATION - DEMO - BASE BID
1/8" = 1'-0"



3 E-W SECTION AT OVERHEAD DOORS - DEMO - BASE BID
1/8" = 1'-0"



2 N-S SECTION AT SHEAR WALLS - DEMO - BASE BID
1/8" = 1'-0"



1 N-S SECTION AT ENTRY / RECEPTION - DEMO - BASE BID
1/8" = 1'-0"

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JOSEPH A. SLACK
COOS BAY, OREGON
STATE OF OREGON

PROJECT NO.: 21.59

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

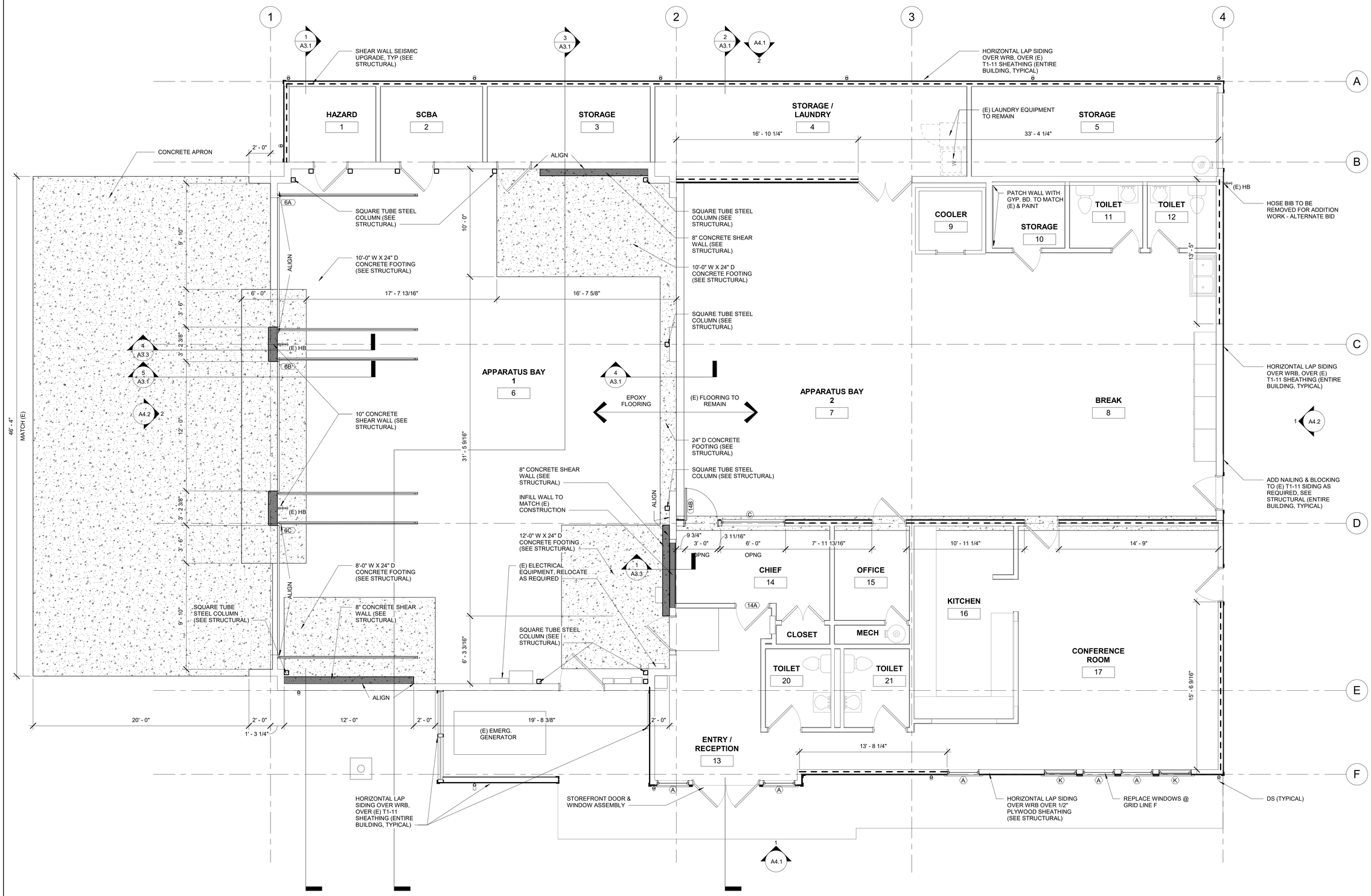
REVISIONS:

#	DATE	DESCRIPTION

DATE: JANUARY 2024

SHEET TITLE:
DEMOLITION BUILDING ELEVATIONS & SECTIONS - BASE BID

D1.1



TRUE NORTH PLAN NORTH

1 FLOOR PLAN - BASE BID

1/4" = 1'-0"

1/24/2024 8:27:29 AM C:\Users\Remote\Documents\21 59 North Bay Fire Station - Seismic Upgrade_gsalazarBHFTX.rvt

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DATE: JANUARY 2024

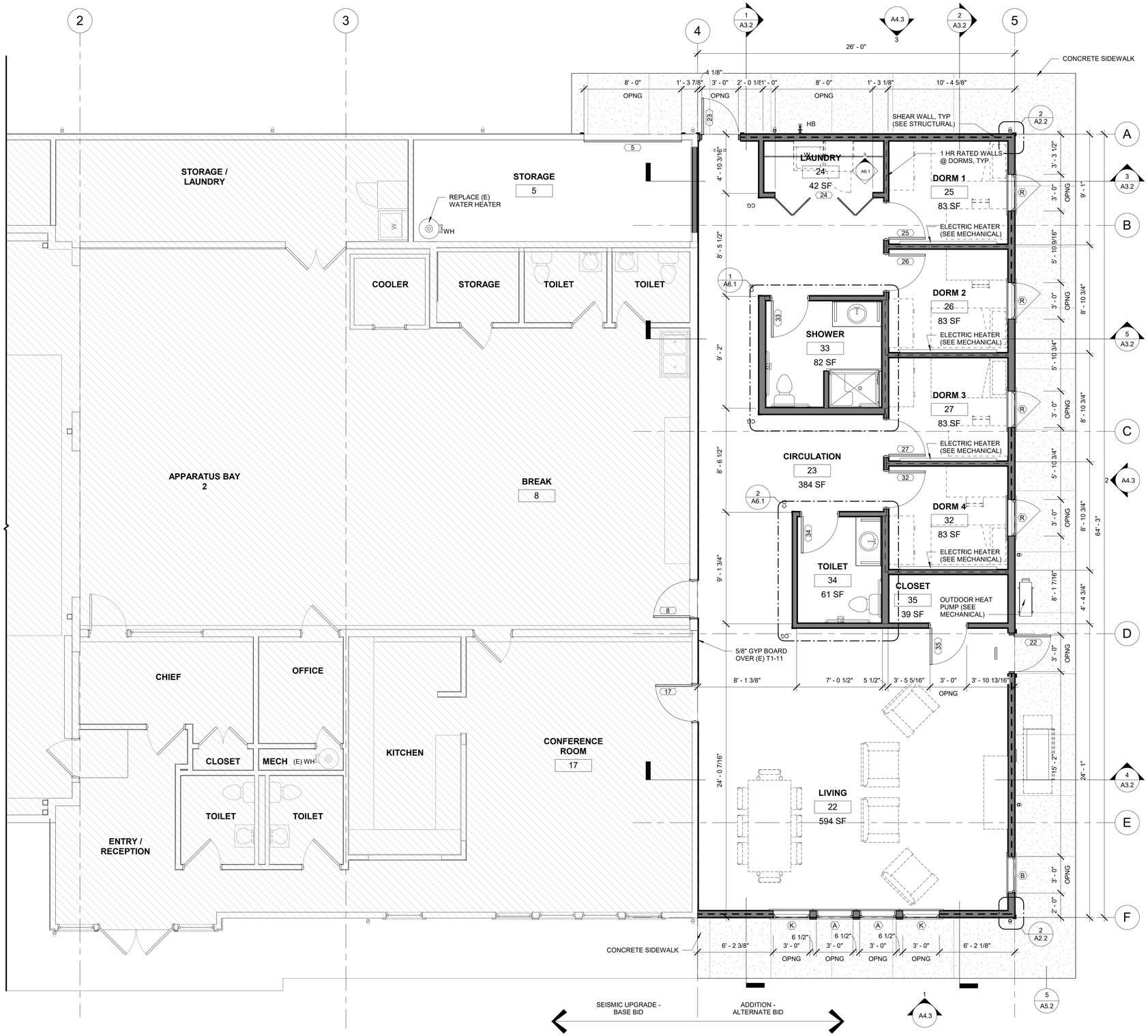
SHEET TITLE:

FIRST FLOOR PLAN - BASE BID

A2.1

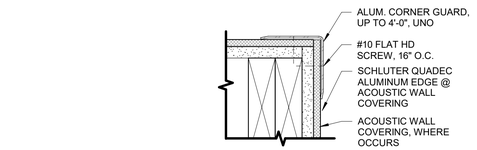
LEGEND

BASE BID AREA OF WORK



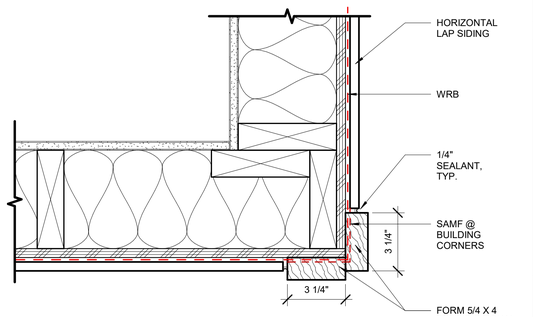
1 FLOOR PLAN - ALTERNATE BID

1/4" = 1'-0"



3 CORNER GUARD

3" = 1'-0"



2 CORNER WITH BOARD SIDING

3" = 1'-0"

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#	DATE	DESCRIPTION

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SHEET TITLE:
FIRST FLOOR PLAN - ALTERNATE BID

A2.2

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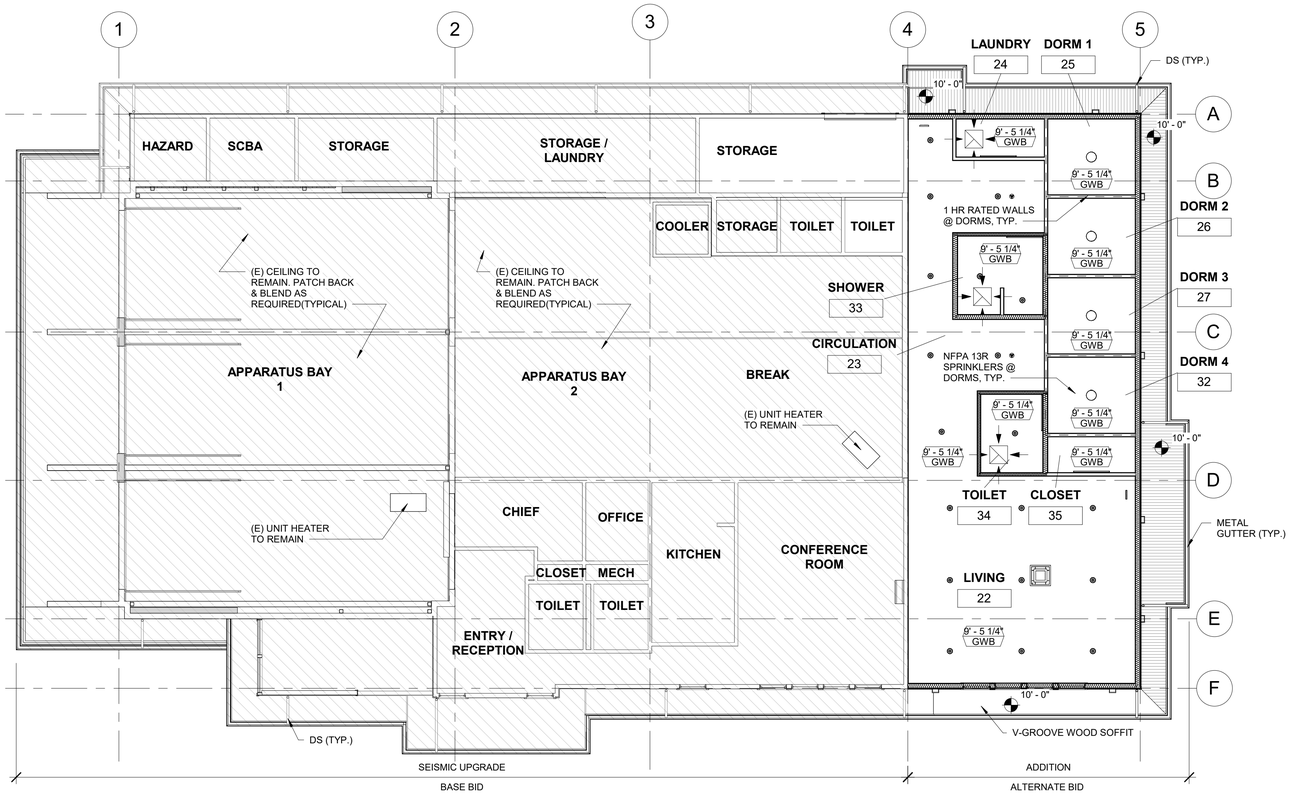
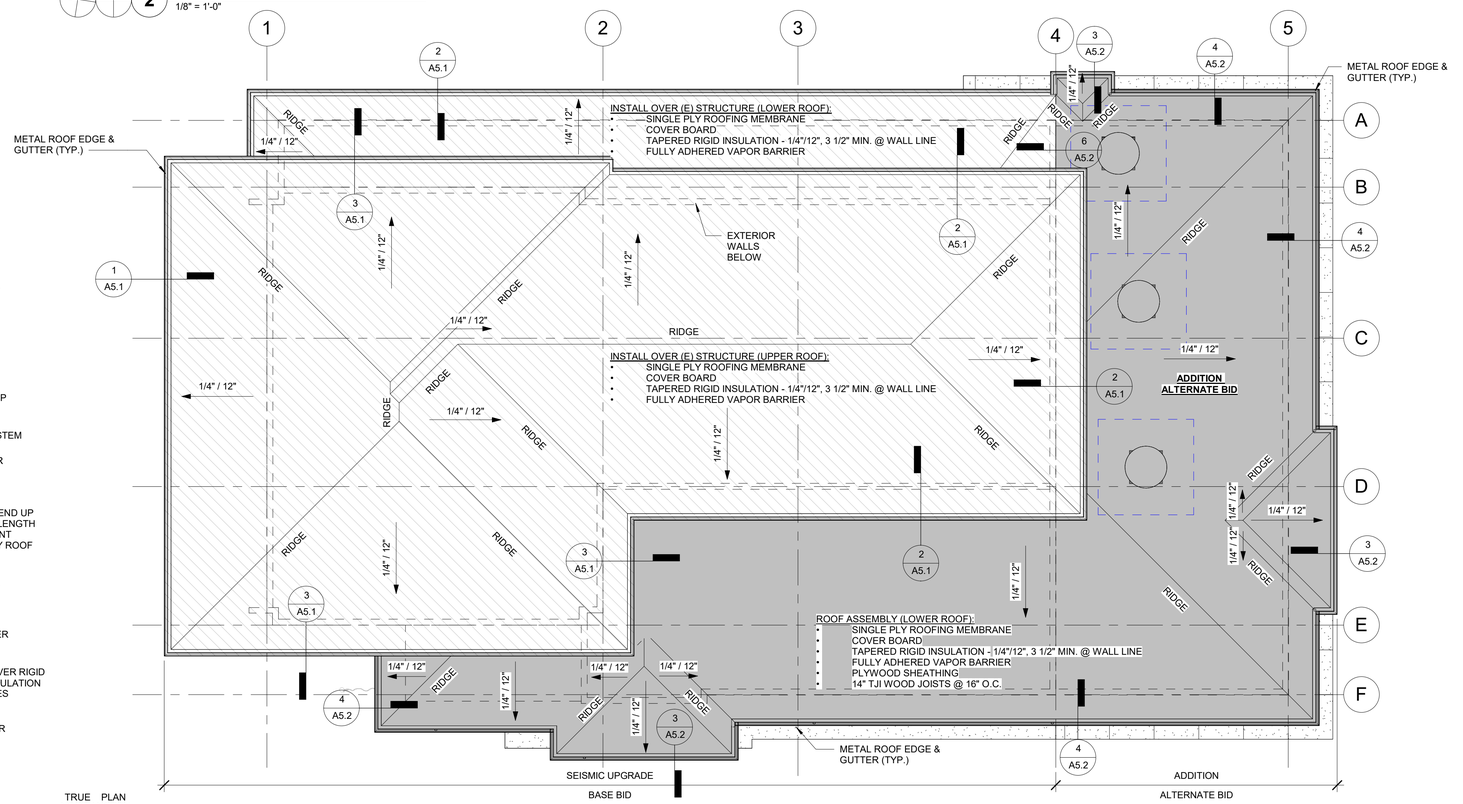


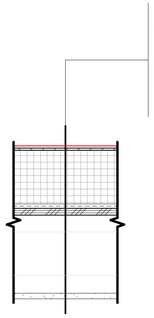
SYMBOLS

- BASE BID AREA OF WORK
- WIFI PORT
- LIGHTING FIXTURES SEE ELECTRICAL FOR TYPE
- 10'-0" ACT — CEILING ELEVATION
- EXIT LIGHT, REFER ELECTRICAL
- EXHAUST FAN, REFER MECHANICAL
- INDOOR HEAT PUMP, COORDINATE WITH FRAMING & MECHANICAL
- RELIEF / INTAKE / EXHAUST HEAD, REFER MECHANICAL

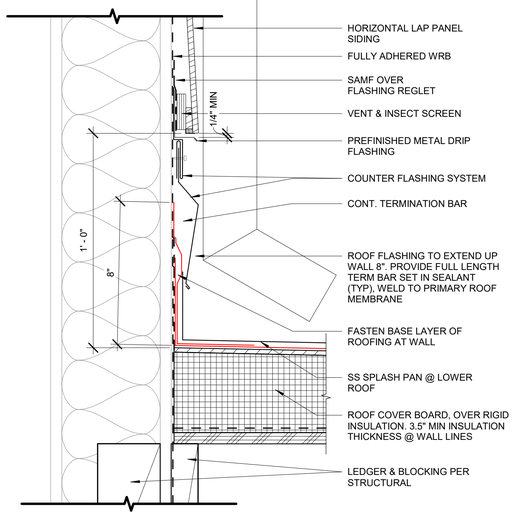
2 REFLECTED CEILING PLAN
1/8" = 1'-0"



1 ROOF PLAN
1/8" = 1'-0"



4 ROOF ASSEMBLY - TYPICAL
1 1/2" = 1'-0"



3 ROOF TO WALL TRANSITION ENLARGED
3" = 1'-0"

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CONSTRUCTION

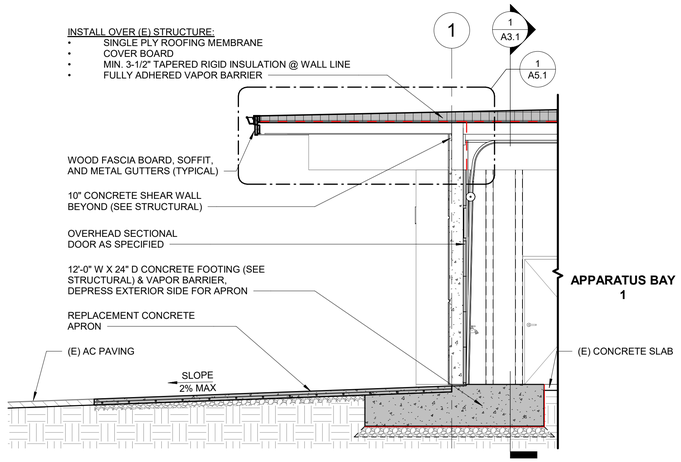
REVISIONS:

#	DATE	DESCRIPTION

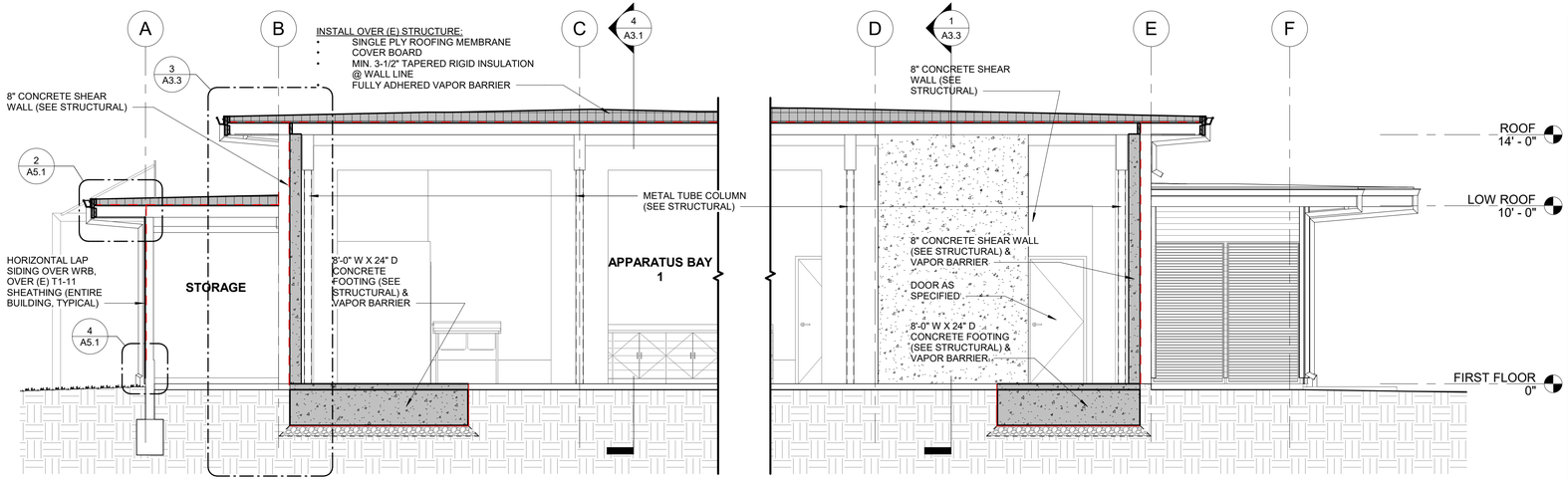
DATE: JANUARY 2024

SHEET TITLE:
REFLECTED CEILING & ROOF PLANS

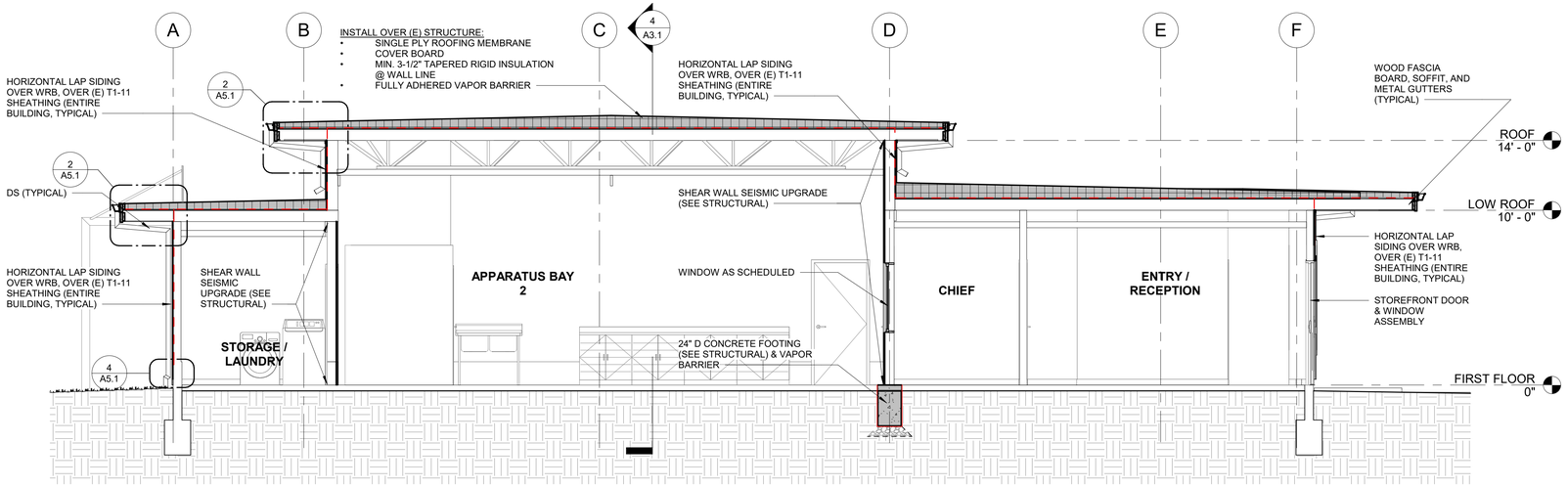
A2.3



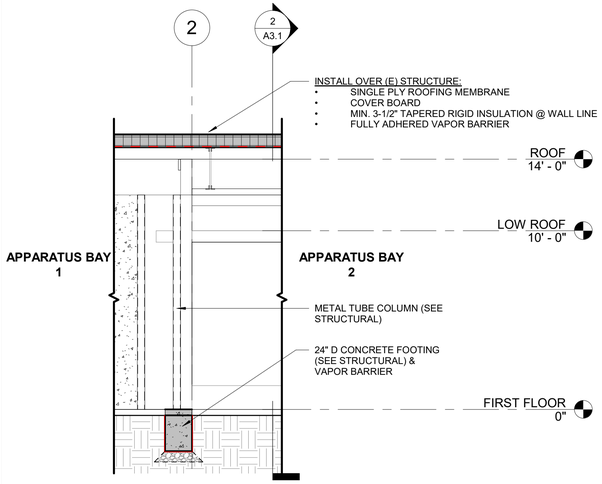
5 E-W SECTION @ OVERHEAD DOORS - BASE BID
1/4" = 1'-0"



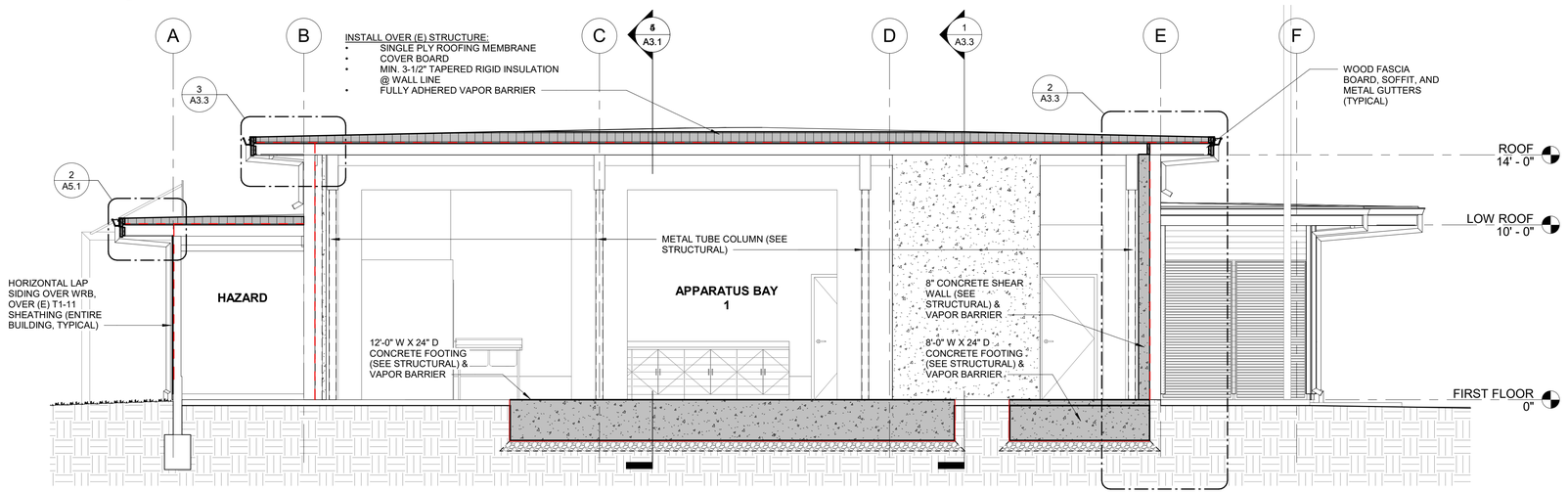
3 N-S SECTION @ SHEAR WALLS - BASE BID
1/4" = 1'-0"



2 N-S SECTION @ ENTRY / RECEPTION - BASE BID
1/4" = 1'-0"



4 E-W SECTION @ APPARATUS BAYS - BASE BID
1/4" = 1'-0"



1 N-S SECTION @ APPARATUS BAY 1 - BASE BID
1/4" = 1'-0"

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REVISIONS:

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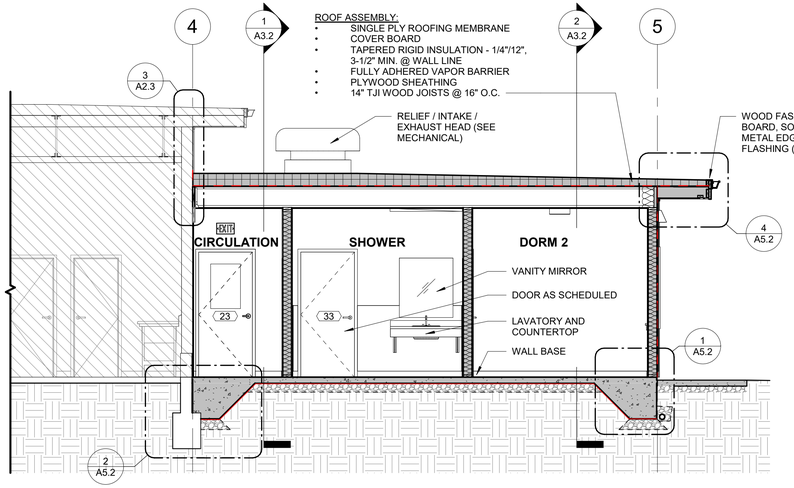
DATE: JANUARY 2024

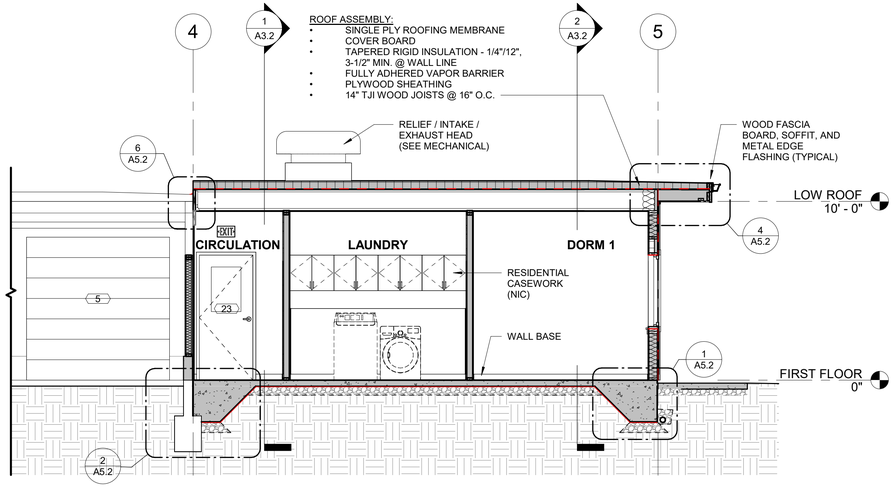
SHEET TITLE:
BUILDING SECTIONS - BASE BID

A3.1

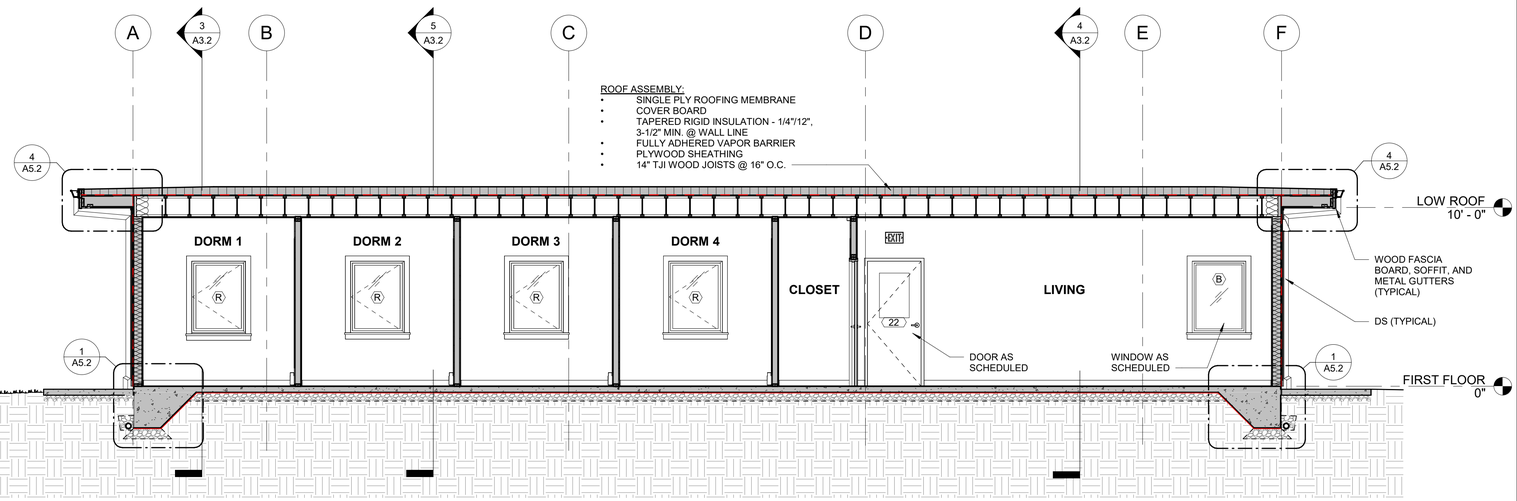
1/24/2024 8:27:32 AM C:\Users\Remote\Documents\21.59 North Bay Fire Station - Seismic Upgrade_gsalazarBHFTX.rvt



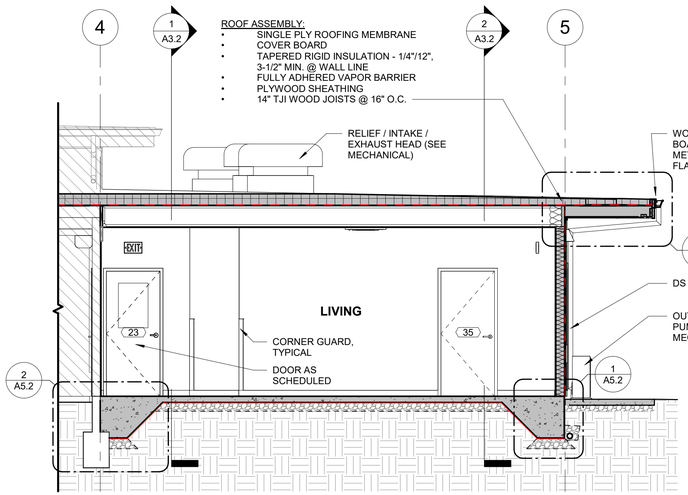
5 E-W SECTION AT SHOWER - ALTERNATE BID
1/4" = 1'-0"



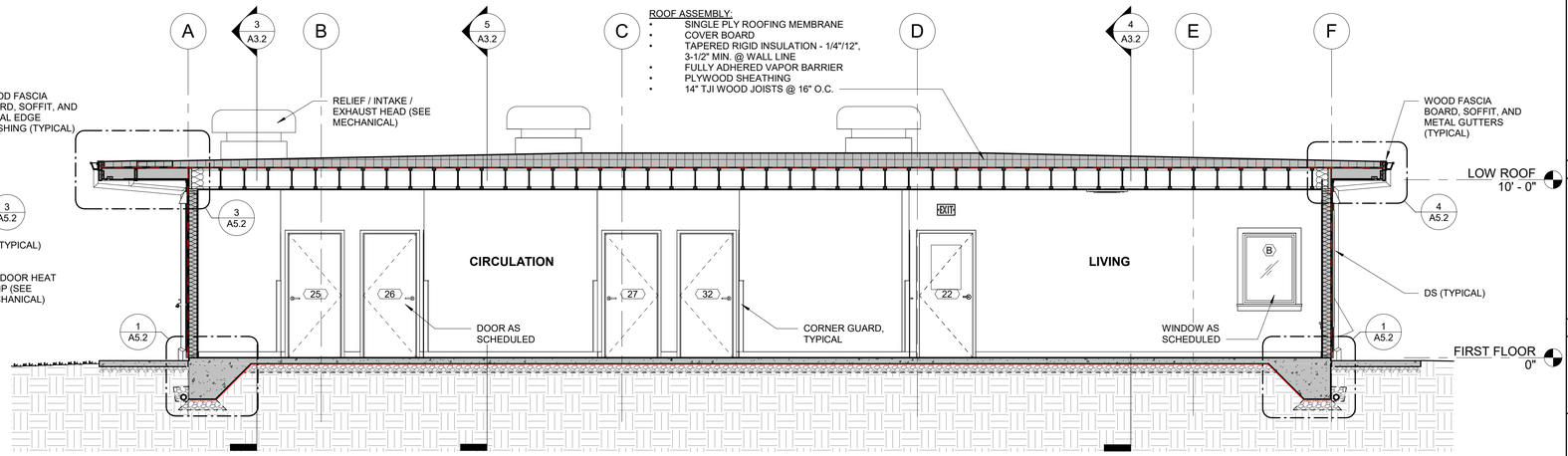
3 E-W SECTION AT LAUNDRY - ALTERNATE BID
1/4" = 1'-0"



2 N-S SECTION AT DORMS - ALTERNATE BID
1/4" = 1'-0"



4 E-W SECTION AT LIVING - ALTERNATE BID
1/4" = 1'-0"



1 N-S SECTION AT CIRCULATION - ALTERNATE BID
1/4" = 1'-0"

LEGEND
BASE BID AREA OF WORK

HGE ARCHITECTS
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REGISTERED ARCHITECT 2840 JOSEPH A. SLACK COOS BAY, OREGON STATE OF OREGON

PROJECT NO.: 21.59

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

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CONSTRUCTION

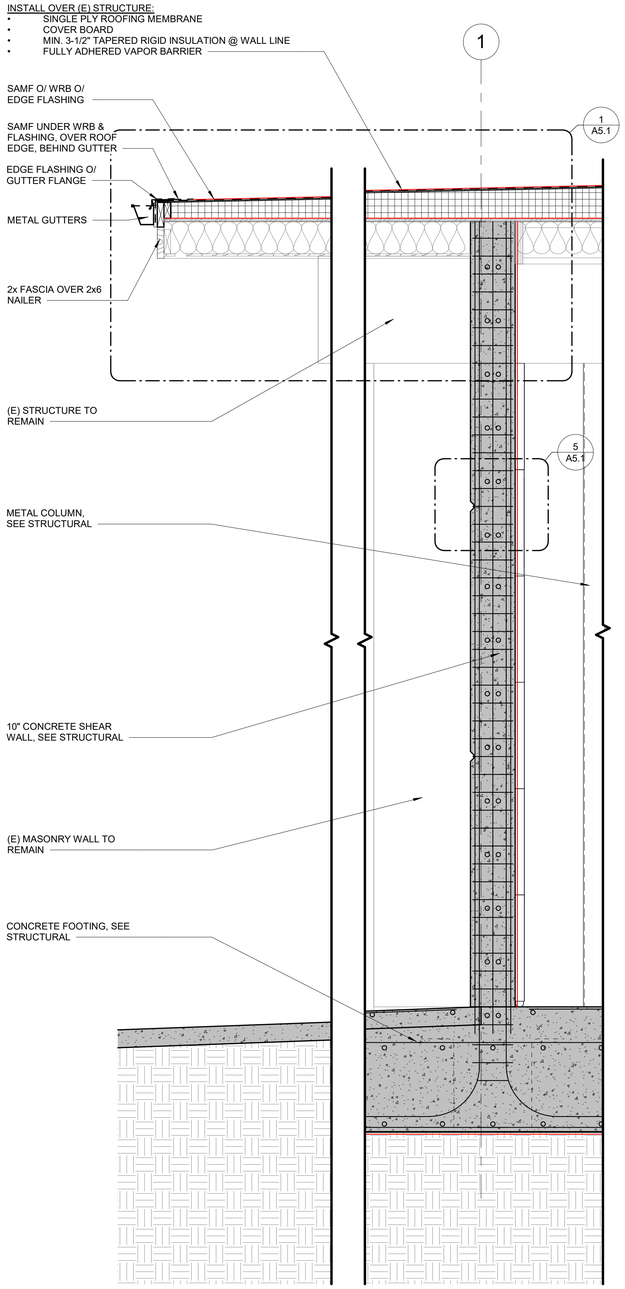
REVISIONS:

#	DATE	DESCRIPTION

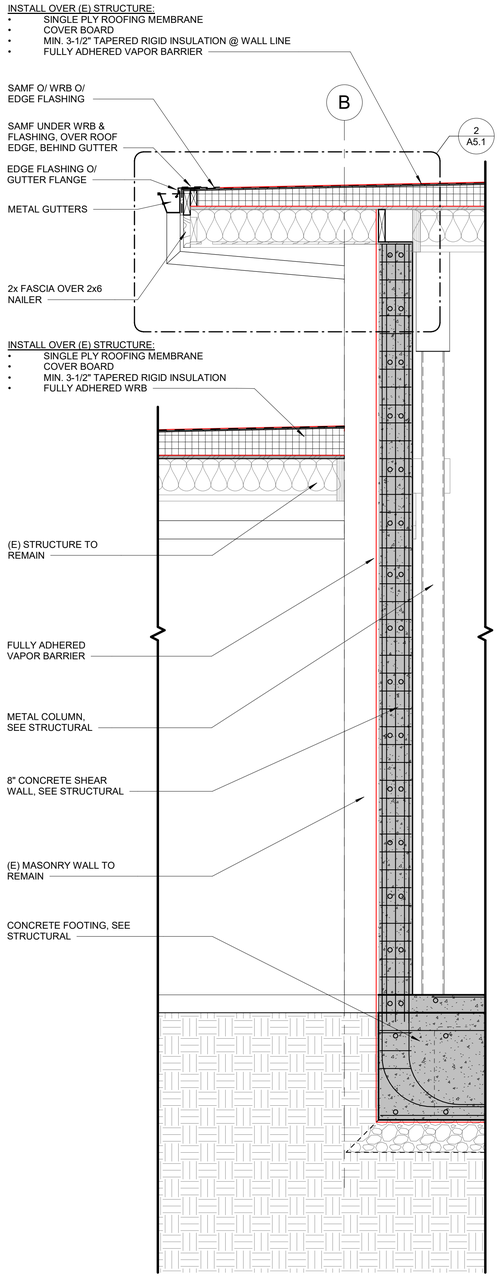
DATE: JANUARY 2024

SHEET TITLE:
BUILDING SECTIONS - ALTERNATE BID

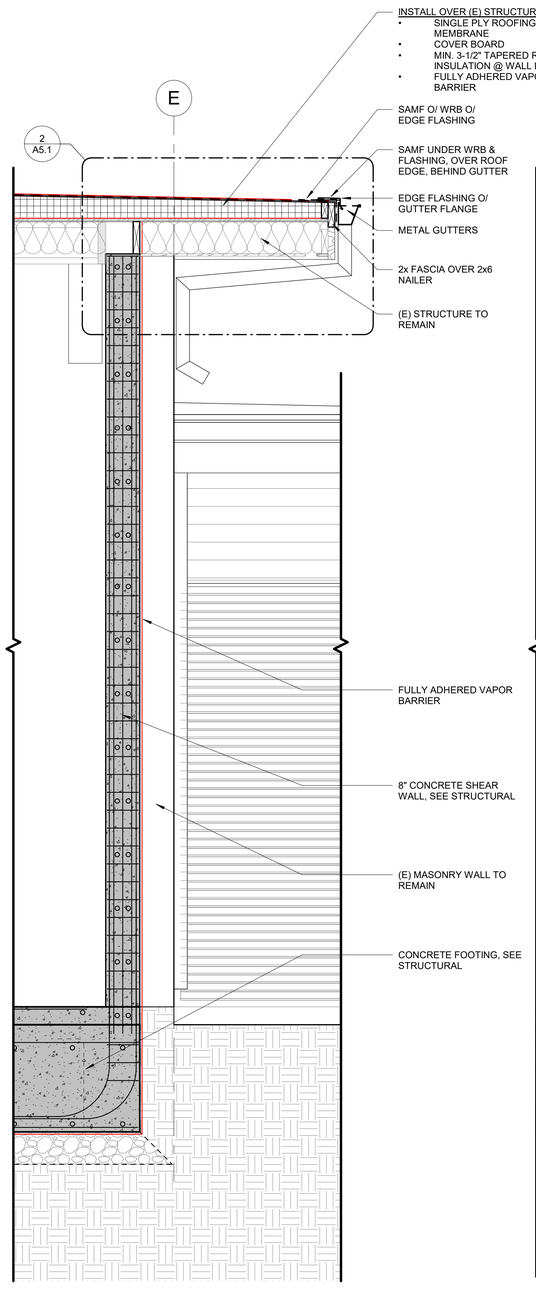
A3.2



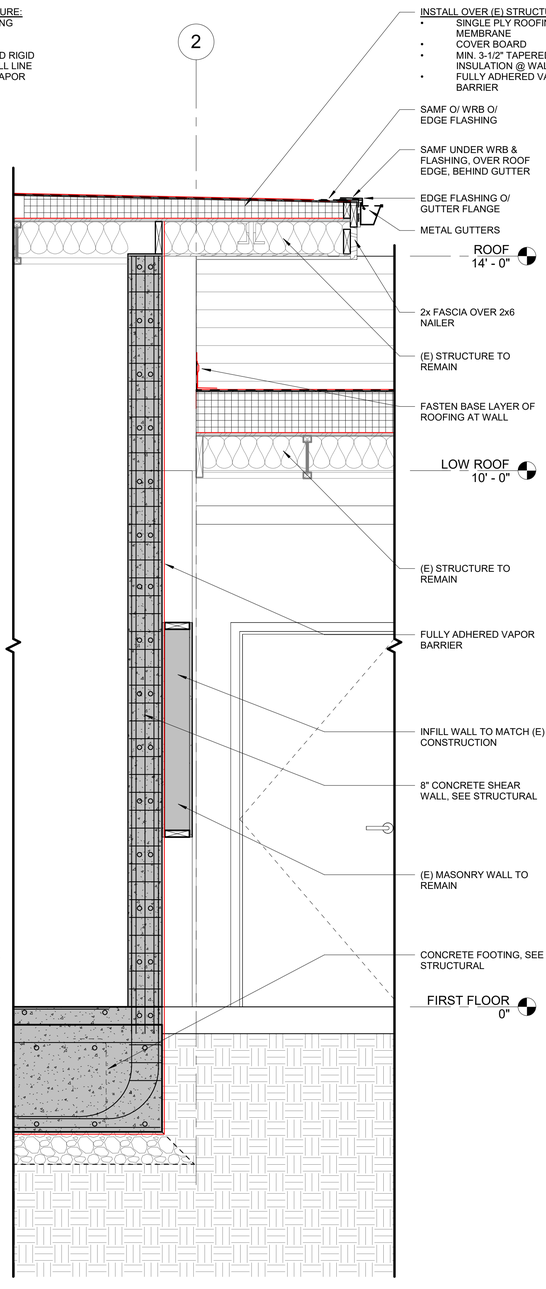
4 **CONC SHEAR WALL @ WEST WALL - BASE BID**
3/4" = 1'-0"



3 **CONC SHEAR WALL @ NORTH WALL - BASE BID**
3/4" = 1'-0"



2 **CONC SHEAR WALL @ SOUTH WALL - BASE BID**
3/4" = 1'-0"



1 **CONC SHEAR WALL @ EAST WALL - BASE BID**
3/4" = 1'-0"

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DATE: JANUARY 2024

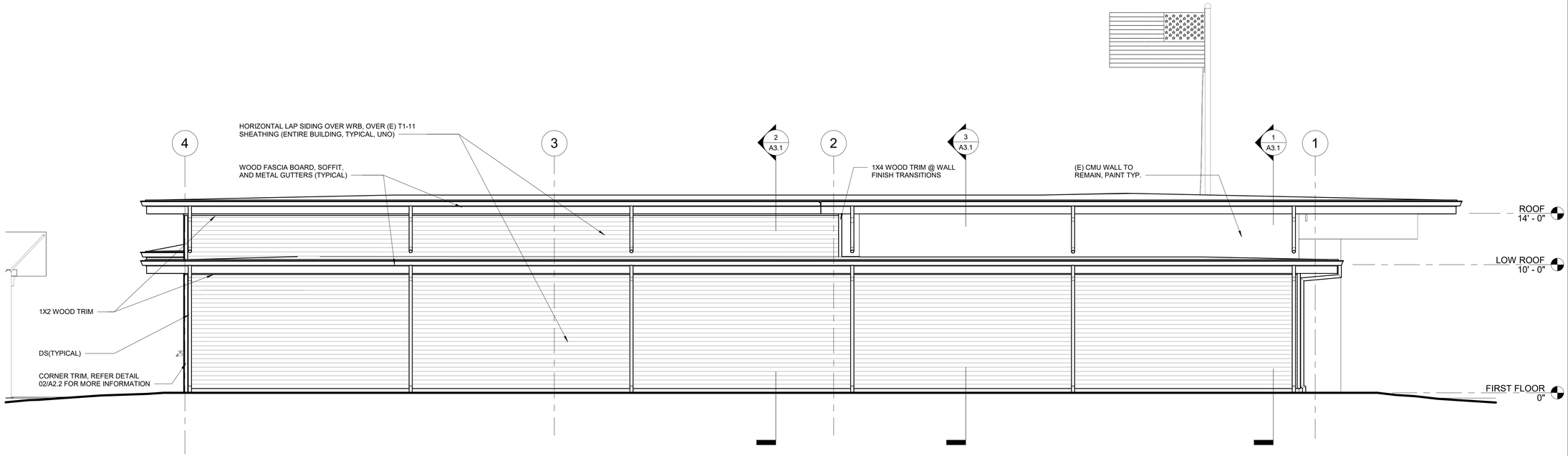
SHEET TITLE:
WALL SECTIONS - BASE BID

A3.3

1/24/2024 8:27:34 AM C:\Users\Remote\Documents\21.59 North Bay Fire Station - Seismic Upgrade_gsalazarBHFTX.rvt

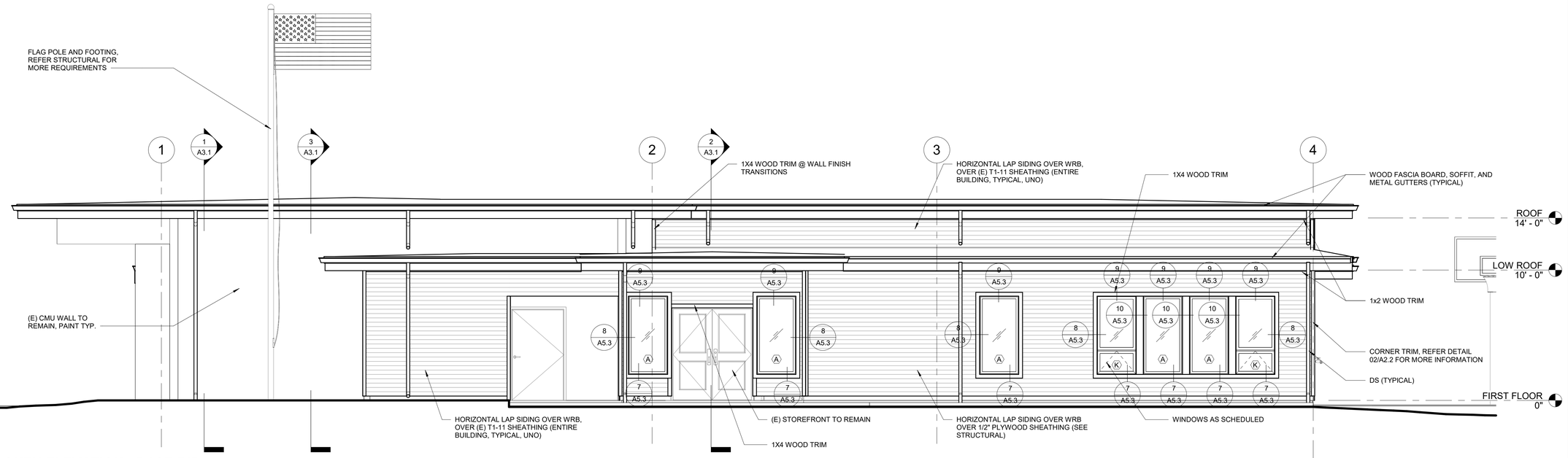
GENERAL NOTES

1. (E) T1-11 SIDING TO REMAIN UNO.
2. CONFIRM BLOCKING @ ALL PANEL JOINTS @ (E) T1-11 SIDING. ADD NAILING AS REQUIRED (SEE STRUCTURAL).
3. REPLACE ANY (E) T1-11 THAT IS ROTTEN OR STRUCTURALLY COMPROMISED W/ PLYWOOD SHEATHING OF MATCHING THICKNESS.



2 NORTH ELEVATION - BASE BID

1/4" = 1'-0"



1 SOUTH ELEVATION - BASE BID

1/4" = 1'-0"

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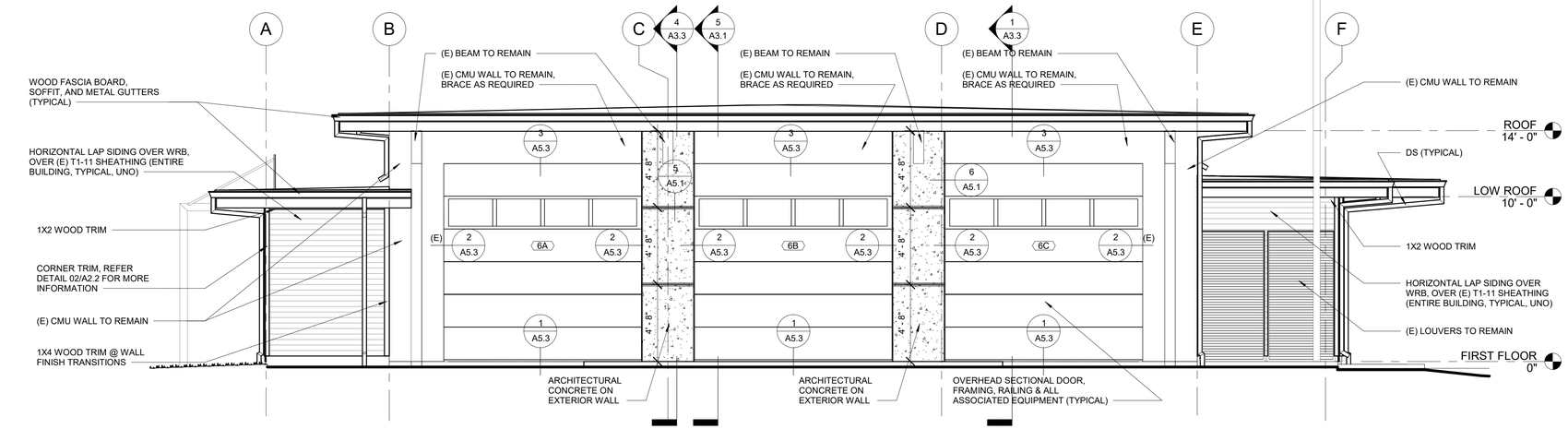
SHEET TITLE:

BUILDING ELEVATIONS - BASE BID

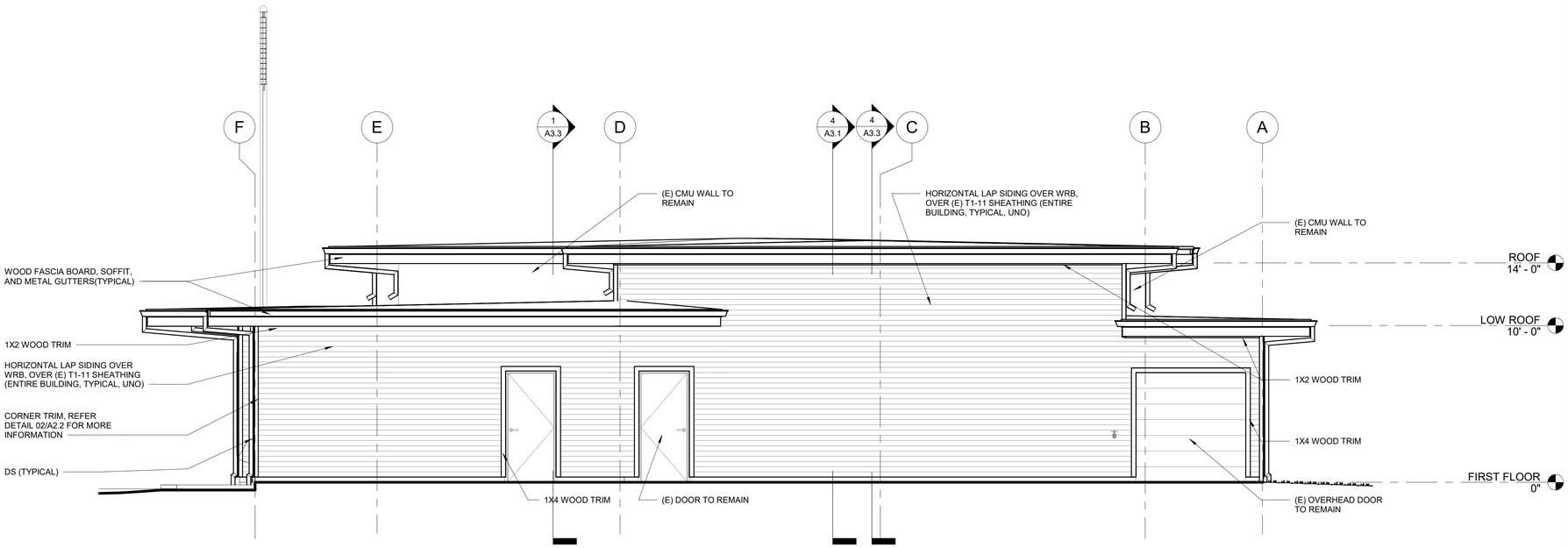
A4.1

GENERAL NOTES

1. (E) T1-11 SIDING TO REMAIN UNO.
2. CONFIRM BLOCKING @ ALL PANEL JOINTS @ (E) T1-11 SIDING, ADD NAILING AS REQUIRED (SEE STRUCTURAL).
3. REPLACE ANY (E) T1-11 THAT IS ROTTEN OR STRUCTURALLY COMPROMISED W/ PLYWOOD SHEATHING OF MATCHING THICKNESS.



2 WEST ELEVATION - BASE BID
1/4" = 1'-0"



1 EAST ELEVATION - BASE BID
1/4" = 1'-0"

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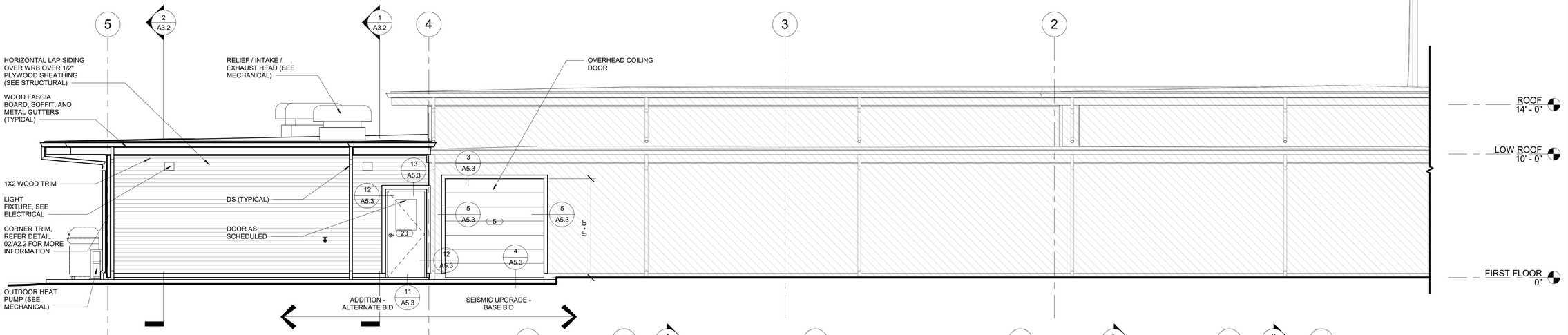
SHEET TITLE:
BUILDING ELEVATIONS - BASE BID

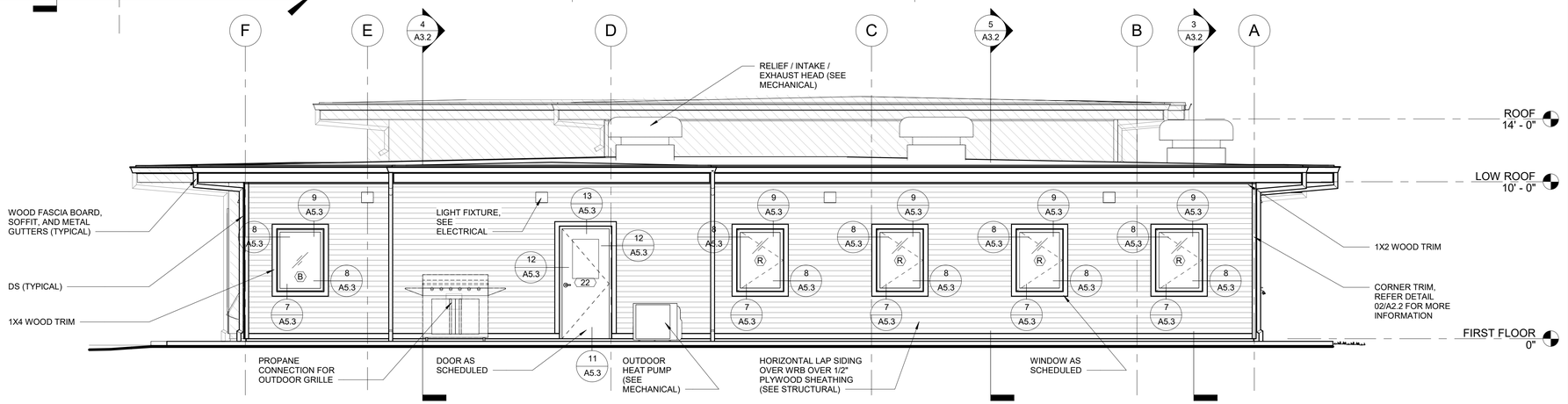
A4.2

LEGEND

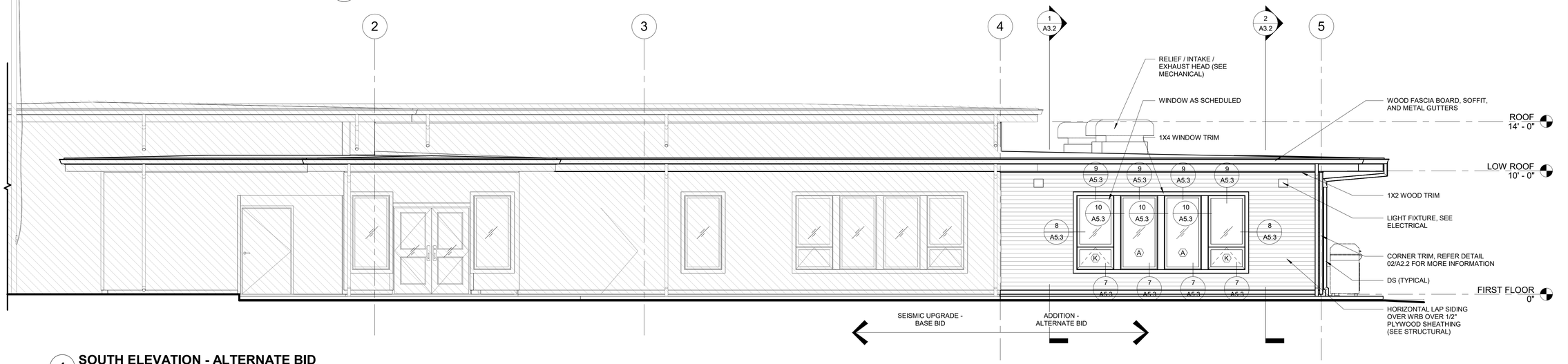
BASE BID AREA OF WORK



3 NORTH ELEVATION - ALTERNATE BID
1/4" = 1'-0"



2 EAST ELEVATION - ALTERNATE BID
1/4" = 1'-0"



1 SOUTH ELEVATION - ALTERNATE BID
1/4" = 1'-0"

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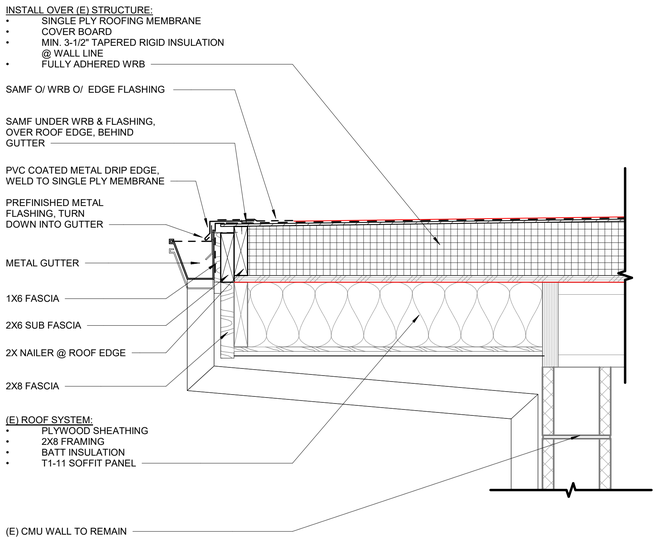
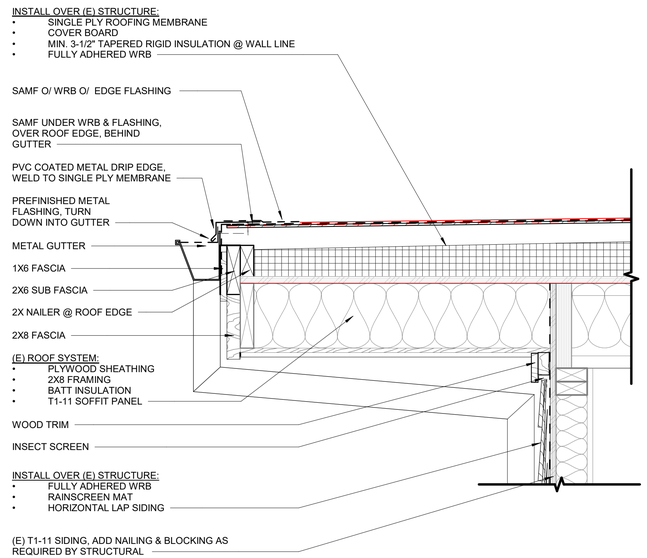
BUILDING ELEVATIONS - ALTERNATE BID

A4.3

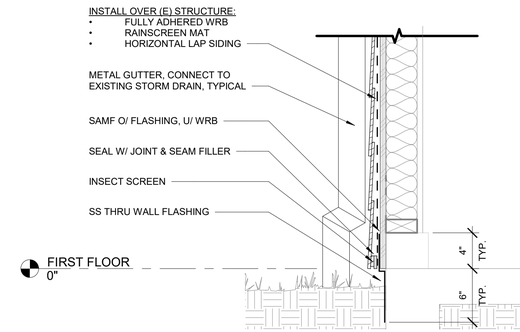
1/24/2024 8:27:38 AM C:\Users\Remote\Documents\21.59 North Bay Fire Station - Seismic Upgrade_gsalazarBHFTX.rvt



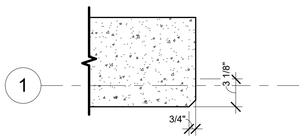
3 WALL EAVE @ CMU WALL - BASE BID
1 1/2" = 1'-0"



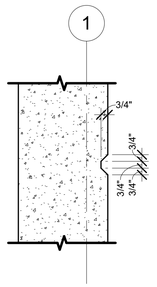
2 WALL EAVE @ HORIZONTAL SIDING - BASE BID
1 1/2" = 1'-0"



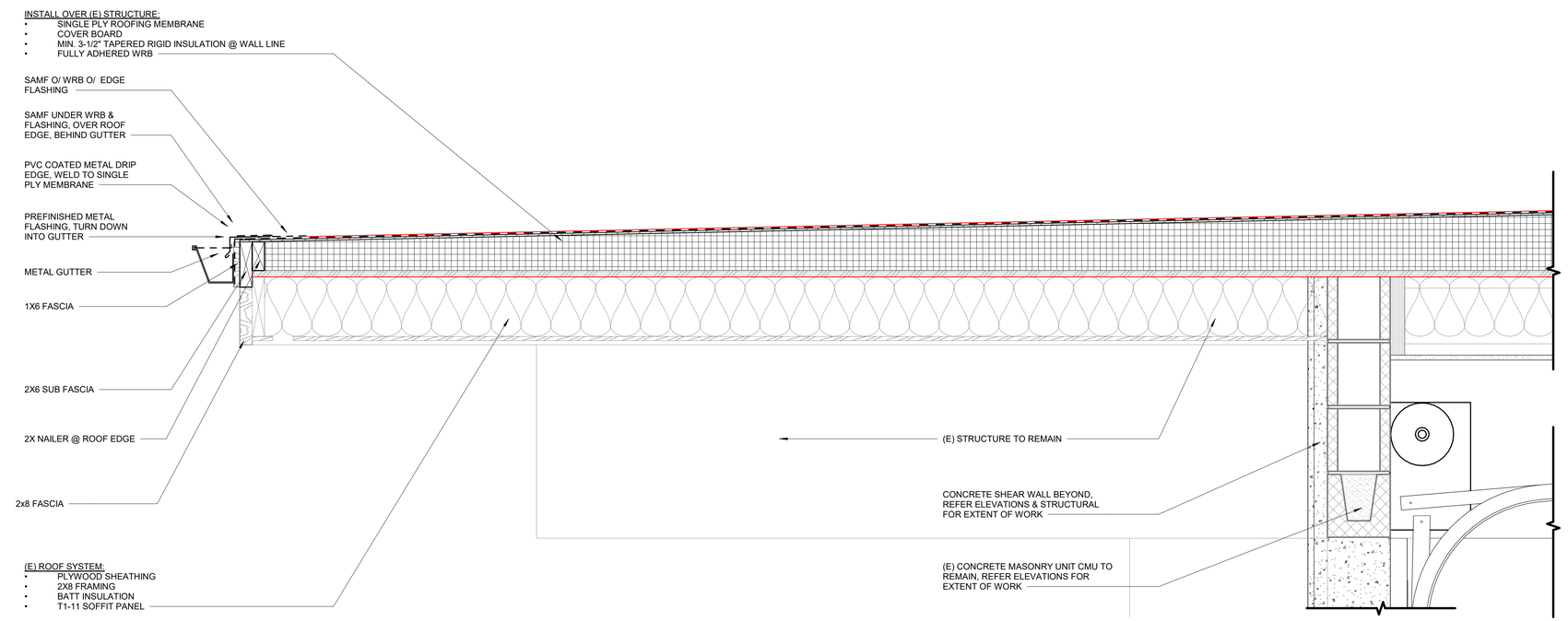
4 WALL BASE @ HORIZONTAL SIDING - BASE BID
1 1/2" = 1'-0"



6 CONCRETE WALL REVEAL - PLAN
1 1/2" = 1'-0"



5 CONCRETE WALL REVEAL - SECTION
1 1/2" = 1'-0"



1 WALL EAVE @ OVERHEAD DOORS - BASE BID
1 1/2" = 1'-0"

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REGISTERED ARCHITECT
2840
JOSEPH A. SLACK
COOS BAY, OREGON
STATE OF OREGON

PROJECT NO.: **21.59**

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

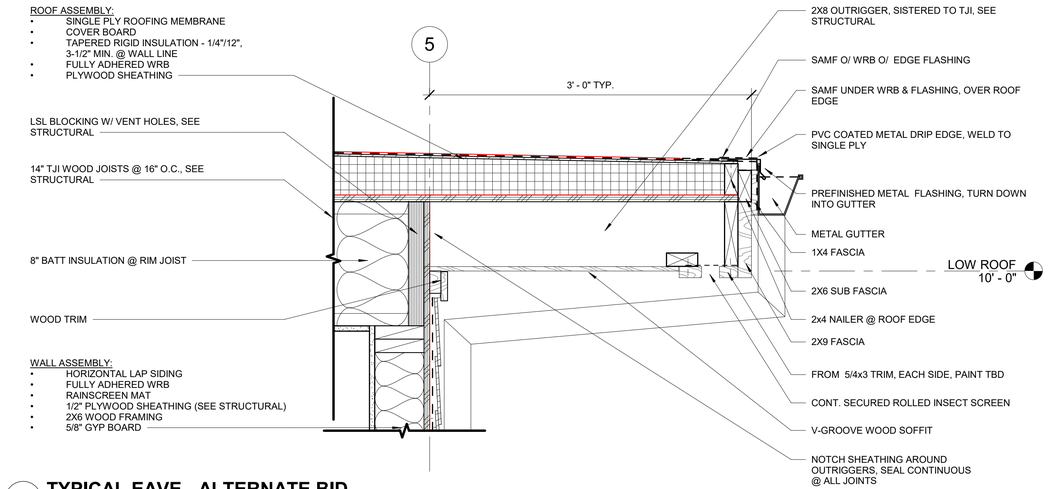
REVISIONS:

#	DATE	DESCRIPTION

DATE: JANUARY 2024

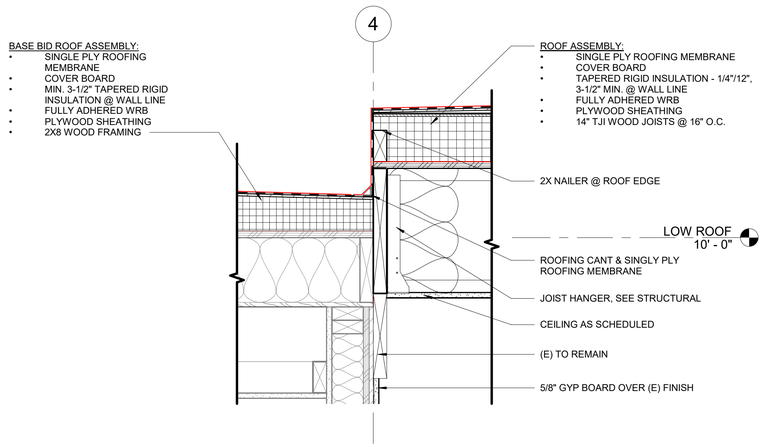
SHEET TITLE:
DETAILS - BASE BID

A5.1



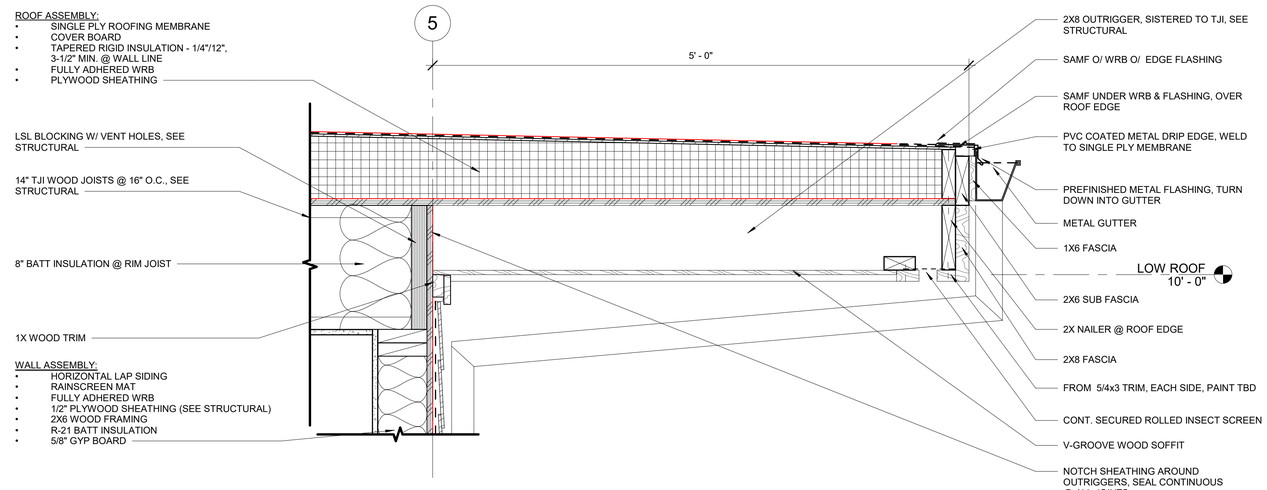
4 TYPICAL EAVE - ALTERNATE BID

1 1/2" = 1'-0"



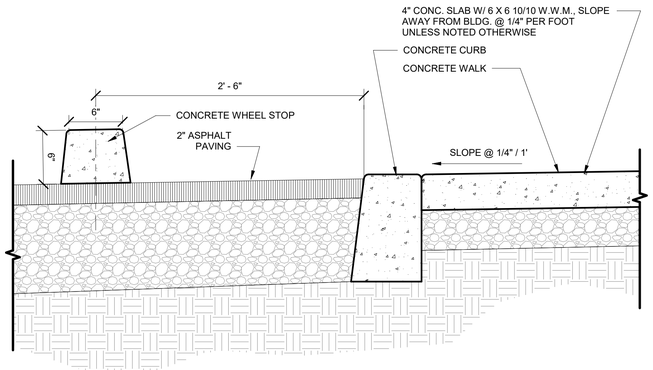
6 ROOF TRANSITION - ALTERNATE BID

1 1/2" = 1'-0"



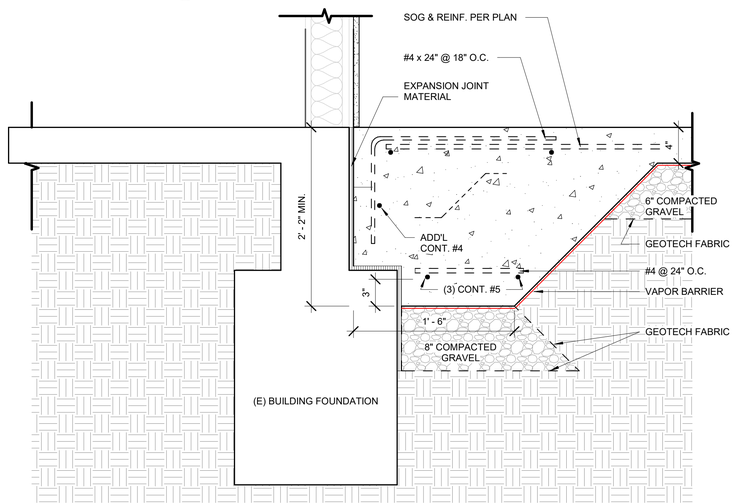
3 EAVE @ DOORS - ALTERNATE BID

1 1/2" = 1'-0"



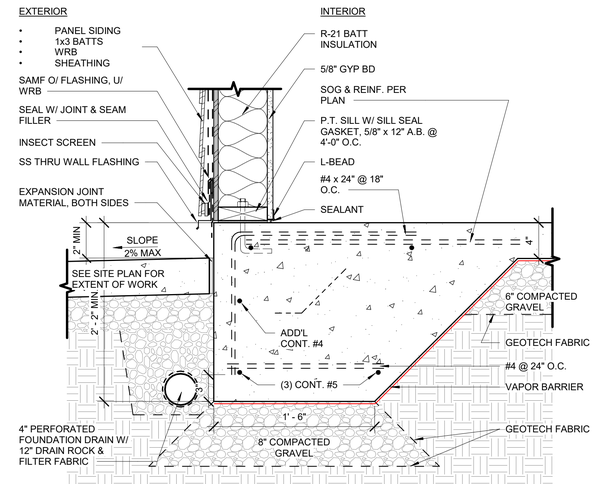
5 WHEEL STOP AT FLUSH SIDEWALK

1 1/2" = 1'-0"



2 SLAB EDGE @ EXISTING BUILDING - ALTERNATE BID

1 1/2" = 1'-0"



1 SLAB EDGE - ALTERNATE BID

1 1/2" = 1'-0"

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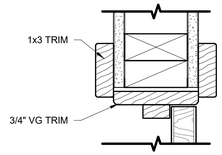
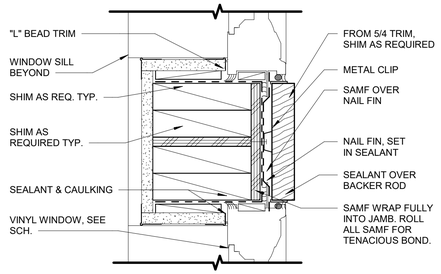
SHEET TITLE:
DETAILS - ALTERNATE BID

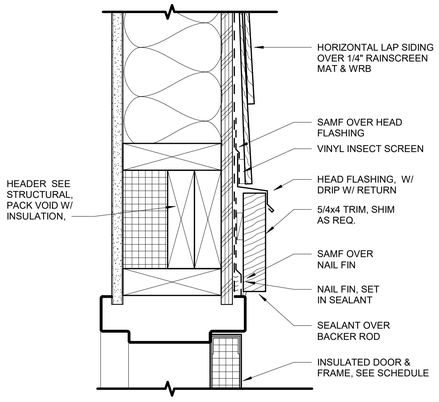
A5.2



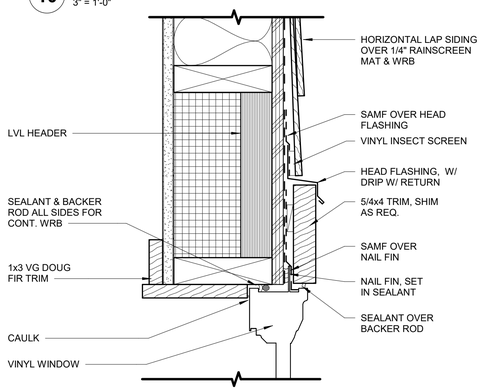
14 INT. WD DOOR HEAD/JAMB
3" = 1'-0"



10 TYP. WINDOW MULLION
3" = 1'-0"



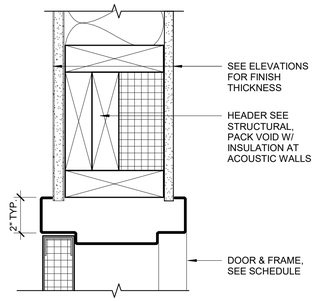
13 HM DOOR HEAD - ALTERNATE BID
3" = 1'-0"



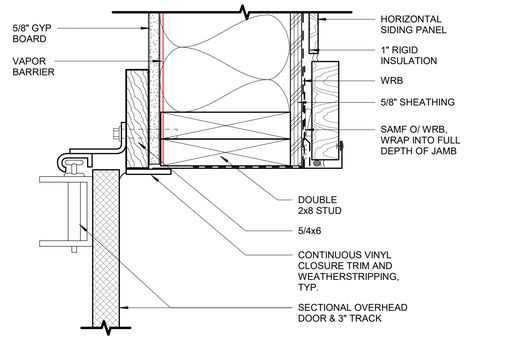
9 TYP. WINDOW HEAD
3" = 1'-0"

12 HM DOOR JAMB - ALTERNATE BID
3" = 1'-0"

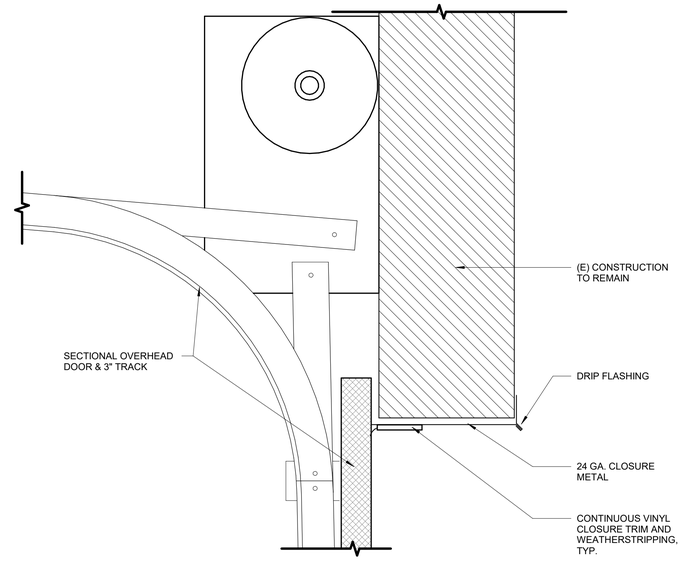
8 TYP. WINDOW JAMB
3" = 1'-0"



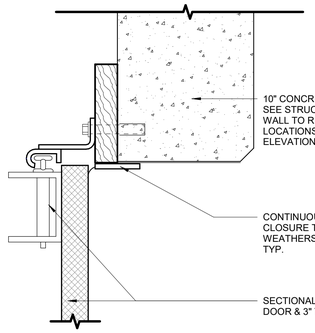
6 INT. HM DOOR HEAD/JAMB
3" = 1'-0"



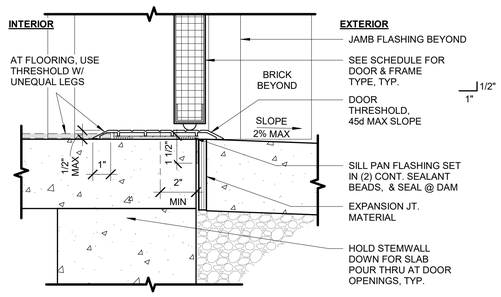
5 OVERHEAD DOOR JAMB @ SIDING
3" = 1'-0"



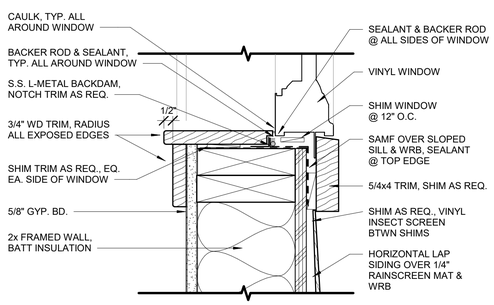
3 OVERHEAD DOOR HEAD
3" = 1'-0"



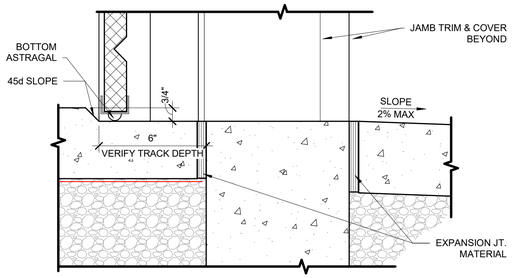
2 OVERHEAD DOOR JAMB @ CONCRETE - BASE BID
3" = 1'-0"



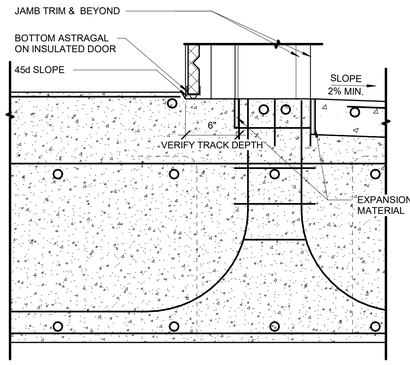
11 HM DOOR THRESHOLD - ALTERNATE BID
3" = 1'-0"



7 TYP. WINDOW SILL
3" = 1'-0"



4 SLAB @ GARAGE DOOR - ALTERNTE BID
3" = 1'-0"



1 SLAB @ GARAGE DOOR - BASE BID
1 1/2" = 1'-0"

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REGISTERED ARCHITECT 2840 JOSEPH A. SLACK COOS BAY, OREGON STATE OF OREGON

PROJECT NO.: **21.59**

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

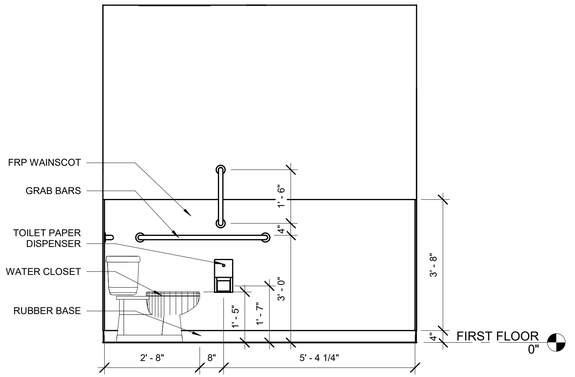
REVISIONS:

#	DATE	DESCRIPTION

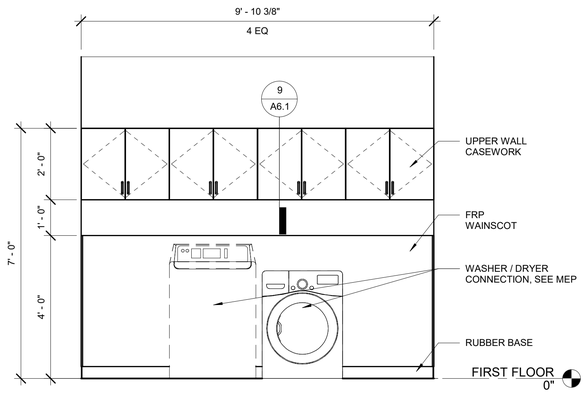
DATE: JANUARY 2024

SHEET TITLE:
OPENINGS DETAILS

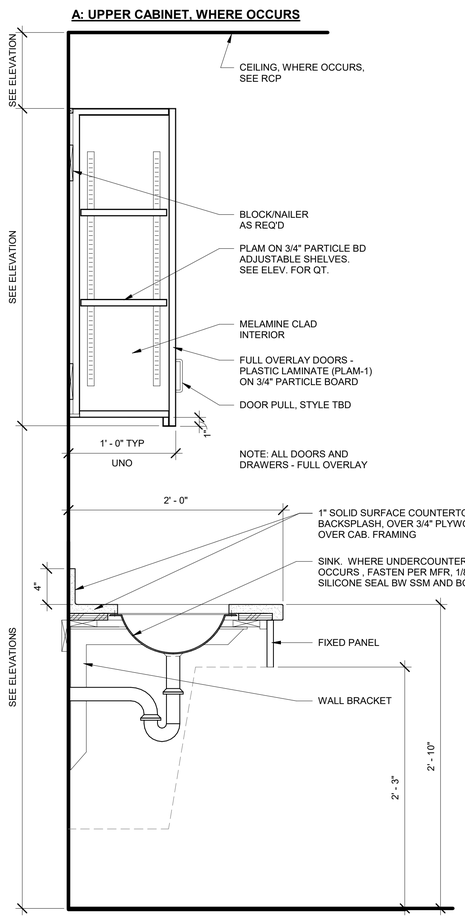
A5.3



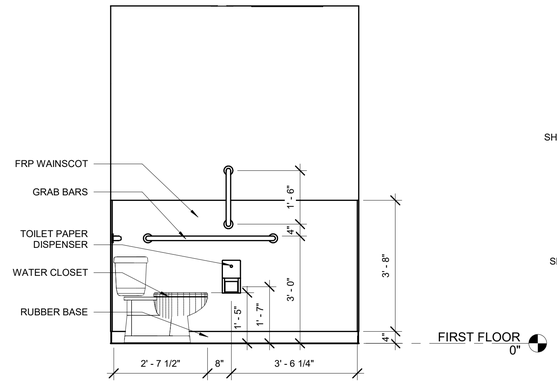
6 SHOWER 29 WEST - ALTERNATE BID
1/2" = 1'-0"



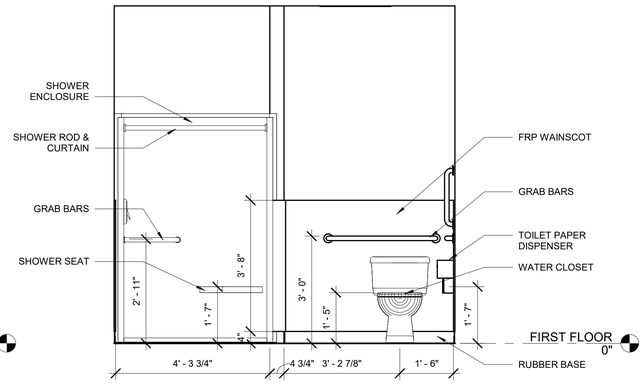
3 LAUNDRY 24 - ALTERNATE BID
1/2" = 1'-0"



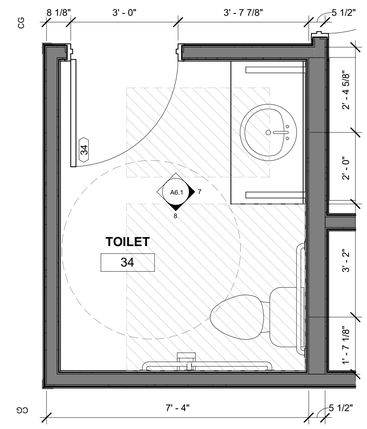
9 CABINETRY SECTION, TYPICAL - ALTERNATE BID
1 1/2" = 1'-0"



8 TOILET 34 SOUTH - ALTERNATE BID
1/2" = 1'-0"



5 SHOWER 29 SOUTH - ALTERNATE BID
1/2" = 1'-0"



2 ENLARGED PLAN - TOILET 34 - ALTERNATE BID
1/2" = 1'-0"

7 TOILET 34 EAST - ALTERNATE BID
1/2" = 1'-0"

4 SHOWER 29 NORTH - ALTERNATE BID
1/2" = 1'-0"

1 ENLARGED PLAN - SHOWER 33 - ALTERNATE BID
1/2" = 1'-0"

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NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
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DATE: JANUARY 2024

SHEET TITLE:
ENLARGED PLAN & INTERIOR ELEVATIONS - ALTERNATE BID

A6.1

1/24/2024 8:27:40 AM C:\Users\Remote\Documents\21.59 North Bay Fire Station - Seismic Upgrade_gsalazarBHFTX.rvt

ROOM FINISH SCHEDULE - ALTERNATE BID

Location		Floor		Wall				Ceiling		NOTES / REMARKS
ROOM NAME	No.	Floor Finish	Base	North	South	East	West	Ceiling Finish	Hgt.	
LIVING	22	LVT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
CIRCULATION	23	LVT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
LAUNDRY	24	RF	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
DORM 1	25	CPT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
DORM 2	26	CPT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
DORM 3	27	CPT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
DORM 4	32	CPT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
SHOWER	33	RF	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
TOILET	34	RF	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	
CLOSET	35	LVT	RB	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	GWB/PNT	9'-0"	

ROOM FINISH LEGEND

ACT - ACOUSTICAL CEILING TILE
CB - COVE BASE
CPT - CARPET
(E) - EXISTING
GWB - GYPSUM WALL BOARD
LVT - RESILIENT FLOORING, LUXURY VINYL TILE
PNT - PAINT
RB - RESILIENT BASE
RF - RESILIENT FLOORING
T - TILE
-- - NO WORK (UNO)

ROOM FINISH SCHEDULE - BASE BID

Location		Floor		Wall				Ceiling		NOTES / REMARKS
ROOM NAME	No.	Floor Finish	Base	North	South	East	West	Ceiling Finish	Hgt.	
HAZARD	1	-	-	-	-	-	-	-	-	
SCBA	2	-	-	-	-	-	-	-	-	
STORAGE	3	-	-	-	-	-	-	-	-	
STORAGE / LAUNDRY	4	-	-	-	GWB/PNT	-	-	-	-	
STORAGE	5	-	-	-	-	-	-	-	-	
APPARATUS BAY 1	6	SC	RB	PNT	PNT	PNT	PNT	-	-	
APPARATUS BAY 2	7	-	-	-	GWB/PNT	-	-	-	-	
BREAK	8	-	-	-	-	-	-	-	-	
COOLER	9	-	-	-	-	-	-	-	-	
STORAGE	10	-	-	-	-	-	PNT	-	-	
TOILET	11	-	-	-	-	-	-	-	-	
TOILET	12	-	-	-	-	-	-	-	-	
ENTRY / RECEPTION	13	SC	RB	PNT	-	-	PNT	-	-	
CHIEF	14	SC	RB	PNT	-	-	PNT	-	-	
OFFICE	15	-	-	-	-	-	-	-	-	
KITCHEN	16	-	-	-	-	-	-	-	-	
CONFERENCE ROOM	17	-	-	-	-	-	-	-	-	
CLOSET	18	-	-	--	-	-	-	-	-	
MECH	19	-	-	-	-	-	-	-	-	
TOILET	20	-	-	-	-	-	-	-	-	
TOILET	21	-	-	-	-	-	-	-	-	

WINDOW SCHEDULE - ALTERNATE BID

MARK	WINDOW SIZE		COUNT	TYPE	COMMENTS
	W	H			
A	3' - 0"	6' - 0"	2	FIXED	VINYL
B	3' - 0"	4' - 0"	1	FIXED	VINYL
K	3' - 0"	6' - 0"	2	FIXED O/ AWNING	VINYL
R	3' - 0"	4' - 0"	4	CASEMENT	VINYL

WINDOW SCHEDULE - BASE BID

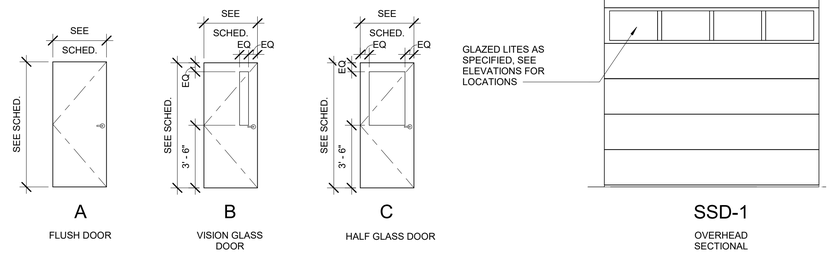
MARK	WINDOW SIZE		COUNT	TYPE	COMMENTS
	W	H			
A	3' - 0"	6' - 0"	5	FIXED	VINYL
C	6' - 0"	4' - 0"	1	FIXED	VINYL W/ SAFETY GLASS
K	3' - 0"	6' - 0"	2	FIXED O/ AWNING	VINYL

DOOR SCHEDULE - ALTERNATE BID

Room Name	Door No.	Size (WxH)	Type	Material	Frame	Hardware Group	Sill	Jamb	Head	Rating	Remarks
STORAGE	5	8' - 0" X 8' - 0"	SSD-1	STL	STL	--	4	5	3	-	SECTIONAL OVERHEAD DOOR
BREAK	8	3' - 0" X 7' - 0"	B	HM	HM	HW-2F		6	6	as Specified	20 MIN. RATED, RELOCATE KEY PAD TO DOOR 22
CONFERENCE ROOM	17	3' - 0" X 7' - 0"	B	HM	HM	HW-45		6	6	as Specified	
LIVING	22	3' - 0" X 7' - 0"	C	HM	HM	HW-55	11	12	13	-	
CIRCULATION	23	3' - 0" X 7' - 0"	C	HM	HM	HW-11	11	12	13	-	
LAUNDRY	24	8' - 0" X 7' - 0"	A	WD	HM	--	0	14	14	-	BIFOLD CLOSET
DORM 1	25	3' - 0" X 7' - 0"	P	WD	HM	HW-9		14	14	1 HR	
DORM 2	26	3' - 0" X 7' - 0"	P	WD	HM	HW-9		14	14	1 HR	
DORM 3	27	3' - 0" X 7' - 0"	P	WD	HM	HW-9		14	14	1 HR	
DORM 4	32	3' - 0" X 7' - 0"	P	WD	HM	HW-9		14	14	1 HR	
SHOWER	33	3' - 0" X 7' - 0"	A	WD	HM	HW-5		14	14	-	
TOILET	34	3' - 0" X 7' - 0"	A	WD	HM	HW-5		14	14	-	
CLOSET	35	3' - 0" X 7' - 0"	A	WD	WD	HW-2		14	14	-	

DOOR SCHEDULE - BASE BID

Room Name	Door No.	Size (WxH)	Type	Material	Frame	Hardware Group	Sill	Jamb	Head	Rating	Remarks
APPARATUS BAY 1	6A	12' - 0" X 12' - 0"	SSD-1	STL	STL		1	2	3	-	SECTIONAL OVERHEAD DOOR
APPARATUS BAY 1	6B	12' - 0" X 12' - 0"	SSD-1	STL	STL		1	2	3	-	SECTIONAL OVERHEAD DOOR
APPARATUS BAY 1	6C	12' - 0" X 12' - 0"	SSD-1	STL	STL		1	2	3	-	SECTIONAL OVERHEAD DOOR
CHIEF	14B	3' - 0" X 7' - 0"	A	WD	WD			13	13	-	



DOOR TYPES

1/4" = 1'-0"

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PROJECT NO.: **21.59**

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NORTH BEND, OR 97459

CONSTRUCTION

REVISIONS:

#	DATE	DESCRIPTION

DATE: JANUARY 2024

SHEET TITLE:
SCHEDULES

A7.1

STRUCTURAL - GENERAL NOTES

GENERAL REQUIREMENTS

GOVERNING CODE: The design and construction of this project is governed by the **"Oregon Structural Specialty Code (OSSC)", 2022 Edition**, hereafter referred to as the OSSC, as adopted and modified by the City of **North Bend, OR** understood to be the Authority Having Jurisdiction (AHJ).

REFERENCE STANDARDS: Refer to Chapter 35 of 2022 OSSC. Where other Standards are noted in the drawings, use the latest edition of the standard unless a specific date is indicated. Reference to a specific section in a code does not relieve the contractor from compliance with the entire standard.

DEFINITIONS: The following definitions cover the meanings of certain terms used in these notes:

(1) "**Architect/Engineer**" – The Architect of Record and the Structural Engineer of Record.

(2) "**Structural Engineer of Record**" **(SER)** – The structural engineer who is licensed to stamp & sign the structural documents for the project. The SER is responsible for the design of the Primary Structural System.

(3) **"Submit for review"** - Submit to the Architect/SER for review prior to fabrication or construction.

(4) "**Per Plan**" – Indicates references to the structural plans, elevations and structural general notes.

(5) "**Seismic Force Resisting System (SFRS)**" **– A** recognized structural system of components (beams, braces, drags, struts, collectors, diaphragms, columns, walls, etc) of the primary structure that are specially designed and proportioned to resist earthquake-induced ground motions and maintain stability of the structure. Fabrication and installation of components designated as part of the SFRS require the general contractor, subcontractor, or supplier who is responsible for any portion of SFRS fabrication or installation to comply with special requirements (including, but not limited to, material control, compliance certifications, personnel qualifications, documentation, reporting requirements, etc) and to provide the required Quality Control including the required coordination of Special Inspections (Quality Assurance – QA). Special provisions apply to any member designated as part of the SFRS. Refer to plans, elevations, details, Design Criteria and Symbols and Legends for applicable members and connections.

(6) **"Specialty Structural Engineer" (SSE) –** A professional engineer (PE or SE), licensed in the State where the project is located, (typically not the SER), who performs specialty structural engineering services for selected specialty-engineered elements identified in the Contract Documents, and who has experience and training in the Specialty. Documents stamped and signed by the SSE shall be completed by or under the direct supervision of the SSE.

(7) **"Bidder-designed" –** Components of the structure that require the general contractor, subcontractor, or supplier who is responsible for the design, fabrication and installation of specialty-engineered elements identified in the Contract Documents to retain the services of an SSE. Submittals of "Bidder-designed" elements shall be stamped and signed by the SSE.

SPECIFICATIONS: Refer to the project specifications issued as part of the contract documents for information supplemental to these drawings.

OTHER DRAWINGS: Refer to the architectural, mechanical, electrical, civil and plumbing drawings for additional information including but not limited to dimensions, elevations, slopes, door and window openings, non-bearing walls, stairs, finishes, drains, waterproofing, railings, mechanical unit locations, and other nonstructural items.

STRUCTURAL DETAILS: The structural drawings are intended to show the general character and extent of the project and are not intended to show all details of the work. Use entire detail sheets and specific details referenced in the plans as "typical" wherever they apply. Similarly, use details on entire sheets with "typical" in the name wherever they apply.

STRUCTURAL RESPONSIBILITIES: The structural engineer (SER) is responsible for the strength and stability of the primary structure in its completed form.

COORDINATION: The Contractor is responsible for coordinating details and accuracy of the work; for confirming and correlating all quantities and dimensions; for selecting fabrication processes; for techniques of assembly; and for performing work in a safe and secure manner.

EXISTING CONDITIONS: Information shown on the drawings related to existing conditions represent the present knowledge, but without guarantee of accuracy. Report conditions that conflict with contract documents to the architect or SER. Do not deviate from the contract documents without written direction from the architect and/or SER. All existing dimensions and information shall be field verified prior to fabrication as required to coordinate with new construction.

NEW CONSTRUCTION: The contractor shall remove all interfering items for new construction and shall repair or replace all removed items to match the existing conditions in accordance with the architectural drawings. New construction elements shall be designed and installed per current International Building Code 2021, hereafter referred to as OSSC as allowed by IEBC.

MEANS, METHODS and SAFETY REQUIREMENTS: The contractor is responsible for the means and methods of construction and all job-related safety standards such as OSHA and DOSH (Department of Occupational Safety and Health). The contractor is responsible for means and methods of construction related to the intermediate structural conditions (i.e., movement of the structure due to moisture and thermal effects; construction sequence; temporary bracing, etc.).

BRACING/SHORING DESIGN ENGINEER: The contractor shall at their discretion employ an SSE, a registered professional engineer for the design of any temporary bracing and shoring. Submit construction sequence to Architect/Engineer for review.

TEMPORARY SHORING, BRACING: The contractor is responsible for the strength and stability of the structure during construction and shall provide temporary shoring, bracing and other elements required to maintain stability until the structure is complete. It is the contractor's responsibility to be familiar with the work required in the construction documents and the requirements for executing it properly.

CONSTRUCTION LOADS: Loads on the structure during construction shall not exceed the design loads as noted in DESIGN CRITERIA & LOADS below or the capacity of partially completed construction as determined by the Contractor's SSE for Bracing/Shoring.

CHANGES IN LOADING: The contractor has the responsibility to notify the SER of any architectural, mechanical, electrical, or plumbing load imposed onto the structure that differs from, or that is not documented on the original Contract Documents (architectural / structural / mechanical / electrical or plumbing drawings). Provide documentation of location, load, size and anchorage of all undocumented loads in excess of **400** pounds. Provide marked-up structural plan indicating locations of any new equipment or loads. Submit plans to the Architect/Engineer for review prior to installation.

NOTE PRIORITIES: Plan and detail notes and specific loading data provided on individual plans and detail drawings supplements information in the Structural General Notes.

DISCREPANCIES: In case of discrepancies between the General Notes, Specifications, Plans/Details or Reference Standards, the Architect/Engineer shall determine which shall govern. Discrepancies shall be brought to the attention of the Architect/Engineer before proceeding with the work. Should any discrepancy be found in the Contract Documents, the Contractor will be deemed to have included in the price the most expensive way of completing the work, unless prior to the submission of the price, the Contractor asks for a decision from the Architect as to which shall govern. Accordingly, any conflict in or between the Contract Documents shall not be a basis for adjustment in the Contract Price.

SITE VERIFICATION: The contractor shall verify all dimensions and conditions at the site. Conflicts between the drawings and actual site conditions shall be brought to the attention of the Architect/Engineer before proceeding with the work.

ALTERNATES: Alternate products of similar strength, nature and form for specified items may be submitted with adequate technical documentation (proper test report, etc.) to the Architect/Engineer for review. Alternate materials that are submitted without adequate technical documentation or that significantly deviate from the design intent of materials specified may be returned without review. Alternates that require substantial effort to review will not be reviewed unless authorized by the Owner.

NARRATIVE: Seismic retrofit of an existing fire station using ASCE 41-17 as part of the Oregon Grant program.

ADDITIONS/ALTERATIONS/REPAIRS: Additions, alterations, and/or repairs to the existing structure has been analyzed for additional loading and/or modification due to the addition, the alteration or the repair. All affected existing member have been analyzed or reinforced as required per IEBC.

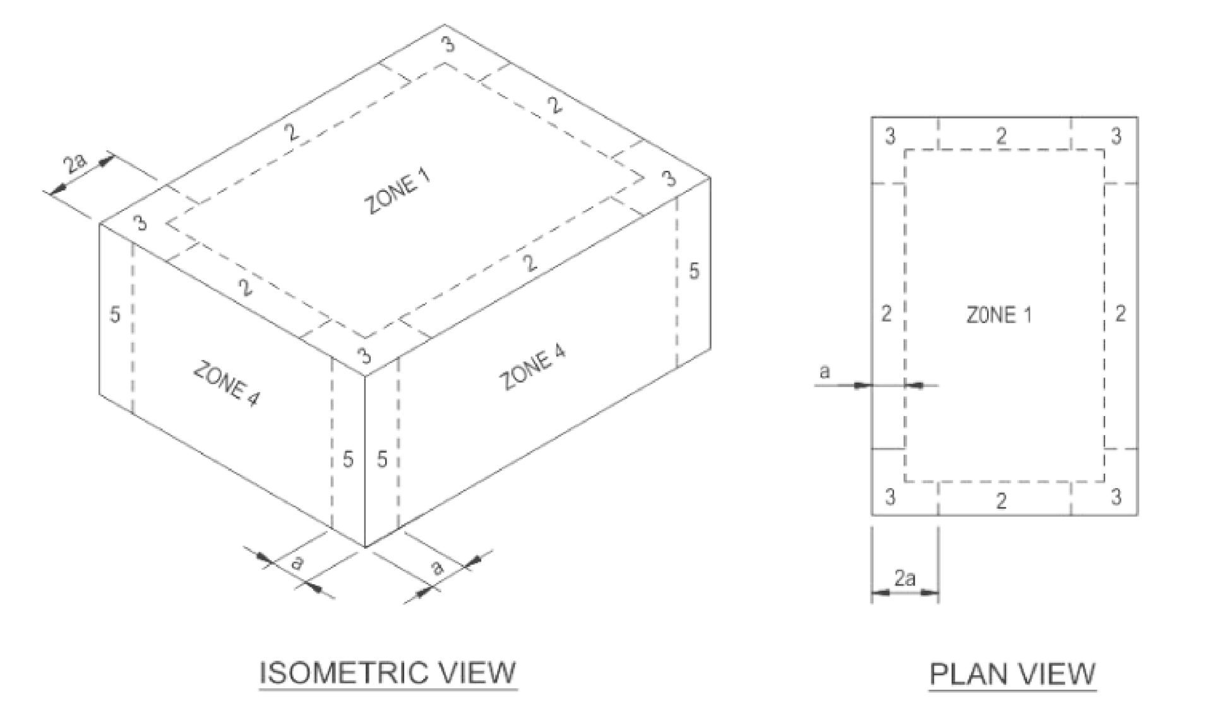
All Demolition or removal of architectural, mechanical or structural elements shall not damage structural items to remain.

DESIGN CRITERIA AND LOADS

OCCUPANCY:	Risk Category of Building per 2022 OSSC Table 1604.5 =	**IV**

WIND DESIGN:	**MAIN WIND FORCE RESISTING SYSTEM**	
	Ultimate Design Wind Speed, V_{ULT} (MPH)	**106**
	Exposure Category	**B**
	Internal Pressure Coefficient **Cpi =**	**+/- 0.18**
	Topographic Factor **Kzt =**	**1.0**
	Wind Analysis procedure used:	**Directional**

WIND DESIGN: COMPONENTS & CLADDING PRESSURES FOR DESIGN (PSF, ULTIMATE)



EFFECTIVE WIND AREA (SQ. FT)

a = 3'-0"	10	20	50	100
ZONE 1	32.2	30.1	27.3	25.2
ZONE 2	18.5	18.5	18.5	18.5
ZONE 3	42.5	39.8	36.2	33.4
ZONE 4	21.9	21	19.8	18.9
ZONE 5	27.1	25.3	22.8	21.0

(1) Components and Cladding Wind Pressures are based on ASCE 7-16 Chapter 30 Part 3: Buildings with h > 60 ft.
(2) Components and Cladding zone locations are based on ASCE 7-16 Table 30.5-1 for Flat Roofs θ < 10 deg.
(3) For parapets around the perimeter of the roof equal to or higher than 3 ft, Zone 3 shall be treated as Zone 2.
(4) All Parapet Components and Cladding Wind Pressures shall be determined through ASCE 7-16 Figure 30.6-2.

SEISMIC DESIGN:	**Seismic Design Category: SDC =**	**D**
	Basic Structural System	**Bearing Wall**
	Seismic Force Resisting System	**Concrete and Wood Shear Walls**
	Site Classification per [IEBC 301.1.4.1] [IEBC 301.1.4.2]& ASCE 41-13, Ch. 2.4 **Site Class =**	**D**
	Basic Safety Earthquakes(BSE):	
	Spectral Acceleration:	
	BSE-1E, Immediate Occupancy – Base Shear	**360k**
	Spectral Response Acceleration (Short Period) **S_{xs} =**	**0.375**
	Spectral Response Acceleration (1-Second Period) **S_{x1} =**	**0.228**
	BSE-2E, Life Safety – Base Shear	**158.4k**
	Spectral Response Acceleration (Short Period) **S_{xs} =**	**1.315**
	Spectral Response Acceleration (1-Second Period) **S_{x1} =**	**0.957**
	Seismic Analysis procedure used:	Linear Static Procedure

SNOW LOAD: (1)	Flat Roof Snow Load, (PSF) p_f =	**25** (2)
	Snow Drift Loading required by Authority Having Jurisdiction?	**No**
	Snow Load Importance Factor **I_s =**	**1.0** (3)
	Ground Snow Load, (PSF) **p_g =**	**2**
	Snow Exposure Factor **C_e =**	**B**
	Thermal Factor **C_t =**	**1.0**
	See Roof Plan for Drift Loading	

(1) Snow Load is un-reducible and includes 5 psf rain-on-snow surcharge where ground snow load is greater than zero and 20 psf or less per ASCE 7-16 Section 7.10.
(2) Snow Load based on SEAO Oregon Snow Loading.
(3) Snow Load Importance Factor per ASCE 7-16 Table 1.5-2.

DESIGN LIVE LOADS	**AREA**	**LIVE LOADS (PSF) UNO**	**REMARKS & FOOTNOTES** (1)
	See structural loading plans for area loads and line loads. Loads listed below are for miscellaneous items.		
	Lobbies	100	2000 lbs
	Corridors at First Floor	100	
	Dorms	40	
	Roofs	20 PSF or 300 LB	Area load is reducible. Point load per note (2), see above for Snow Load

(1) Unless otherwise noted, point loads to be distributed over a 2.5ft x 2.5ft area and located to produce maximum load effects on structural members.

SUBMITTALS

SUBMIT FOR REVIEW: SUBMITTALS of shop drawings, product data and mill tests are required for items noted in the individual materials sections and for *bidder designed* elements.

SUBMITTAL REVIEW PERIOD: Submittals shall be made in time to provide a minimum of TWO WEEKS or 10 WORKING DAYS for review by the Architect/Engineer prior to the onset of fabrication.

GENERAL CONTRACTOR'S PRIOR REVIEW: Prior to submission to the Architect/Engineer, the Contractor shall review the submittal for completeness. Dimensions and quantities are not reviewed by the SER, and therefore, must be verified by the General Contractor. Contractor shall provide any necessary dimensional details requested by the Detailer and provide the Contractor's review stamp and signature before forwarding to the Architect/Engineer.

SHOP DRAWING REVIEW: Once the contractor has completed their review, the SER will review the submittal for general conformance with the design concept and the contract documents of the building and will stamp the submittal accordingly. Markings or comments shall not be construed as relieving the contractor from compliance with the project plans and specifications, nor departures there from. The SER will return submittals in the form they are submitted in (either hard copy or electronic). For hard copy submittals, the contractor is responsible for submitting the required number of copies to the SER for review.

SHOP DRAWING DEVIATIONS: When shop drawings (component design drawings) differ from or add to the requirements of the structural drawings they shall be designed and stamped by the responsible SSE.

DEFERRED SUBMITTALS

BIDDER-DESIGNED ELEMENTS

Submit "Bidder-Designed" deferred submittals to the Architect and SER for review. The deferred submittals shall also be submitted to the city for approval, if required by the city.

Design of prefabricated, "bidder designed", manufactured, pre-engineered, or other fabricated products shall comply with the following requirements:

(1) Design considers tributary dead, live, wind and earthquake loads in combinations required by OSSC.
(2) Design within the Deflection Limits noted herein and as specified or referenced in the OSSC.
(3) Design shall conform to the specifications and reference standards of the governing code.
(4) Submittal shall include:
   a. Calculations prepared, stamped and signed by the SSE demonstrating code conformance.
   b. Engineered component design drawings are prepared, stamped and signed by the SSE.
   c. Product data, technical information and manufacturer's written requirements and Agency approvals as applicable.
   d. SSE may submit to the Architect/Engineer, a request to utilize relevant alternate design criteria of similar nature and generally equivalency which is recognized by the Code and acceptable to the Authority Having Jurisdiction. Submit adequate documentation of design.

INSPECTIONS, QUALITY ASSURANCE VERIFICATIONS AND TEST REQUIREMENTS

INSPECTIONS: Foundations, footings, under slab systems and framing are subject to inspection by the Building Official in accordance with OSSC 110.3. Contractor shall coordinate all required inspections with the Building Official.

SPECIAL INSPECTIONS, VERIFICATIONS and TESTS: Special Inspections, Verifications and Testing shall be done in accordance with OSSC Chapter 17, the STATEMENT AND SCHEDULES OF SPECIAL INSPECTIONS listed in these drawings.

STRUCTURAL OBSERVATION: per OSSC Section 1704.6

Structural Observation is the visual observation of the structural system by a registered design professional for general conformance to the approved construction documents. It is not always required on a project, does not include or waive the responsibility for the special inspections and tests required by a Special Inspector per OSSC Chapter 17, is not continuous, and does not certify conformance with the approved construction documents.

Structural Observation for this project is required per OSSC Section 1704.6. Contractor shall notify the SER in a timely manner to allow required Structural Observations to occur. Reports will be distributed to the Architect, the Contractor, Special Inspector and the Authority Having Jurisdiction.

The frequency and extent of observations is at the discretion of the structural observer. Only significant stages of construction identified by the Structural Observer require observation. For repetitive or similar structural elements identified as significant, only the first element of a stage requires observation unless noted otherwise. The following significant stages of construction require observation: prior to foundation concrete placement, and after roof diaphragm is complete prior to roofing.

CONTRACTOR RESPONSIBILITY: Prior to issuance of the building permit, the Contractor is required to provide the Authority Having Jurisdiction a signed, written acknowledgement of the Contractor's responsibilities associated with the above Statement of Special Inspections addressing the requirements listed in OSSC Section 1704.4. Contractor is referred to OSSC Sections 1705.12.5 and 1705.12.6 for architectural and MEP building systems that may be subject to additional inspections (based on the building's designated Seismic Design Category listed in the CRITERIA), including anchorage of HVAC ductwork containing hazardous materials, piping systems and mechanical units containing flammable, combustible or highly toxic materials, electrical equipment used for emergency or standby power, exterior wall panels and suspended ceiling systems.

SOILS AND FOUNDATION

REFERENCE STANDARDS: Conform to OSSC Chapter 18 "Soils and Foundations."

GEOTECHNICAL SUBGRADE INSPECTION: The Geotechnical Engineer shall inspect all sub-grades and prepared soil bearing surfaces, prior to placement of foundation reinforcing steel and concrete. Geotechnical Engineers shall provide a letter to the owner stating that soils are adequate to support the "Allowable Foundation Bearing Pressure(s)" shown below. Assumed values shall be field verified by the Building Official or the Geotechnical Engineer prior to placing concrete.

DESIGN SOIL VALUES:

Safety Factor per Soils Report	1.5	
Allowable Foundation Bearing Pressure	**1500**	PSF
Passive Lateral Pressure	300	PSF/FT
Active Lateral Pressure (unrestrained)	35 PSF/FT	
At-Rest Lateral Pressure (restrained)	55 PSF/FT	
Seismic Lateral Pressure	8HPSF	
Coefficient of Sliding Friction	0.35	

FOUNDATIONS and FOOTINGS: Foundations shall bear on competent native soil as per the geotechnical report. Exterior perimeter footings shall bear not less than **12** inches below finish grade, unless otherwise specified by the geotechnical engineer and/or the building official.

FOOTING DEPTH: Tops of footings shall be as shown on plans with vertical changes as indicated with steps in the footings; locations of steps shown as approximate and shall be coordinated with the civil grading plans.

SLABS-ON-GRADE: All slabs-on-grade shall bear on compacted structural fill or competent native soil per the geotechnical report. All moisture sensitive slabs-on-grade or those subject to receive moisture sensitive coatings/covering shall be provided with an appropriate capillary break and vapor barrier/retardant over the subgrade prepared and installed as noted in the geotechnical report, barrier manufacturer's written recommendations and coordinated with the finishes specified by the Architect.

CAST-IN-PLACE CONCRETE

REFERENCE STANDARDS: Conform to:
(1) ACI 301-20 "Specifications for Structural Concrete"
(2) OSSC Chapter 19 "Concrete"
(3) ACI 318-19 "Building Code Requirements for Structural Concrete"
(4) ACI 117-10 "Specifications for Tolerances for Concrete Construction and Materials"

FIELD REFERENCE: The contractor shall keep a copy of ACI Field Reference manual, SP-15, "Standard Specifications for Structural Concrete (ACI 301) with Selected ACI and ASTM References."

CONCRETE MIXTURES: Conform to ACI 301 Section 4 "Concrete Mixtures" and OSSC Section 1904.1.

MATERIALS: Conform to ACI 301 Section 4.2.1 "Materials" for requirements for cementitious materials, aggregates, mixing water and admixtures.

SUBMITTALS: Provide all submittals required by ACI 301 Section 4.1.2. Submit mix designs for each mix in the table below. Substantiating strength results from past tests shall not be older than 24 months per ACI 318 Section 26.4.3.1 (b).

DRAWING LEGEND

MARK	DESCRIPTION	MARK	DESCRIPTION
F2.0	FOOTING SYMBOL (REFER TO SPREAD FOOTING SCHEDULE)		INDICATES WIDE FLANGE COLUMN
W4	SHEAR WALL SYMBOL (REFER TO SHEAR WALL SCHEDULE)		INDICATES HOLLOW STRUCTURAL SECTION (HSS) COLUMN OR TUBE STEEL (TS) COLUMN
0 RFI 00	REVISION TRIANGLE		INDICATES HOLLOW STRUCTURAL SECTION (HSS) COLUMN OR STEEL PIPE COLUMN
T/FTG = X'-X"	ELEVATION SYMBOL (T/ REFERS TO COMPONENT THAT THE ELEVATION REFERENCES)		INDICATES WOOD POST
3	STUD BUBBLE (INDICATES NUMBER OF STUDS REQUIRED IF EXCEEDS NUMBER SPECIFIED IN PLAN NOTE)		INDICATES BUNDLED STUDS
S	INDICATES STEP IN FOOTING (REFER TO TYPICAL STEP IN FOOTING DETAIL)		INDICATES MOMENT FRAME CONNECTION
X SX.X	DETAILS OR SECTION CUT (DETAIL NUMBER/SHEET NUMBER)		INDICATES CANTILEVER CONNECTION
00 S0.0	DETAILS OR SECTION CUT IN PLAN VIEW (DETAIL NUMBER/SHEET NUMBER)		INDICATES DRAG CONNECTION
XX/SXX.XX	INDICATES LOCATION OF CONCRETE WALLS, SHEAR WALLS OR BRACED FRAME ELEVATIONS		INDICATES A LEDGER
	STRUCTURAL EXTENT SYMBOL SINGLE ARROW - END OF EXTENT DOUBLE ARROW - CONTINUOUS EXTENT ALONG THE ELEMENT LINE UNTIL THE ELEMENT IS INTERRUPTED		INDICATES WOOD OR STEEL STUD BEARING WALL LINE PER KEY ON SHEET
		OR	INDICATES WOOD OR STEEL STUD SHEAR WALL LINE AND HOLD-DOWNS PER KEY ON SHEET
	INDICATES DIRECTION OF DECK SPAN		INDICATES MASONRY/CMU WALL
	INDICATES EXISTING WALL		INDICATES CONCRETE/TILT-UP CONCRETE WALL
			INDICATES BEARING WALL BELOW

ABBREVIATIONS

L	Angle	FB	Factory-Built	PJP	Partial Joint Penetration
AB	Anchor Bolt	FD	Floor Drain	PREFAB	Prefabricated
ADDL	Additional	FDN	Foundation	PSF	Pounds per Square Foot
ADH	Adhesive	FIN	Finish	PSI	Pounds Per Square Inch
ALT	Alternate	FLR	Floor	PSL	Parallel Strand Lumber
ARCH	Architectural	FRP	Fiberglass Reinforced Plastic	P-T	Post-Tensioned
B or BOT	Bottom	FRT	Fire Retardant Treated	PT	Pressure Treated
B/	Bottom Of	FTG	Footing	R	Radius
BLDG	Building	F/	Face of	RD	Roof Drain
BLKG	Blocking	GA	Gage	REF	Refer/Reference
BMU	Brick Masonry Unit	GALV	Galvanized	REINF	Reinforcing
BP	Baseplate	GEOTECH	Geotechnical	REQD	Required
BRBF	Buckling Restrained	GL	Glue Laminated Timber	RET	Retaining
	Braced Frame	GWB	Gypsum Wall Board	SB	Site-Built
BRG	Bearing	HDR	Header	SCBF	Special Concentric
BTWN	Between	HF	Hem-Fir		Braced Frame
C	Camber	HGR	Hanger	SCHED	Schedule
CB	Castellated Beam	HD	Hold-down	SER	Structural Engineer of
C'BORE	Counterbore	HORIZ	Horizontal		Record
CL or ℄	Centerline	HP	High Point	SFRS	Seismic Force-
CLT	Cross-Laminated Timber	HSS = TS	(Hollow Structural Section)		Resisting System
CIP	Cast in Place	IBC	International Building Code	SHTHG	Sheathing
CJ	Construction or	ID	Inside Diameter	SIM	Similar
	Control Joint	IE	Invert Elevation	SLBB	Short Leg Back-to-Back
CJP	Complete Joint	IF	Inside Face	SMF	Special Moment Frame
	Penetration	INT	Interior	SOG	Slab on Grade
CLR	Clear	k	Kips	SP	Southern Pine
CLG	Ceiling	KSF	Kips Per Square Foot	SPEC	Specification
CMU	Concrete Masonry Unit	LF	Lineal Foot	SQ	Square
COL	Column	LL	Live Load	SR	Studrail
CONC	Concrete	LLBB	Long Leg Back-to-Back	SF	Square Foot
CONN	Connection	LLH	Long Leg Horizontal	SST	Stainless Steel
CONST	Construction	LLV	Long Leg Vertical	STAGG	Stagger/Staggered
CONT	Continuous	LP	Low Point	STD	Standard
C'SINK	Countersink	LONGIT	Longitudinal	STIFF	Stiffener
CTRD	Centered	LSL	Laminated Strand Lumber	STL	Steel
DIA	Diameter	LVL	Laminated Veneer Lumber	STRUCT	Structural
DB	Drop Beam	MAS	Masonry	SWWJ	Solid Web Wood Joist
DBA	Deformed Bar Anchor	MAX	Maximum	SYM	Symmetrical
DBL	Double	MECH	Mechanical	T	Top
DEMO	Demolish	MEP	Mechanical, Electrical,	T/	Top Of
DEV	Development		Plumbing	T&B	Top & Bottom
DF	Douglas Fir	MEZZ	Mezzanine	TC AX LD	Top Chord Axial Load
DIAG	Diagonal	MFR	Manufacturer	TCX	Top Chord Extension
DIST	Distributed	MIN	Minimum	TDS	Tie Down System
DL	Dead Load	MISC	Miscellaneous	T&G	Tongue & Groove
DN	Down	NIC	Not In Contract	THKND	Thickened
DO	Ditto	NLT	Nail-Laminated Timber	THRD	Threaded
DP	Depth/Deep	NTS	Not To Scale	THRU	Through
DWG	Drawing	OC	On Center	TRANSV	Transverse
(E)	Existing	OCBF	Ordinary Concentric Braced	TYP	Typical
EA	Each		Frame	UNO	Unless Noted Otherwise
EF	Each Face	OD	Outside Diameter	URM	Unreinforced Masonry
EL	Elevation	OF	Outside Face		Unit
ELEC	Electrical	OPNG	Opening	VERT	Vertical
ELEV	Elevator	OPP	Opposite	W	Wide
EMBED	Embedment	OWSJ	Open Web Steel Joist	W/	With
EQ	Equal	OWWJ	Open Web Wood Joist	W/O	Without
EQUIP	Equipment	PL	Plate	WHS	Welded Headed Stud
EW	Each Way	PAF	Powder Actuated Fastener	WP	Working Point
EXP	Expansion	PC	Precast	WWF	Welded Wire Fabric
EXP JT	Expansion Joint	PERP	Perpendicular	±	Plus or Minus
EXT	Exterior	PLWD	Plywood		

STRUCTURAL SHEET LIST

FOR PERMIT

The Contractor shall not use these drawings for construction until Contractor receives written approval for use in construction by the authority having jurisdiction and DCI Engineers.

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STRUCTURAL REGISTERED PROFESSIONAL ENGINEER
74858PE
OREGON
MARCH 23, 2011
SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: **21031-0263**

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97220

EXISTING

REVISIONS:

#	DATE	DESCRIPTION

DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - GENERAL NOTES, LEGENDS AND ABRREVIATIONS

S1.1

TABLE OF MIX DESIGN REQUIREMENTS

Member Type/Location	Strength f'c (psi)	Test Age (days)	Nominal Maximum Aggregate	Exposure Class	Max W/C Ratio	Air Con-tent	Notes (1 to 9 Typical UNO)
Footings	4000	28	1"	-	-	-	-
Interior Slabs on Grade	3000	28	1"	-	-	-	10
Shear Walls	4000	28	¾"	-	-	-	-

Table of Mix Design Requirements Notes:

(1) W/C Ratio: Water–cementitious material ratios shall be based on the total weight of cementitious materials. Maximum ratios are controlled by strength noted in the Table of Mix Design Requirements and durability requirements given in ACI 318 Section 19.3.

(2) Cementitious Materials:
  a. DCI encourages the reduction of cement content and/or the use of blended hydraulic cements. Where requirements of this section prohibit inclusion of any of these mixes, contact DCI for further coordination.
  b. The use of fly ash, other pozzolans, silica fume, or slag shall conform to ACI 318 Sections 19.3.2 and 26.4.2.2. [For mixes in exposure class F3, as noted in the table above, maximum amount of fly ash shall be 25% of total cementitious content and maximum amount of slag shall be 50%.] [Where supplementary cementitious material (SCM) exceeds 25% of total cementitious content, submit for SER approval.]
  c. For concrete used in elevated floors, minimum cementitious-materials content shall conform to ACI 301 Table 4.2.1.1.(b) Acceptance of lower cement content is contingent on providing supporting data to the SER for review and acceptance.
  d. Cementitious materials shall conform to the relevant ASTM standards listed in ACI 318 Section 26.4.1.1.1(a).

(3) Air Content: Conform to ACI 318 Section 19.3.3.1. Minimum standards for exposure class are noted in the table. If freezing and thawing class is not noted, air content given is that required by the SER. Tolerance is ±1-½%. Air content shall be measured at point of placement.

(4) Aggregates shall conform to ASTM C33.

(5) Slump: Conform to ACI 301 Section 4.2.2.1. Slump shall be determined at point of placement.

(6) Chloride Content: Conform to ACI 318 Table 19.3.2.1.

(7) Non- chloride accelerator: Non-chloride accelerating admixture may be used in concrete placed at ambient temperatures below 50°F at the contractor's option.

(8) ACI 318, Section 19.3.1.1 exposure classes shall be assumed to be F0, S0, W0, and C0 unless different exposure classes are listed in the Table of Mix Design Requirements that modify these base requirements.

(9) Modulus of Elasticity shall be a minimum of $57{,}000 \times \sqrt{f'c}$ for all mix designs.

(10) Shrinkage Limit: Concrete used in elevated slabs and beams shall have a shrinkage limit of [0.045%]. Concrete used in slab-on-grade shall have a shrinkage limit of [0.040%] when exposed or used in industrial applications and a shrinkage limit of [0.045%] for all other conditions and applications. Submit laboratory test results at 28 days measured in accordance with ASTM C157 to SER for approval prior to construction.

FORMWORK & RESHORING: Conform to ACI 301 Section 2 "Formwork and Form Accessories." Removal of Forms shall conform to Section 2.3.2 except strength indicated in Section 2.3.2.5 shall be 0.75 f' c.

MEASURING, MIXING, AND DELIVERY: Conform to ACI 301 Section 4.3.

HANDLING, PLACING, CONSTRUCTING AND CURING: Conform to ACI 301 Section 5. In addition, hot weather concreting shall conform to ACI 305R-20 and cold weather concreting shall conform to ACI 306R-16.

CONSTRUCTION JOINTS: Conform to ACI 301 Sections. 2.2.2.5 and 5.3.2.6. Construction joints shall be located and detailed as on the construction drawings. Submit alternate locations per ACI 301 Section 5.1.2.3(a) for review and approval by the SER two weeks minimum prior to forming. Use of an acceptable adhesive, surface retardant, portland cement grout or roughening the surface is not required unless specifically noted on the drawings.

EMBEDDED ITEMS: Position and secure in place expansion joint material, anchors and other structural and non-structural embedded items before placing concrete. Contractor shall refer to mechanical, electrical, plumbing and architectural drawings and coordinate other embedded items.

GROUT: Use 7000 psi non-shrink grout under column base plates.

GROUTED REBAR: See Post-Installed Anchors to Concrete.

POST-INSTALLED ANCHORS to CONCRETE: Anchor location, type, diameter and embedment shall be as indicated on drawings. Reference the POST INSTALLED ANCHORS section for applicable Post-Installed Anchor Adhesives. Anchors shall be installed and inspected in strict accordance with the applicable ICC-Evaluation Service Report (ESR). Special inspection shall be per the TESTS and INSPECTIONS section.

SHRINKAGE: Conventional and post-tensioned concrete slabs will continue to shrink after initial placement and stressing of concrete. Contractor and subcontractor shall coordinate jointing and interior material finishes to provide adequate tolerance for expected structural frame shrinkage and shall include, but not be limited to curtain wall, dryvit, storefront, skylight, floor finish, and ceiling suppliers. Contact Engineer for expected range of shrinkage.

MODULUS OF ELASTICITY TESTING AND ACCEPTANCE:

Obtain samples and conduct tests of mix designed specified in accordance with ACI 318 Section 19.2.2, ACI 318 R19.2.2, ACI 301 Section 1.7.3.3, ASTM C42 and ASTM C469. Tests data shall be performed at the 28-day compressive test age. Provide a minimum of (2) tests for each mix design used in the following elements: **Shear Walls**. Provide a minimum of (1) test for all other elements. The average of the tests shall equal or exceed the specified Modulus of Elasticity. If (2) or more tests are performed, no individual test shall fall below the specified Modulus of Elasticity by more than 20 percent. A "test" for acceptance is the average strength of two 6 by 12 in. cylinders or three 4 by 8 in. cylinders tested at the specified test age.

STRENGTH TESTING AND ACCEPTANCE:

Testing: Obtain samples and conduct tests in accordance with ACI 301 Section 1.7.3.3. Additional samples may be required to obtain concrete strengths at alternate intervals than shown below.
(1) Cure 4 cylinders for 28-day test age. Test 1 cylinder at 7 days, test 2 cylinders at 28 days, and hold 1 cylinder in reserve for use as the Engineer directs. After 56 days, unless notified by the Engineer to the contrary, the reserve cylinder may be discarded without being tested for specimens meeting 28-day strength requirements.
(2) The number of cylinders indicated above reference 6 by 12 in cylinders. If 4 by 8 in cylinders are to be used, additional cylinders must be cured for testing of 3 cylinders at test age per the table of mix design requirements.

Acceptance. Strength is satisfactory when:
(1) The averages of all sets of 3 consecutive tests equal or exceed the specified strength.
(2) No individual test falls below the specified strength by more than 500 psi.

A "test" for acceptance is the average strength of two 6 by 12 in. cylinders or three 4 by 8 in. cylinders tested at the specified test age.

CONCRETE PLACEMENT TOLERANCE: Conform to ACI 117-10 for concrete placement tolerance.

CONCRETE REINFORCEMENT

REFERENCE STANDARDS: Conform to:
(1) ACI 301-20 "Standard Specifications for Structural Concrete", Section 3 "Reinforcement and Reinforcement Supports."
(2) ACI SP-66(04) "ACI Detailing Manual"
(3) CRSI MSP-09, 28th Edition, "Manual of Standard Practice."
(4) ANSI/AWS D1.4: 2005, "Structural Welding Code - Reinforcing Steel."
(5) OSSC Chapter 19-Concrete.
(6) ACI 318-19 "Building Code Requirements for Structural Concrete."
(7) ACI 117-10 "Specifications for Tolerances for Concrete Construction and Materials"

SUBMITTALS: Conform to ACI 301 Section 3.1.2 "Submittals." Submit placing drawings showing fabrication dimensions and placement locations of reinforcement and reinforcement supports.

MATERIALS:

Reinforcing Bars....................ASTM A615, Grade 60, deformed bars.
.ASTM A706, Grade 60, deformed bars.

Smooth Welded Wire Fabric ....ASTM A1064
Deformed Welded Wire Fabric.............ASTM A1064
Bar Supports ....CRSI MSP-09, Chapter 3 "Bar Supports."
Tie Wire ....16 gage or heavier, black annealed.
Stud Rails ....ASTM A1044
Headed Deformed Bars ....ASTM A970

EARTHQUAKE REQUIREMENTS: Longitudinal Bars in shear walls and coupling beams of shear walls shall conform to ASTM A706, Grade 60 or shall conform to the following requirements:
(1) Welding: Welding is not permitted except as specified in the drawings. Weld in accordance with AWS D1.4.
(2) Mill Tests: Submit mill certificates indicating physical and chemical properties.
(3) Yield Strength: Actual yield strength, based on mill tests, does not exceed the specified yield strength by more than 18,000 psi. (Retests shall not exceed this value by more than an additional 3000 psi.)
(4) Ultimate Strength: The ratio of the actual tensile strength to the actual yield strength is not less than 1.25.
(5) Strain: Minimum elongation in 8 in. shall be at least:
  a. 14 percent for bar sizes No. 3 through No. 6
  b. 12 percent for bar sizes No. 7 through No. 11
  c. 10 percent for bar sizes No. 14 and No 18

FABRICATION: Conform to ACI 301, Section 3.2.2. "Fabrication", and ACI SP-66 "ACI Detailing Manual."

WELDING: Bars shall not be welded unless authorized. When authorized, conform to ACI 301, Section 3.2.2.2. "Welding", AWS D1.4, and provide ASTM A706, grade 60 reinforcement.

PLACING: Conform to ACI 301, Section 3.3.2 "Placing." Placing tolerances shall conform to ACI 117.

CONCRETE COVER: Conform to the following cover requirements unless noted otherwise in the drawings.
Concrete cast against earth ....3"
Concrete exposed to earth or weather................2"
Ties in columns and beams ....1-½"
Bars in slabs...¾"
Bars in walls...¾"
Exterior bars in Tilt-up Panels..............................1"

CAST-IN-PLACE CONCRETE COVER AND REINFORCING PROTECTION: Conform to the following cover and corrosion protection requirements unless noted otherwise in the drawings:

Reinforcement Location	Minimum Cover	Rebar Protection
Footing Bottom Reinforcing	3"	Uncoated
Footing Top Reinforcing	2"	Uncoated
Slab-on-Grade Reinforcing	2" from top	Uncoated
Shear Walls Boundary Elements	1-½" to ties	Uncoated
Walls not in Contact with Earth	¾"	Uncoated
Walls in Contact with Earth	2"	Uncoated

SPLICES: Conform to ACI 301, Section 3.3.2.7, "Splices". Refer to "Typical Lap Splice and Development Length Schedule" for typical reinforcement splices. Refer to "Shear Wall Reinforcing Splice Schedule" for those specific elements. Splices indicated on individual sheets shall control over the schedule. Mechanical connections may be used when approved by the SER. For reinforcing within the lateral system (shear walls) and reinforcing connecting the diaphragm slab to the lateral system, mechanical splice strength is increased to develop 125 percent of the specified tensile strength of the splices bar.

FIELD BENDING: Conform to ACI 301 Section 3.3.2.8. "Field Bending or Straightening." Bar sizes #3 through #5 may be field bent cold the first time. Subsequent bends and other bar sizes require preheating. Do not twist bars. Bars shall not be bent past 45 degrees.

TYPICAL CONCRETE REINFORCEMENT: Unless noted on the plans, concrete walls shall have the following minimum reinforcement. Contractor shall confirm minimum reinforcement of walls with SER prior to rebar fabrication.

TABLE of MINIMUM CONCRETE WALL REINFORCING

Wall Thickness	HORIZONTAL Bars	VERTICAL Bars	Location
6"	#4 @ 12" OC	#4 @ 12" OC	center in wall
8"	#5 @ 12" OC	#5 @ 12" OC	center in wall
10"	#4 @ 16" OC EF	#4 @ 16" OC EF	EF = each face
12"	#4 @ 12" OC EF	#4 @ 12" OC EF	EF = each face

POST-INSTALLED ANCHORS (INTO CONCRETE AND MASONRY)

REFERENCE STANDARDS: Conform to:
(1) OSSC Chapter 19 "Concrete"
(2) ACI 318-19 "Building Code Requirements for Structural Concrete"
(3) OSSC Chapter 21 "Masonry"
(4) TMS 402-16 "Building Code Requirements for Masonry Structures"

POST-INSTALLED ANCHORS: Install only where specifically shown in the details or allowed by SER. All post-Installed anchors types and locations shall be approved by the SER and shall have a current ICC-Evaluation Service Report that provides relevant design values necessary to validate the available strength exceeds the required strength. Submit current manufacturer's data and ICC ESR report to SER for approval regardless of whether or not it is a pre-approved anchor. Anchors shall be installed in strict accordance with ICC-ESR and the manufacturer's printed installation instructions (MPII) in conjunction with edge distance, spacing and embedment depth as indicated on the drawings. The contractor shall arrange for a manufacturer's field representative to provide installation training for all products to be used, prior to the commencement of work. Only trained installer shall perform post installed anchor installation. A record of training shall be kept on site and be made available to the SER as requested. Adhesive anchors installed in horizontally or upwardly inclined orientation shall be performed by a certified adhesive anchor installer (AAI) as certified through ACI/CRSI or approved equivalent. Proof of current certification shall be submitted to the engineer for approval prior to commencement of installation. No reinforcing bars shall be damaged during installation of post-installed anchors. Special inspection shall be per the TESTS and INSPECTIONS section. Anchor type, diameter and embedment shall be as indicated on drawings.

(1) ADHESIVE ANCHORS: The following Adhesive-type anchoring systems have been used in the design and shall be used for anchorage to CONCRETE, MASONRY, and UNREINFORCED MASONRY, as applicable and in accordance with corresponding current ICC ESR report. Reference the corresponding ICC ESR report for required minimum age of concrete, concrete temperature range, moisture condition, light weight concrete, and hole drilling and preparation requirements. Drilled-in anchor embedment lengths shall be as shown on drawings, or not less than 7 times the anchor nominal diameter (7D). Adhesive anchors are to be installed in concrete aged a minimum of 21 days, unless otherwise specified in the ICC ESR report.
  a. SIMPSON "SET-3G" – ICC ESR 4057 for anchorage to CONCRETE only
  b. SIMPSON "ET-HP" – ICC ESR-3638, IAPMO 241 for anchorage to UNREINFORCED CONCRETE MASONRY (URM)

(2) EXPANSION ANCHORS: The following Expansion type anchors are pre-approved for anchorage to CONCRETE or MASONRY in accordance with corresponding current ICC ESR report:
  a. SIMPSON "STRONG-BOLT 2" – ICC ESR-3037 for anchorage to CONCRETE, IAPMO 240 to MASONRY
  b. SIMPSON "WEDGE-ALL" – ICC ESR-1396 for anchorage to MASONRY

(3) SCREW ANCHORS: The following Screw type anchor is pre-approved for anchorage to CONCRETE or MASONRY in accordance with corresponding current ICC ESR report:
  a. SIMPSON "TITEN HD" – ICC ESR-2713 for CARBON STEEL to CONCRETE, ICC ESR-1056 for MASONRY Only, IAPMO 493 for STAINLESS STEEL to CONCRETE

STRUCTURAL STEEL

REFERENCE STANDARDS: Conform to:
(1) OSSC Chapter 22 – "Steel"
(2) ANSI/AISC 303-16 – "Code of Standard Practice for Steel Buildings & Bridges"
(3) AISC – "Manual of Steel Construction", Fifteenth Edition (2016)
(4) ANSI/AISC 360-16 – "Specification for Structural Steel Buildings"
(5) AWS D1.1:2015 – "Structural Welding Code – Steel"
(6) 2014 RCSC – "Specification for Structural Joints using High-Strength Bolts"

SUBMITTALS: Submit the following documents to the SER for review:

(1) SHOP DRAWINGS complying with AISC 360 Sections M1and N3 and AISC 303 Section 4.
(2) ERECTION DRAWINGS complying AISC 360 Sections M1and N3 and AISC 303 Section 4.

Make copies of the following documents "Available upon Request" to the SER or Owner's Inspection Agency in electronic or printed form prior to fabrication per AISC 360 Section N3.2 requirements:

(1) Fabricator's written Quality Control Manual that includes, as a minimum:
  a. Material Control Procedures
  b. Inspection Procedures
  c. Non-conformance Procedures
(2) Steel & Anchor Rod suppliers' Material Test Reports (MTR's) indicating the compliance with specifications.
(3) Fastener manufacturer's Certification documenting conformance with the specification.
(4) Filler metal manufacturer's product data for SMAW, FCAW and GMAW indicating:
  a. Product specification compliance
  b. Recommended welding parameters
  c. Recommended storage and exposure requirements including baking
  d. Limitations of use
(5) Welded Headed (Shear) Stud Anchors Manufacturer's certification indicating the meet specifications.
(6) Weld Procedure Specifications (WPS's) for shop and field welding.
(7) Manufacturer's Certificates of Conformance for electrodes, fluxes and gases (welding consumables).
(8) Procedure Qualification Records (PQR's) for WPS's that are not prequalified in accordance with AWS.
(9) Welding personnel Performance Qualification Records (WPQR) and continuity records conforming to AWS standards

MATERIALS:
Structural steel materials shall conform to materials and requirements listed in AISC 360 section A3 including, but not limited to:
Wide Flange (W), Tee (WT) Shapes ....ASTM A992 Fy = 50 ksi
Structural (S), (M) & (HP) Shapes ....ASTM A36, Fy = 36 ksi
Channel (C) & Angle (L) Shapes ....ASTM A36, Fy = 36 ksi
Structural Plate (PL)..ASTM A36, Fy = 36 ksi
High Strength Plate (Gr 50 PL)..........................ASTM A572, Fy = 50 ksi
Hollow Structural Section – Square/Rect (HSS)..ASTM A500, Grade C Fy = 50 ksi
Structural Pipe, (PIPE) 12" dia. and less ....ASTM A53, Grade B Fy = 35 ksi
High Strength, Heavy Hex Structural Bolts..........ASTM F3125 Gr. A325/F1852, Type 1 or 3, Plain
Heavy Hex Nuts ....ASTM A563, Grade and Finish per RCSC Table 2.1
Washers (Hardened Flat or Beveled) ....ASTM F436, Grade and Finish per RCSC Table 2.1
Anchor Rods (Anchor Bolts, typical) ....ASTM F1554, Gr. 36
Anchor Rods (High Strength)..............................ASTM F1554, Gr. 55 (weldable) per Supplement S1
Welded Headed Stud Anchors............................ASTM A108 – Nelson/TRW S3L
Welded Headed Stud Anchors............................ASTM A108 – Nelson/TRW H4L
Deformed Bar Anchors (DBA) ....ASTM A496 – Nelson/TRW D2L, Fy = 70 ksi

STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS:

(1) ASTM F3125 Gr. A325-N bolts – "threads NOT excluded in the shear plane".
(2) High strength bolted joints have been designed as "BEARING" connections.
(3) Provide ASTM Bolt Grade and Type as specified in the Materials section above.
(4) Provide Washers over outer ply of slotted holes and oversize holes per RCSC Table 6.1.
(5) Provide Nut and Washer grades, types and finishes conforming to RCSC specification Table 2.1.
(6) Provide *fastener assemblies* from a single supplier.
(7) Joint Types shall be:
  a. ST - "Snug Tight", for typical beam end "shear" connections, unless noted otherwise.
  b. SC - "Slip Critical", where specifically indicated. Provide with Class A [B] Faying surface.
(8) Install bolts in joints in accordance with the RCSC Specification Section 8 and Table 4.1.
(9) Inspection is per RCSC Section 9.

ANCHORAGE to CONCRETE:

(1) SHEAR STUDS on STEEL BEAMS for COMPOSITE CONSTRUCTION: Headed Shear Studs welded to tops of Wide Flange Beams, shall be 3/4" diameter WHS with nominal stud lengths as indicated. Unless noted otherwise, provide minimum shear stud height equal to the (metal deck depth + 1 ½") and a maximum shear stud height that allows for ½" of concrete cover over the stud.

(2) EMBEDDED STEEL PLATES for Anchorage to Concrete: Plates (PL) embedded in concrete with studs (WHS) or dowel bar anchors (DBA) shall be of the sizes and lengths as indicated on the plans with minimum 1/2" dia. WHS x 6" long but provide not less than ¾" interior cover or 1 ½" exterior cover to the opposite face of concrete, unless noted otherwise.

(3) COLUMN ANCHOR RODS and BASE PLATES: All columns (vertical member assemblies weighing over 300 pounds) shall be provided with a **minimum of four ¾" diameter anchor rods**. Column base plates shall be at least ¾" thick, unless noted otherwise. Cast-in-place anchor rods shall be provided unless otherwise approved by the Engineer. Unless noted otherwise, embedment of cast-in-place anchor rods shall be 12 times the anchor diameter (12D).

FABRICATION:

(1) Conform to AISC 360 Section M2 "Fabrication" and AISC 303 Section 6 "Shop Fabrication".

(2) Quality Control (QC) shall conform to:
  a. AISC 360 Chapter N "Quality Control and Quality Assurance" and
  b. AISC 303 Section 8 "Quality Control".
  c. Fabricator and Erector shall establish and maintain written Quality Control (QC) procedures per AISC 360 section N3.
  d. Fabricator shall perform self-inspections per AISC 360 section N5 to ensure that their work is performed in accordance with Code of Standard Practice, the AISC Specification, Contract Documents and the Applicable Building Code.
  e. QC inspections may be coordinated with Quality Assurance inspections per Section N5.3 where fabricators QA procedures provide the necessary basis for material control, inspection, and control of the workmanship expected by the Special Inspector.

WELDING:

(1) Welding shall conform to AWS D1.1 with Prequalified Welding Processes except as modified by AISC 360 section J2. Welders shall be qualified in accordance with AWS D1.1requirements.
(2) Use 70ksi strength, low-hydrogen type electrodes (E7018) or E71T as appropriate for the process selected.
(3) Welding of high strength anchor rods is prohibited unless approved by Engineer.
(4) Welding of headed stud anchors shall be in accordance with AWS D1.1 Chapter 7 "Stud Welding".

ERECTION:

(1) Conform to AISC 360 Section M4 "Erection" and AISC 303 Section 7 "Erection".
(2) Conform to AISC 360 Chapter N "Quality Control and Quality Assurance" and AISC 303 Section 8.
  a. The Erector shall maintain detailed erection quality control procedures that ensure that the work is performed in accordance with these requirements and the Contract Documents.
(3) Steel work shall be carried up true and plumb within the limits defined in AISC 303 Section 7.13.
(4) High strength bolting shall comply with the RCSC requirements including RCSC Section 7.2 "Required Testing", as applicable and AISC 360 Chapter J, Section M2.5 and Section N5.6.
(5) Welding of HEADED STUD ANCHORS shall be in accordance with AWS D1.1 Chapter 7 "Stud Welding.
(6) Provide Headed (Shear) Stud Anchors welded through the metal deck to tops of beams denoted in plans.
(7) The contractor shall provide temporary bracing and safety protection required by AISC 360 Section M4.2 and AISC 303 Section 7.10 and 7.11.

PROTECTIVE COATING REQUIREMENTS:

(1) SHOP PAINTING: Conform to AISC 360 Section M3 and AISC 303 Section 6.5 unless otherwise specified by the project specifications.

(2) INTERIOR STEEL:
  a. Unless noted otherwise, ***do not paint*** any of the steel surfaces meeting the following conditions:
    i. Concealed by the interior building finishes,
    ii. Fireproofed,
    iii. Embedded in concrete,
    iv. Specially prepared as a "faying surface" for Type-SC "slip-critical" connections including bolted connections that form a part of the Seismic Force Resisting System governed by AISC 341 unless the coating conforms to requirements of the RCSC Bolt Specification and is approved by the Engineer.
    v. Welded; if area requires painting, do not paint until after weld inspections and non-destructive testing requirement, if any, are satisfied.
  b. Interior steel, exposed to view, shall be painted with one coat of shop primer unless otherwise indicated in the project specifications. Field touch-ups to match the finish coat or as otherwise indicated in the project specifications.

WOOD FRAMING

REFERENCE STANDARDS: Conform to:
(1) OSSC Chapter 23 "WOOD"
(2) ANSI/AWC NDS - 2018: "National Design Specification (NDS) for Wood Construction - with 2018 NDS Supplement"
(3) ANSI/AWC – SDPWS-2021: Special Design Provisions for Wind and Seismic
(4) APA PDS - 20: "Panel Design Specification"
(5) TPI 1-2014 "National Design Standard for Metal-Plate-Connected Wood Truss Construction"
(6) BCSI B1 "Guide to Good Practice for Handling, Installing, Restraining & Bracing of Trusses"
(7) DSB-89 "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses"
(8) APA Report TT-045B "Minimum Nail Penetration for Wood Structural Panel Connections Subject to Lateral Loads"
(9) APA Report TT-061C "1-5/16 Inch-Thick I-Joist Flanges and Diaphragm Nail Penetration

SUBMITTALS: Submit shop drawings to the Architect/Engineer for review. Shop drawings shall include member size, spacing, camber, material type, grade, shop and field assembly details and connections, types and location of bolts and other fasteners. Supply shop drawings for the following:
(1) Glued laminated members
(2) Tapered & Parallel Wood I Joists (Solid web-wood joists)

IDENTIFICATION: All sawn lumber and pre-manufactured wood products shall be identified by the grade mark or a certificate of inspection issued by the certifying agency.

MATERIALS:

(1) Sawn Lumber: Conform to grading rules of WWPA, WCLIB or NLGA and Table below. Finger jointed studs acceptable at interior walls only.

TABLE of SOLID SAWN LUMBER

Member Use	Size	Species	Grade
Wall Stud/ Top & Bottom Plates	2x4, 3x4, 2x6, 3x6	Doug Fir Larch	No. 2
Sill Plate (at concrete)	2x4, 3x4, 2x6, 3x6	PT Doug Fir Larch	No. 2
Post	4x4, 4x6, 4x8	Doug Fir Larch	No. 2
Floor or Roof Joist	2x6 through 2x12	Doug Fir Larch	No. 2
Beam	4x8 through 4x12	Doug Fir Larch	No. 2
Beam	6x8 through 6x12	Doug Fir Larch	No. 1
Post or Timber	6x6, 8x8	Doug-Fir Larch	No. 1

(2) Glued Laminated Timber: Conform to ANSI 117**-2020** "Standard Specifications for Structural Glued laminated Timber of Softwood Species, Manufacturing and Design" and ANSI A190.1 "Structural Glued Laminated Timber." Camber all glued laminated beams, except cantilevered and continuous beams, to 3000' radius, unless shown otherwise on the plans. Fabricate cantilevered and continuous beams flat, unless shown otherwise on plans.

TABLE of GLULAM and GRADE

Member	Sizes	Species	Comb. Symbol	Uses
Beams	All	DF/DF	24F–V4	Simple Spans
Beams	All	DF/DF	24F–V8	Continuous or with Cantilever Spans
Columns	All	DF	L2	Post, Truss Member

(3) Wood Structural Sheathing (Plywood): Wood APA-rated structural sheathing includes: all veneer plywood, oriented strand board, waferboard, particleboard, T1-11 siding, and composites of veneer and wood-based material with T&G joint. **Architect** may disallow OSB. Confirm with **Architect**. Conform to "Structural Plywood" based on Product Standard PS 1-19 by the U.S. Dept. of Commerce, and "Performance Standard for Wood Structural Panels" based on Product Standard PS 2-18 by the U.S. Dept. of Commerce and "Panel Design Specification" based on APA D510 by the Engineered Wood Association. Unless noted otherwise, sheathing shall comply with the following table:

TABLE of SHEATHING - Use, Minimum Thickness and Minimum APA Rating

Location	Thickness	Span Rating	Plywood Grade	Exposure
Roof	15/32"	32/16	C-D	1
Floor	23/32" T&G	24 OC	STURD-I-FLOOR	1
Walls	15/32"	32/16	C-D	1

Unless noted otherwise on drawings, install roof and floor panels with long dimension across supports and with panel continuous over two or more spans. End joints shall occur over supports.

(1) Timber Connectors: Shall be "Strong Tie" by Simpson Company as specified in their latest catalog. Alternate connectors by other manufacturers may be substituted provided they have current ICC approval for equivalent or greater load capacities and are reviewed and approved by the SER prior to ordering. Connectors shall be installed per the manufacturer's instructions. Where connector straps connect two members, place one-half of the nails or bolts in each member. Where connectors are in exposed exterior applications in contact with preservative treated wood (PT) other than CCA, connectors shall be either batch hot-dipped galvanized (HDG), mechanically galvanized (ASTM B695, Class 55 minimum) stainless steel, or provided with 1.85 oz/sf of zinc galvanizing equal to or better than Simpson ZMAX finish.

Where straps are used as hold-downs, nail straps to wood framing just prior to drywall application, as late as possible in the framing process to allow the wood to shrink and the building to settle. Premature nailing of the strap may lead to strap buckling and potential finish damage.

Fasteners (nails, bolts, screws, etc) attaching timber connectors (joist hangers, post caps and bases, etc) to PT wood shall have similar corrosion resistance properties (matching protective treatments) as the protected connector. Fasteners (nails, bolts, screws, etc) attaching sawn timber members or sheathing (shear walls) to PT wood shall be corrosion resistant; nails and lag bolts shall be either HDG (ASTM A153) or stainless steel. Verify the suitability of the fastener protection/coating with the wood treatment chemical manufacturer/supplier.

Provide washers under the heads and nuts of all bolts and lag screws bearing on wood.

(1) Lag Screws/Bolts: Conform to ASTM A307 and NDS Chapter 12.

Lag screw installation shall conform to NDS Section 12.1.4, including required drilled lead and/or clearance holes.

(2) Nails and Staples: Conform to ASTM F1667 and OSSC Sections 2303.6 and 2304.10.

FOR PERMIT
The Contractor shall not use these drawings for construction until Contractor receives written approval for use in construction by the authority having jurisdiction and DCI Engineers.

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CIVIL / STRUCTURAL

STRUCTURAL REGISTERED PROFESSIONAL ENGINEER
74858PE
OREGON
MARCH 23, 2011
SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: **21031-0263**

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97220

EXISTING

REVISIONS:

#	DATE	DESCRIPTION

DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - GENERAL NOTES CONTINUED

S1.2

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NAILING REQUIREMENTS: Conform to OSSC Section 2304.10 "Connectors and fasteners." Unless noted on plans, nail per Table 2304.10.2. Nailing for roof/floor diaphragms/shear walls shall be per drawings. Nails shall be driven flush and shall not fracture the surface of sheathing. Alternate nails may be used but are subject to review and approval by the Structural Engineer. Substitution of staples for the nailing of rated sheathing is subject to review by the structural engineer prior to construction.

STANDARD LIGHT-FRAME CONSTRUCTION: Unless noted on the plans, construction shall conform to OSSC Section 2308 "Conventional Light-Frame Construction."

NAILERS ON STEEL COLUMNS and BEAMS: Wood 3x nailers are generally required on all HSS columns and steel beams abutting or embedded within wood framing. Unless noted otherwise, attach with 5/8" diameter bolts or welded studs at 16" on centers. Unless noted otherwise, wood nailers on beams supporting joist hangers shall not overhang the beam flange by more than ¼".

WOOD SHRINKAGE AND EXPANSION: Wood materials will expand, or contract based on relative changes in moisture. The contractor is responsible for means and methods of construction related to mitigating and managing the effects of changes in moisture.

MOISTURE CONTENT: The contractor shall make provisions during handling and construction to prevent the structural wood members from exceeding the appropriate moisture content limits. The moisture content for solid sawn wood material used for this project shall not exceed 19%. The moisture content for engineered wood products, laminated lumber and sheathing shall not exceed the limits required by the manufacturer or 12%, whichever is less. The moisture content limits may be more stringent for particular product requirements (e.g., finishes, cladding, insulation systems, etc.). The contractor shall refer to the Architect's drawings, project specifications, or installer/product requirements for additional requirements.

SHRINKAGE COMPENSATION FOR MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS: MEP systems, including ductwork, pipes, and other elements that run continuously between levels shall be installed/designed in such a manner to accommodate shrinkage in the wood framing. Wood shrinkage amounts will vary depending on the construction process and materials used. The anticipated shrinkage under typical conditions is expected to range between 1/8" and 1/4" per floor.

CLADDING COMPATIBILITY: The Architect/Owner and contractor shall review the cladding, finishes, insulation systems, other non-structural components and construction procedures proposed for the project with respect to their performance over wood framing. EIFS systems should be avoided on wood-framed projects due to problems with moisture proofing. Note that DCI is not responsible for the attachment of the cladding to the wood studs which needs to be verified and provided by the cladding supplier.

STORAGE & HANDLING: All storage and handling is to be a means and methods provided by the contractor. The contractor is to determine the best practices in order to avoid damage to the members during storage, such as fungal growth, exposure to weather conditions. The following are suggestions to aid the contractor. All materials should be stored level on site and must be raised off the ground a minimum of 6" by means of blocking and separating spacers. It is recommended that the materials are covered with an additional opaque waterproof material (i.e. good quality tarpaulin). When members are wrapped in poly or another material which may inhibit air flow, the material should have slots in the material or perforations to allow for air flow and prevent the accumulation of water and or condensation. Ensure that all exposed members are protective material from exposure should be removed only after the roof or structure providing cover is installed. Members should be constantly protected from weather during transportation, storage, and erection.

For interior glulam members the heat in the building should be gradually increased over a two-to-three-week period in order to provide a gradual change in moisture content. Do not direct any forced air heating systems onto the glulam members. It is recommended to apply the final finish to the glulam member before heat is applied.

Members that are to be exposed to view in the finished structure should be handled using nylon or fabric slings to prevent surface damage. The contractor should also use means to protect corners of members to prevent "crushing" during transportation, storage and erection. All bolts should be galvanized, or make sure that they are free of oil to prevent staining. Glulam members should be treated and stained per the architect of records recommendations. The following are provided in order to help guide the contractor in the best practices to preserve the quality of wood products. These notes are not intended to be comprehensive and an end all solution and should be taken under consideration by the contractor and supplemented as necessary.

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74858PE
OREGON
MARCH 23, 2011
SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 21031-0263

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97220

EXISTING

REVISIONS:

#	DATE	DESCRIPTION

DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - GENERAL NOTES CONTINUED

S1.3

SPECIAL INSPECTIONS

The following Statement and Schedules of Inspections are those Special Inspections and Tests that shall be performed for this project. Special Inspectors shall reference these plans and IBC Chapter 17 for all special inspection requirements.
The owner shall retain an "approved agency" per OSSC 1703 to provide special inspections for this project. Special Inspectors shall be qualified persons per OSSC 1704.2.1.
Special inspection reports shall be provided on a weekly basis. Submit copies of all inspection reports to the Architect/Engineer and the Authority Having Jurisdiction for review. In addition to special inspection reports and tests, submit reports and certificates noted in IBC 1704.5 to the Authority Having Jurisdiction. Final special inspection reports will be required by each special inspection firm per IBC 1704.2.4.

STATEMENT OF SPECIAL INSPECTIONS:

This statement of Special Inspections has been written with the understanding that the Building Official will:
- **Review and approve the qualifications of the Special Inspectors**
- **Monitor the special inspection activity on the project site to assure that Special Inspectors are qualified and performing their duty as state within this statement.**
- **Review all Special Inspection Reports submitted to them by the Special Inspector**
- **Perform inspections as required by OSSC Section 110.3.**

The following Special Inspections are applicable to this project:
- **Special Inspections for Standard Buildings (per OSSC 1705.1) REQUIRED**
- **Special Inspections for Seismic Resistance (per OSSC 1705.13) REQUIRED**
- **Testing for Seismic Resistance (per OSSC 1705.14) REQUIRED**
- **Special Inspections for Wind Resistance (per OSSC 1705.12) NOT REQUIRED**

STRUCTURAL STEEL

per OSSC 1705.2.1

A qualified Special Inspector of an "approved agency" providing Quality Assurance (QA) Special Inspections for the project shall review and confirm the Fabricator and Erector's Quality Control (QC) procedures for completeness and adequacy relative to AISC 360-16 Chapter N, AISC 303-16 Code of Standard Practice, AWS D1.1-2015 Structural Welding Code, and 2022 OSSC code requirements for the fabricator's scope of work.

- QA Agency providing Special Inspections shall provide personnel meeting the minimum qualification requirements for Inspection and Nondestructive Testing NDT per AISC 360 Section N4.
- Verify Fabricator and Erector QC Program per AISC 360 Section N2.
- Inspection of welds and bolts by both QC and QA personnel shall be per the Schedule of Special Inspections below. All provisions of AWS D1.1 Structural Welding Code for statically loaded structures shall apply.
- Nondestructive Testing (NDT) of welds:
  - Non-Destructive Testing (NDT) of welded joints per AISC 360 N5.5
  - Risk Category for determination of extent of NDT per AISC 360 N5.5b is noted in the Design Criteria and Loads section of these General Requirements.
  - NDT performed shall be documented and reports shall identify the tested weld by piece mark and location of the piece.
  - For field work, the NDT report shall identify the tested weld by location in the structure, piece mark and location of the piece.
- Additional Inspection tasks per AISC 360 Section N5.8.
- Inspection for Composite Construction shall be done per AISC 360 Section N6.

POST-INSTALLED ANCHORS TO CONCRETE AND MASONRY: shall comply with IBC Section 1703. Inspections shall be in accordance with the requirements set forth in the approved ICC Evaluation Report and as indicated by the design requirements specified on the drawings. Refer to the POST INSTALLED ANCHORS section of these notes for anchors that are the basis of the design. Special inspector shall verify anchors are as specified in the POST INSTALLED ANCHORS section of these notes or as otherwise specified on the drawings. Substitutions require approval by the SER and require substantiating calculations and current 2021 OSSC recognized ICC Evaluation Services (ES) Report. Special Inspector shall document in their Special Inspection Report compliance with each of the elements required within the applicable ICC Evaluation Services (ES) Report.

PREFABRICATED CONSTRUCTION: All prefabricated construction shall conform to IBC Section 1703.

SCHEDULES OF SPECIAL INSPECTIONS:

TABLE 1705.6 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1.Verify materials below shallow foundations are adequate to achieve the design bearing capacity	-	X
2.Verify excavations are extended to proper depth and have reach proper material	-	X
3.Perform classification and testing of compacted fill materials	-	X
4.During fill placement, verify use of proper materials and procedures in accordance with the provisions of the approved geotechnical report. Verify densities and lift thicknesses during placement and compaction of compacted fill.	X	-
5.Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly	-	X

TABLE 1705.3 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	OSSC REFERENCE
1.Inspection, reinforcement, and verify placement.	-	X	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	-
2.Reinforcing bar welding:				
a. Verify weldability of reinforcing bars other than ASTM A706	-	X	AWS D1.4 ACI 318: 26.6.4	-
b. Inspect single pass fillet weld maximum 5/16"	-	X		
c.Inspect all other welds	X	-		
3.Inspect anchors cast in concrete	-	X	ACI 318: 17.8.2	-
4.Inspect anchors post-installed in hardened concrete members:				
a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads	X	-	ACI 318: 17.8.2.4	-
b. Mechanical anchors and adhesive anchors not defined in 4.a	-	X	ACI 318: 17.8.2	
5. Verify use of required design mix	-	X	ACI 318: Ch. 19,	1904.1, 1904.2
6. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete	X	-	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	-
7. Inspect concrete and shotcrete placement for proper application techniques	X	-	ACI 318: 26.5	-
8. Verify maintenance of specified curing temperature and techniques	-	X	ACI 318 :26.5.3 – 26.5.5	-
9. Inspect formwork for shape, location and dimensions of the concrete member being formed	-	X	ACI 318: 26.11.1.2 (b)	-

REQUIRED SPECIAL INSPECTIONS OF WOOD CONSTRUCTION

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD
1. High Load Diaphragms			OSSC Section 1705.5.1
a. Panel sheathing for grade and thickness	X	-	
b. Nominal size of framing members at adjoining panel edges	X	-	
c. Nail and staple diameters and lengths	X	-	
d. Number of fastener lines, spacing between fasteners in each line and at edge margins	X	-	
2. Shear Walls (where fastener spacing of the sheathing is 4 inches or less on center)			OSSC Section 1705.12.1 OSSC Section 1705.13.2
a. Anchor Bolts including proper bottom plate sizes (2x and 3x) and plate washers	-	X	
b. Hold-downs (HD) and Continuous Rod Tie-Down Systems (TDS) including squash blocks and anchors to concrete	-	X	
c. A35 and LPT shear connectors	-	X	
d. Strap Connectors	-	X	
e. Boundary Edge Nailing	-	X	
f. Plate Nailing and Panel Edge Nailing for size and spacing	-	X	
g. Blocking	-	X	
3. Blocked and Unblocked Diaphragms (where fastener spacing of the sheathing is 4 inches or less on center)			OSSC Section 1705.12.1 OSSC Section 1705.13.2
a. Blocking and strap connections	-	X	
b. Boundary edge and panel shear nailing size and spacing	-	X	
4. Moisture Content of wood studs, plates, beams, decking, and joists	-		
5. Roof truss 'hurricane clips'	-	X	

MINIMUM REQUIREMENTS FOR INSPECTIONS OF STRUCTURAL STEEL CONSTRUCTION

INSPECTION TASKS	QC	QA	REFERENCED STANDARD
INSPECTION TASKS PRIOR TO WELDING			
1. Welder qualification records and continuity records	P	O	AISC 360 TABLE N5.4-1
2. Welding procedure specifications (WPSs) available	P	P	AISC 360 TABLE N5.4-1
3. Manufacturing certifications for welding consumables available	P	P	AISC 360 TABLE N5.4-1
4. Material identification (type/grade)	O	O	AISC 360 TABLE N5.4-1
5. Welder Identification system	O	O	AISC 360 TABLE N5.4-1
6. Fit-up of groove welds (including joint geometry) • Joint preparation • Dimensions (alignment, root opening, root face, bevel) • Cleanliness (condition of steel surfaces) • Tacking (tack welding quality and location) • Backing type and fit (if applicable)	O	O	AISC 360 TABLE N5.4-1
7. Fit-up of CJP groove welds of HSS T-, Y-, and K-joints without backing (including joint geometry) • Joint preparation • Dimensions (alignment, root opening, root face, bevel) • Cleanliness (condition of steel surfaces) • Tacking (tack welding quality and location)	P	O	AISC 360 TABLE N5.4-1
8. Configuration and finish of access holes	O	O	AISC 360 TABLE N5.4-1
9. Fit-up of fillet welds • Dimensions (alignment, gaps at root) • Cleanliness (condition of steel surfaces) • Tacking (tack weld quality and location)	O	O	AISC 360 TABLE N5.4-1
10. Check welding equipment	O	-	AISC 360 TABLE N5.4-1
INSPECTION TASKS DURING WELDING			
1. Use of qualified welders	O	O	AISC 360 TABLE N5.4-2
2. Control and handling of welding consumables • Packaging • Exposure control	O	O	AISC 360 TABLE N5.4-2
3. No welding over cracked tack welds	O	O	AISC 360 TABLE N5.4-2
4. Environmental conditions • Wind speed within limits • Precipitation and temperature	O	O	AISC 360 TABLE N5.4-2
5.WPS followed • Settings on welding equipment • Travel speed • Selected welding materials • Shielding gas type/flowrate • Preheat applied • Interpass temperature maintained (min/max) • Proper position (F, V, H, OH)	O	O	AISC 360 TABLE N5.4-2
6.Welding techniques • Interpass and final cleaning • Each pass within profile limitations • Each pass meets quality requirements	O	O	AISC 360 TABLE N5.4-2
INSPECTION TASKS AFTER WELDING			
1.Welds cleaned	O	O	AISC 360 TABLE N5.4-3
2.Size, length, and locations of welds	P	P	AISC 360 TABLE N5.4-3
3.Welds meet visual acceptance criteria • Crack prohibition • Weld/base-metal fusion • Crater cross section • Weld profiles • Weld size • Undercut • Porosity	P	P	AISC 360 TABLE N5.4-3
4.Arc strikes	P	P	AISC 360 TABLE N5.4-3
5.k-area	P	P	AISC 360 TABLE N5.4-3
6.Weld access holes in rolled heavy shapes and built-up heavy shapes	P	P	AISC 360 TABLE N5.4-3
7.Backing removed and weld tabs removed (if required)	P	P	AISC 360 TABLE N5.4-3
8.Repair activities	P	P	AISC 360 TABLE N5.4-3
9.Document acceptance or rejection of welded joint or member	P	P	AISC 360 TABLE N5.4-3
10.No prohibited welds have been added without the approval of the EOR	P	P	AISC 360 TABLE N5.4-3
INSPECTION TASKS PRIOR TO BOLTING			
1.Manufacturer's certifications available for fastener materials	O	P	AISC 360 TABLE N5.6-1
2.Fasteners marked in accordance with ASTM requirements	O	O	AISC 360 TABLE N5.6-1
3.Correct fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	O	O	AISC 360 TABLE N5.6-1
4.Correct bolting procedure selected for joint detail	O	O	AISC 360 TABLE N5.6-1
5.Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	O	O	AISC 360 TABLE N5.6-1
6.Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used.	P	O	AISC 360 TABLE N5.6-1
7.Proper storage provided for bolts, nuts, washers and other fasteners components	O	O	AISC 360 TABLE N5.6-1
INSPECTION TASKS DURING BOLTING			
1.Fastener assemblies, of suitable condition, placed in all holes and washers are positioned as required	O	O	AISC 360 TABLE N5.6-2
2.Joint brought to the snug-tight condition prior to the pre-tensioning operation	O	O	AISC 360 TABLE N5.6-2
3.Fastener component not turned by the wrench prevented from rotating	O	O	AISC 360 TABLE N5.6-2
4.Fasteners are pre-tensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point toward the free edges	O	O	AISC 360-10 TABLE N5.6-2
INSPECTION TASKS AFTER BOLTING			
1.Document acceptance or rejection of bolted connections	P	P	AISC 360 TABLE N5.6-3
INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT			
1.Placement and installation of steel deck	P	P	AISC 360 TABLE N6.1
2.Placement and installation of steel headed stud anchors	P	P	AISC 360 TABLE N6.1
3.Document acceptance or rejection of steel element	P	P	AISC 360 TABLE N6.1

O - Observe these items on a random basis. Operations need not be delayed pending these inspections
P - Perform these tasks for each welded joint or member, each bolted connection, or each steel element

FOR PERMIT
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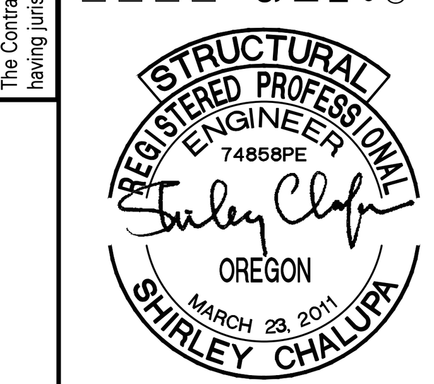


EXPIRES: 12-31-25

PROJECT NO.: 21031-0263
NORTH BAY FIRE SEISMIC GRANT
NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97220

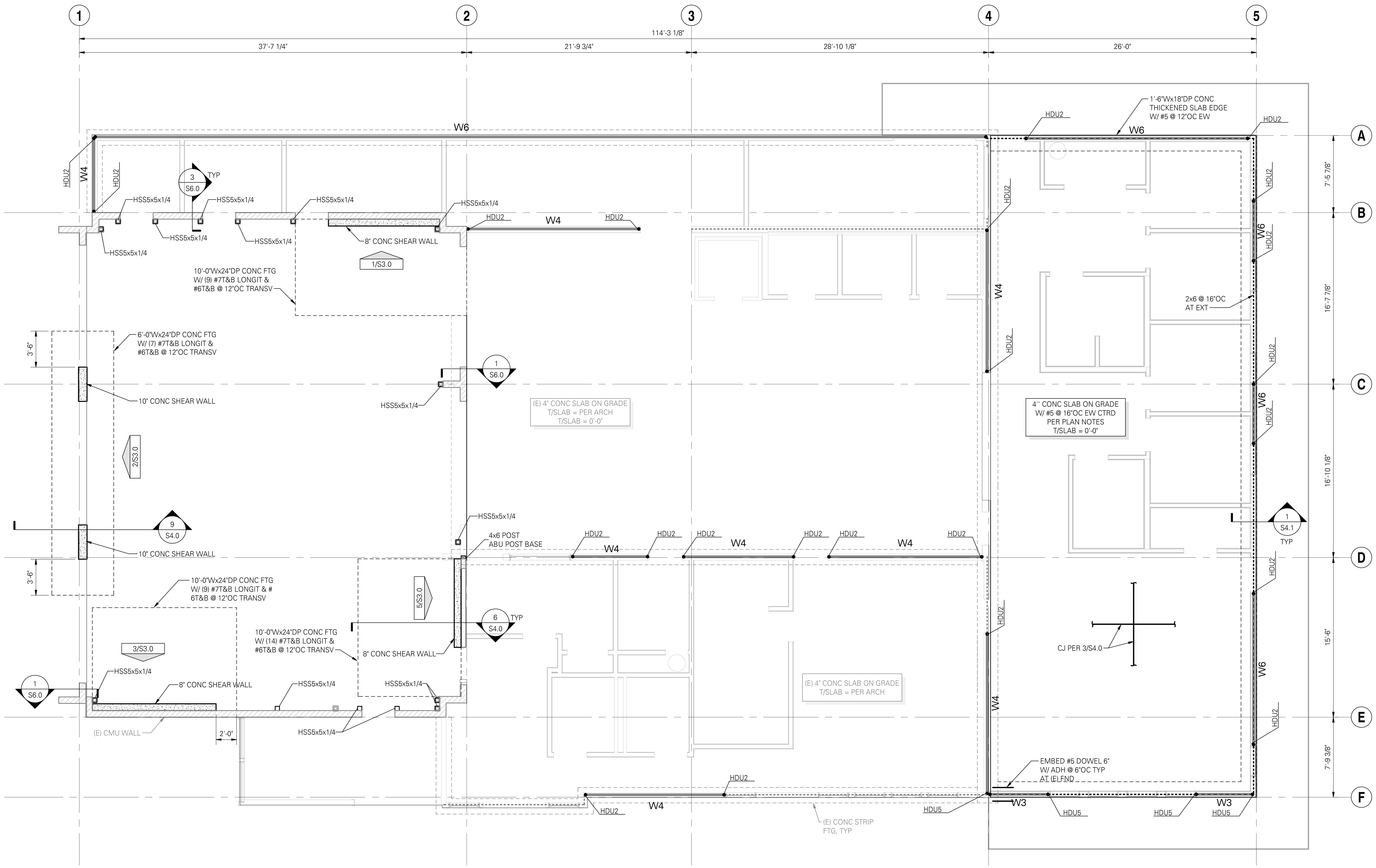
EXISTING

REVISIONS:
DATE DESCRIPTION

DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - SPECIAL INSPECTIONS

S1.4



FOUNDATION PLAN NOTES:

1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
3. CONTRACTOR SHALL LOCATE AND VERIFY THE FOLLOWING WITH OTHERS PRIOR TO POURING CONCRETE: ALL DOOR OPENINGS IN FOUNDATION WALLS; DRAINS AND SLOPES; BLOCKOUTS FOR PLUMBING, SPRINKLERS AND HVAC. ALL DUCTS, CHASES AND PIPES PER MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS. STAIR DETAILS AND GUARDRAILS PER ARCHITECTURAL DRAWINGS.
4. TOP OF SLAB (T/SLAB) ELEVATION ASSUMED 0'-0". FOR ACTUAL T/SLAB ELEVATION REFER TO CIVIL AND ARCHITECTURAL DRAWINGS. PROVIDE 6 MIL VAPOR BARRIER BELOW SLAB AT INTERIOR SPACES. PROVIDE FREE-DRAINING GRANULAR FILL PER GEOTECH REPORT.
5. ALL FOOTINGS AND SLABS TO BEAR ON COMPETENT NATIVE SOIL AND/OR STRUCTURAL FILL. SUBGRADE PREPARATION, STRUCTURAL FILL, DRAINAGE SYSTEM, AND OTHER REQUIREMENTS PER GEOTECH REPORT AS NOTED IN THE STRUCTURAL GENERAL NOTES.
6. CJ INDICATES CONTROL JOINT PER PLAN.
7. TYPICAL DETAILS PER:

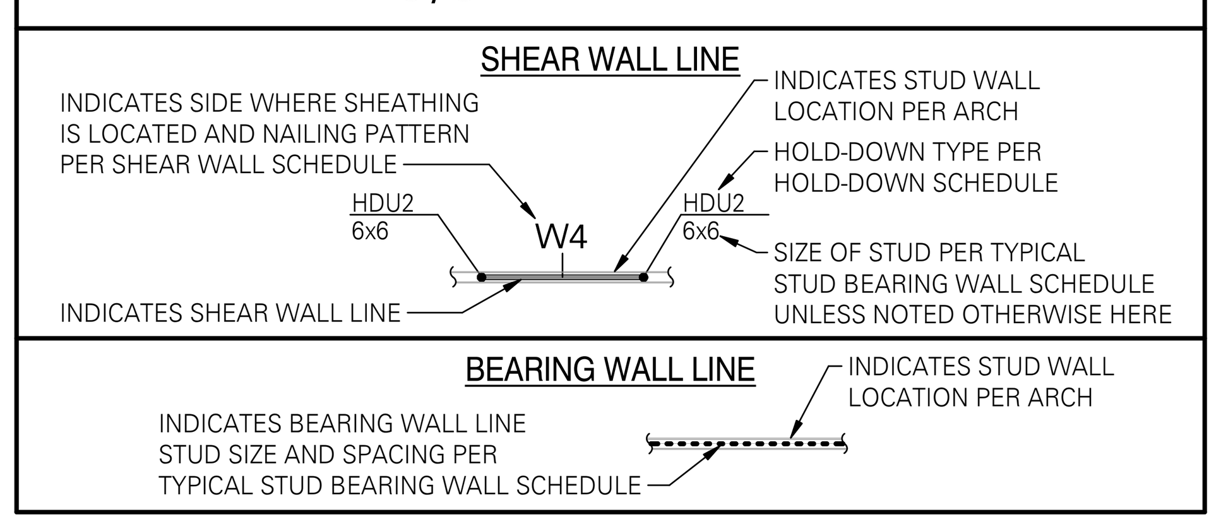
1/S4.0	STANDARD HOOKS AND BAR BENDS
2/S4.0	TYPICAL ANCHOR BOLT SCHEDULE
5/S4.0	TYPICAL CORNER REINFORCING AT CONCRETE FOOTINGS
8/S4.0	TYPICAL LAP SPLICE SCHEDULE
4/S4.1	TYPICAL PIPE EMBEDDED IN SLAB ON GRADE
5/S4.1	TYPICAL PIPE AND TRENCH LOCATIONSW AT CONCRETE STEM WALL
5/S5.0	TYPICAL CONCRETE WALL OPENING REINFORCEMENT
S5.0	CONCRETE SHEAR WALL DETAILS
S6.0	STEEL DETAILS

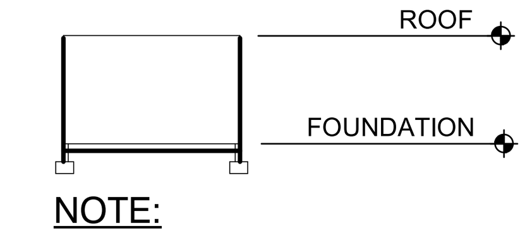
STUD AND SHEAR WALL PLAN NOTES:

1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. LUMBER GRADE PER STRUCTURAL GENERAL NOTES.
3. ALL INTERIOR NON-BEARING, NON-STRUCTURAL WALL STUD REQUIREMENTS PER STRUCTURAL GENERAL NOTES.
4. HEADERS SHOWN ON FRAMING PLAN SHALL BE SUPPORTED BY (1) TRIMMER AND (1) KING STUD MINIMUM, UNO. WHERE MORE THAN (1) TRIMMER IS REQUIRED, THE NUMBER OF TRIMMER STUDS SHALL BE NOTED THUS: ■-②. TRIMMERS TO BE CONTINUOUS TO THE FOUNDATION. BLOCK SOLID AT FLOOR FRAMING.
5. BEAMS SHOWN ON FRAMING PLAN SHALL BE SUPPORTED BY (2) BUNDLED STUDS MINIMUM, UNO. WHERE MORE THAN (2) BUNDLED STUDS ARE REQUIRED, THE NUMBER OF BUNDLED STUDS SHALL BE NOTED THUS: ■-③. BUNDLED STUDS TO BE CONTINUOUS TO THE FOUNDATION. BLOCK SOLID AT FLOOR FRAMING.
6. SHEAR WALL AND NAILING REQUIREMENTS PER SHEAR WALL SCHEDULE 11/S7.1. IT IS PERMISSIBLE TO INSTALL SHEATHING OVER THE TOP OF EXISTING PLYWOOD SIDING.
7. ALL EXTERIOR WALLS REQUIRING WOOD SHEATHING PER THE ARCHITECT SHALL BE SHEAR WALL TYPE W6 UNO.
8. HD / (2) 2x INDICATES HOLD-DOWN TYPE PER HOLD-DOWN SCHEDULE 1/S7.2.
9. TYPICAL HOLD-DOWN ELEVATION PER 3/S7.1.
10. ANCHOR BOLTS TO BE 5/8" DIA x 7" MINIMUM EMBEDMENT PER 6/S7.0. PROVIDE HOT-DIPPED GALVANIZED ANCHOR BOLTS AT PRESSURE-TREATED SILL PLATES.
11. TYPICAL DETAILS PER:

1/S7.0	TYPICAL STUD WALL OPENING (HEADER) DETAIL
2/S7.0	TYPICAL TOP PLATE SPLICE DETAIL
3/S7.0	TYPICAL BUNDLED STUDS NAILING
6/S7.0	TYPICAL SILL PLATE ANCHORAGE TO CONCRETE
9/S7.0	NON-STRUCTURAL PARTITION WALL CONNECTION [I-JOIST]
11/S7.0	TYPICAL HOLES AND NOTCHES IN WOOD STUDS
1/S7.1	TYPICAL FLOOR AND ROOF SHEATHING ATTACHMENT
2/S7.1	TYPICAL SHEAR WALL ELEVATION
9/S7.1	INTERSECTING SHEAR WALLS
11/S7.1	SHEAR WALL SCHEDULE
1/S7.2	HOLD-DOWN/STRAP SCHEDULE - DOUG-FIR STUDS

BEARING/SHEAR WALL LINE KEY





NOTE:
DARKENED LINES DESIGNATE AREA OF WORK.

FOUNDATION AND FIRST FLOOR STUD & SHEAR WALL PLAN

SCALE: 3/16" = 1'-0"

N

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FOR PERMIT
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REGISTERED PROFESSIONAL ENGINEER STRUCTURAL 74858PE OREGON MARCH 23, 2011 SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 21031-0263

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
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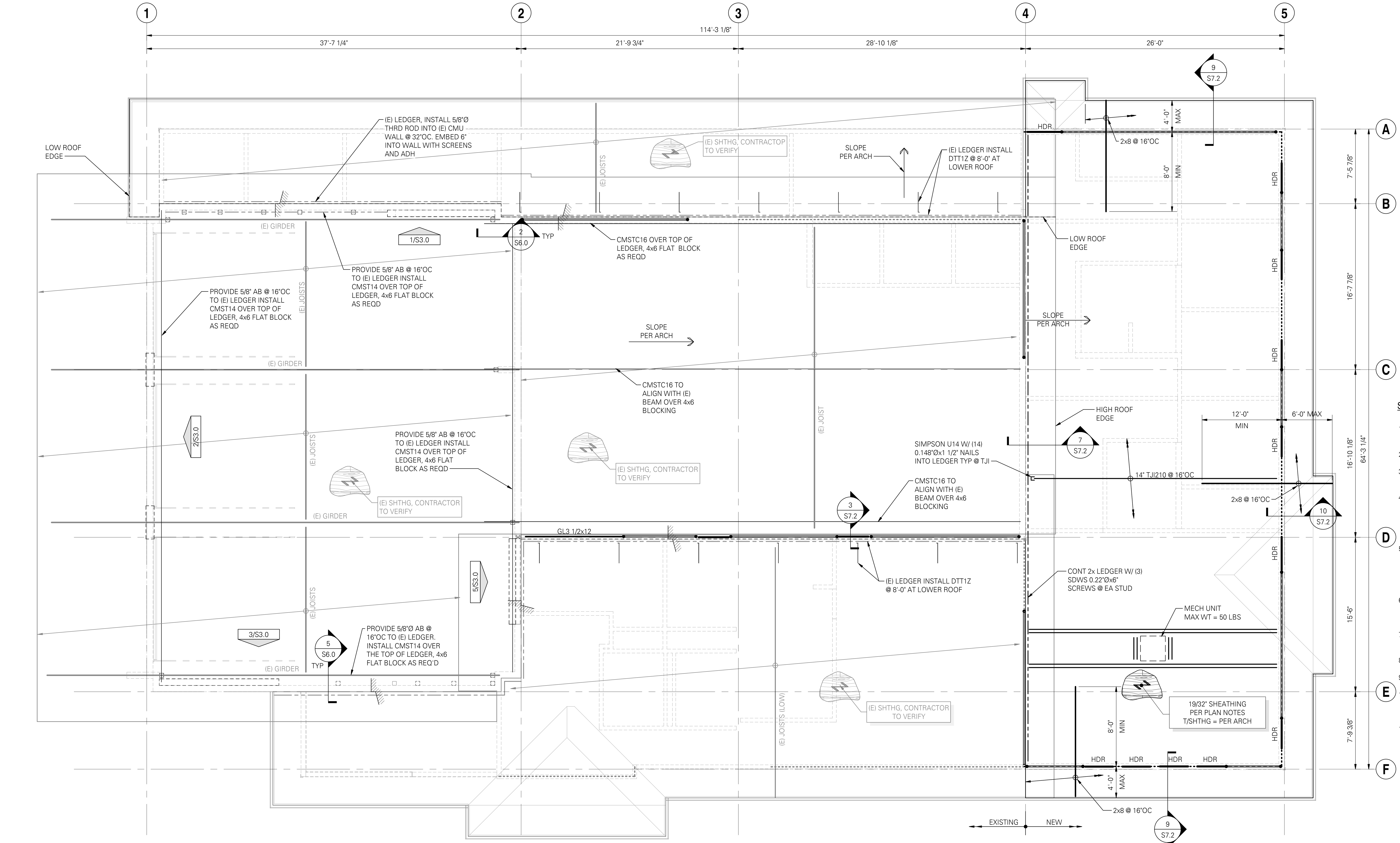
#	DATE	DESCRIPTION

DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - FOUNDATION AND FIRST FLOOR STUD & SHEAR WALL PLAN

S2.1



ROOF FRAMING PLAN

SCALE: 3/16" = 1'-0"

ROOF FRAMING PLAN NOTES:

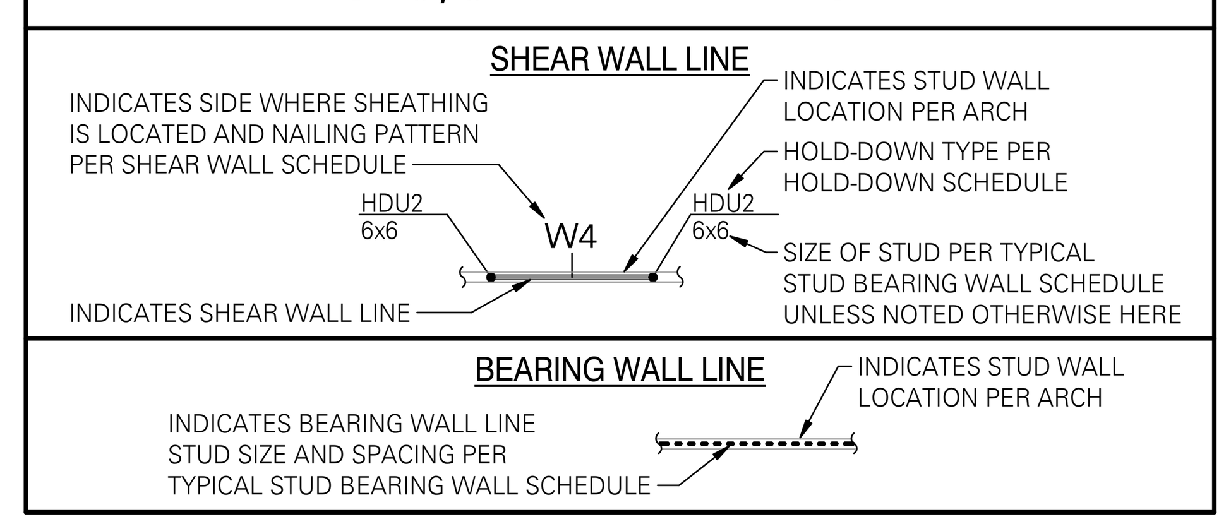
1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
3. ALL DUCTS, CHASES AND PIPES SHALL BE PER MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS. STAIR DETAILS AND GUARDRAILS PER ARCHITECTURAL DRAWINGS.
4. ROOF SHEATHING PER PLAN AND STRUCTURAL GENERAL NOTES. SHEATHING TO BE NAILED TO ROOF FRAMING WITH 0.131"DIAx2 1/2" NAILS @ 6"OC AT SUPPORTED PANEL EDGES AND @ 12"OC FIELD, UNO. LAY SHEATHING WITH FACE GRAIN (LONG DIRECTION) PERPENDICULAR TO SUPPORTS AND STAGGER PANEL END JOINTS. ALLOW 1/8" SPACE BETWEEN PANEL ENDS AND EDGES.
5. ALL 2x HANGERS TO BE FACE MOUNT TYPE LUS, UNO. GLULAM, PARALLAM AND MICROLLAM HANGERS ARE AS SPECIFIED ON PLAN. "I" JOIST HANGERS TO BE TOP FLANGE BEARING SIMPSON MIT OR ITS TYPE, UNO.
6. HEADERS SHOWN BUT NOT SPECIFIED ARE TO BE (2) 2x8 MINIMUM. HEADER SUPPORTS PER STUD AND SHEAR WALL PLAN ON FLOOR BELOW.
7. BEAMS ARE FLUSH FRAMED WITH JOISTS UNLESS NOTED OTHERWISE ON DETAILS, OR ON PLANS AS "DB" INDICATING THAT DROPPED BEAM FRAMING IS REQUIRED. BEAM SUPPORTS PER STUD AND SHEAR WALL PLAN ON LEVEL BELOW. PROVIDE A35 CLIP EACH SIDE OF FLUSH BEAMS THAT BEAR ON DOUBLE TOP PLATES.
8. PROVIDE SIMPSON H2.5A TIES AT ALL ROOF JOISTS, TYPICAL.
9. PROVIDE SOLID BLOCKING OVER ALL SHEAR WALLS AND BEARING WALLS. AT SHEAR WALLS PARALLEL TO FRAMING, ALIGN JOIST OR TRUSS OVER SHEAR WALL (ADDITIONAL JOISTS OR TRUSSES MAY BE REQUIRED).
10. ALL ROOF I-JOISTS TO BE TAPERED PER ARCHITECTURAL DRAWING.
11. ALL RIM JOISTS AND BLOCKING TO BE 1 1/2" LSL MINIMUM UNO.
12. BEARING STUD, SHEAR WALL, HOLD-DOWN, POST SIZE, AND POST CAP AND BASE REQUIREMENTS BELOW PER STUD AND SHEAR WALL PLAN.

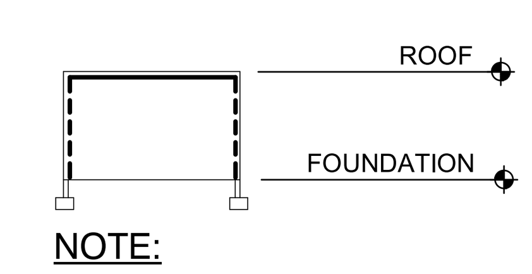
STUD AND SHEAR WALL PLAN NOTES:

1. STRUCTURAL GENERAL NOTES, DESIGN CRITERIA, ABBREVIATIONS AND LEGEND PER S1.1 - S1.4.
2. LUMBER GRADE PER STRUCTURAL GENERAL NOTES.
3. ALL INTERIOR NON-BEARING, NON-STRUCTURAL WALL STUD REQUIREMENTS PER STRUCTURAL GENERAL NOTES.
4. HEADERS SHOWN ON FRAMING PLAN SHALL BE SUPPORTED BY (1) TRIMMER AND (1) KING STUD MINIMUM, UNO. WHERE MORE THAN (1) TRIMMER IS REQUIRED, THE NUMBER OF TRIMMER STUDS SHALL BE NOTED THUS: ■-(2). TRIMMERS TO BE CONTINUOUS TO THE FOUNDATION. BLOCK SOLID AT FLOOR FRAMING.
5. BEAMS SHOWN ON FRAMING PLAN SHALL BE SUPPORTED BY (2) BUNDLED STUDS MINIMUM, UNO. WHERE MORE THAN (2) BUNDLED STUDS ARE REQUIRED, THE NUMBER OF BUNDLED STUDS SHALL BE NOTED THUS: ■-(3). BUNDLED STUDS TO BE CONTINUOUS TO THE FOUNDATION. BLOCK SOLID AT FLOOR FRAMING.
6. SHEAR WALL AND NAILING REQUIREMENTS PER SHEAR WALL SCHEDULE 11/S7.1. IT IS PERMISSIBLE TO INSTALL SHEATHING OVER THE TOP OF EXISTING PLYWOOD SIDING.
7. ALL EXTERIOR WALLS REQUIRING WOOD SHEATHING PER THE ARCHITECT SHALL BE SHEAR WALL TYPE W6 UNO.
8. HD / (2) 2x INDICATES HOLD-DOWN TYPE PER HOLD-DOWN SCHEDULE 1/S7.2.
9. TYPICAL HOLD-DOWN ELEVATION PER 3/S7.1.
10. ANCHOR BOLTS TO BE 5/8" DIA x 7" MINIMUM EMBEDMENT PER 6/S7.0. PROVIDE HOT-DIPPED GALVANIZED ANCHOR BOLTS AT PRESSURE-TREATED SILL PLATES.
11. TYPICAL DETAILS PER:

1/S7.0	TYPICAL STUD WALL OPENING (HEADER) DETAIL
2/S7.0	TYPICAL TOP PLATE SPLICE DETAIL
3/S7.0	TYPICAL BUNDLED STUDS NAILING
6/S7.0	TYPICAL SILL PLATE ANCHORAGE TO CONCRETE
9/S7.0	NON-STRUCTURAL PARTITION WALL CONNECTION [I-JOIST]
11/S7.0	TYPICAL HOLES AND NOTCHES IN WOOD STUDS
1/S7.1	TYPICAL FLOOR AND ROOF SHEATHING ATTACHMENT
2/S7.1	TYPICAL SHEAR WALL ELEVATION
9/S7.1	INTERSECTING SHEAR WALLS
11/S7.1	SHEAR WALL SCHEDULE
1/S7.2	HOLD-DOWN/STRAP SCHEDULE - DOUG-FIR STUDS

BEARING/SHEAR WALL LINE KEY





NOTE:
DARKENED LINES DESIGNATE AREA OF WORK.

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STRUCTURAL REGISTERED PROFESSIONAL ENGINEER 74858PE OREGON MARCH 23, 2011 SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 21031-0263

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
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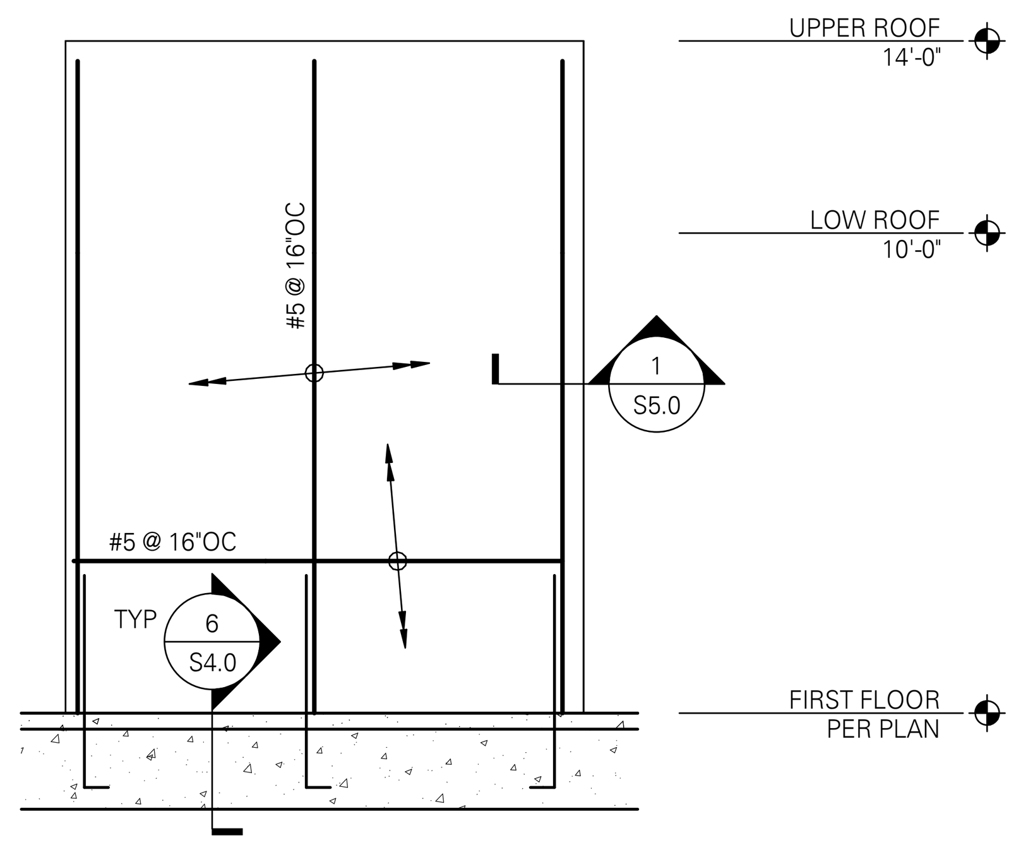
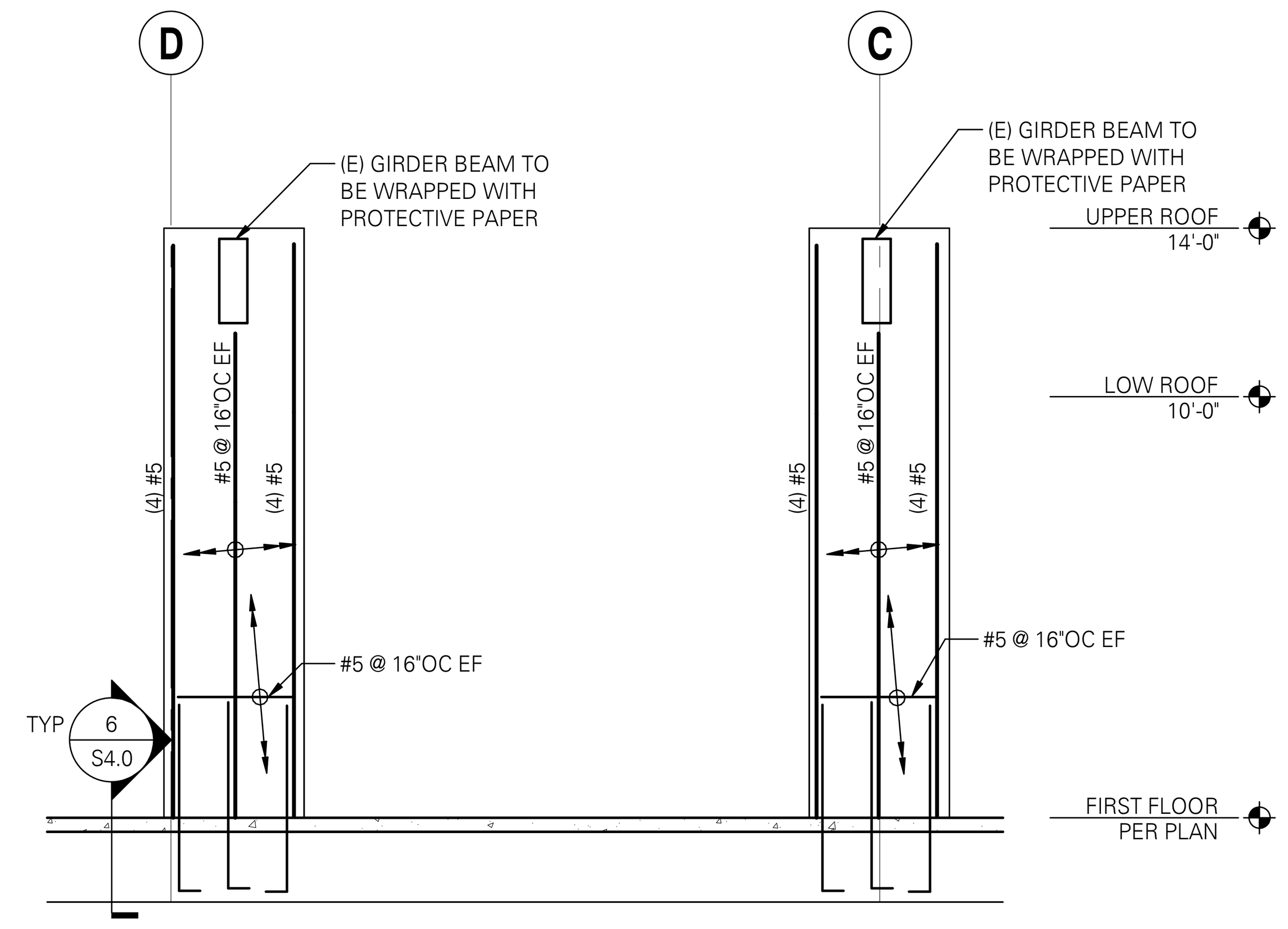
SHEET TITLE:
STRUCTURAL - ROOF FRAMING PLAN

S2.2



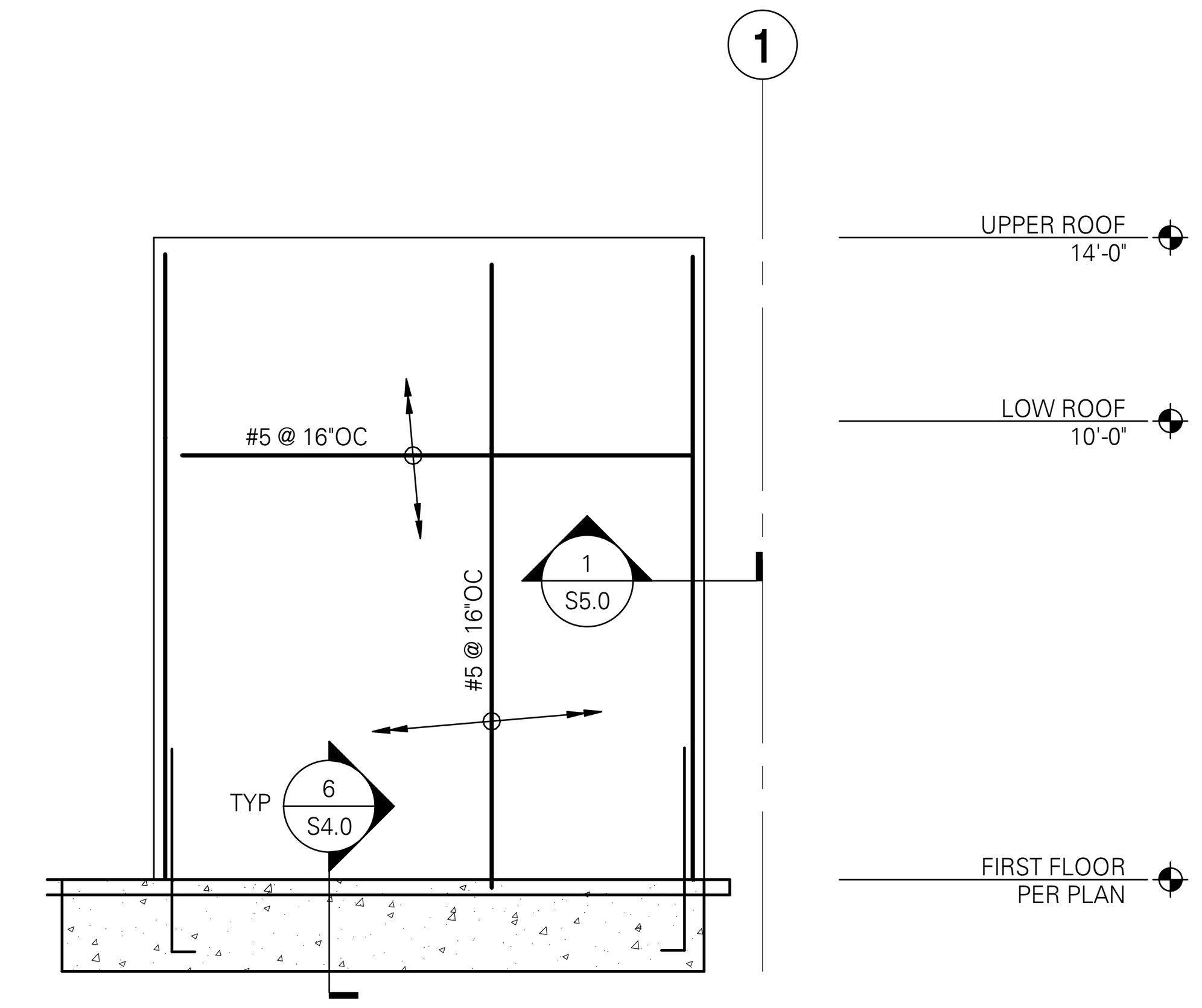
1 **GL B CONCRETE SHEAR WALL ELEVATION**

SCALE: 1/4" = 1'-0"



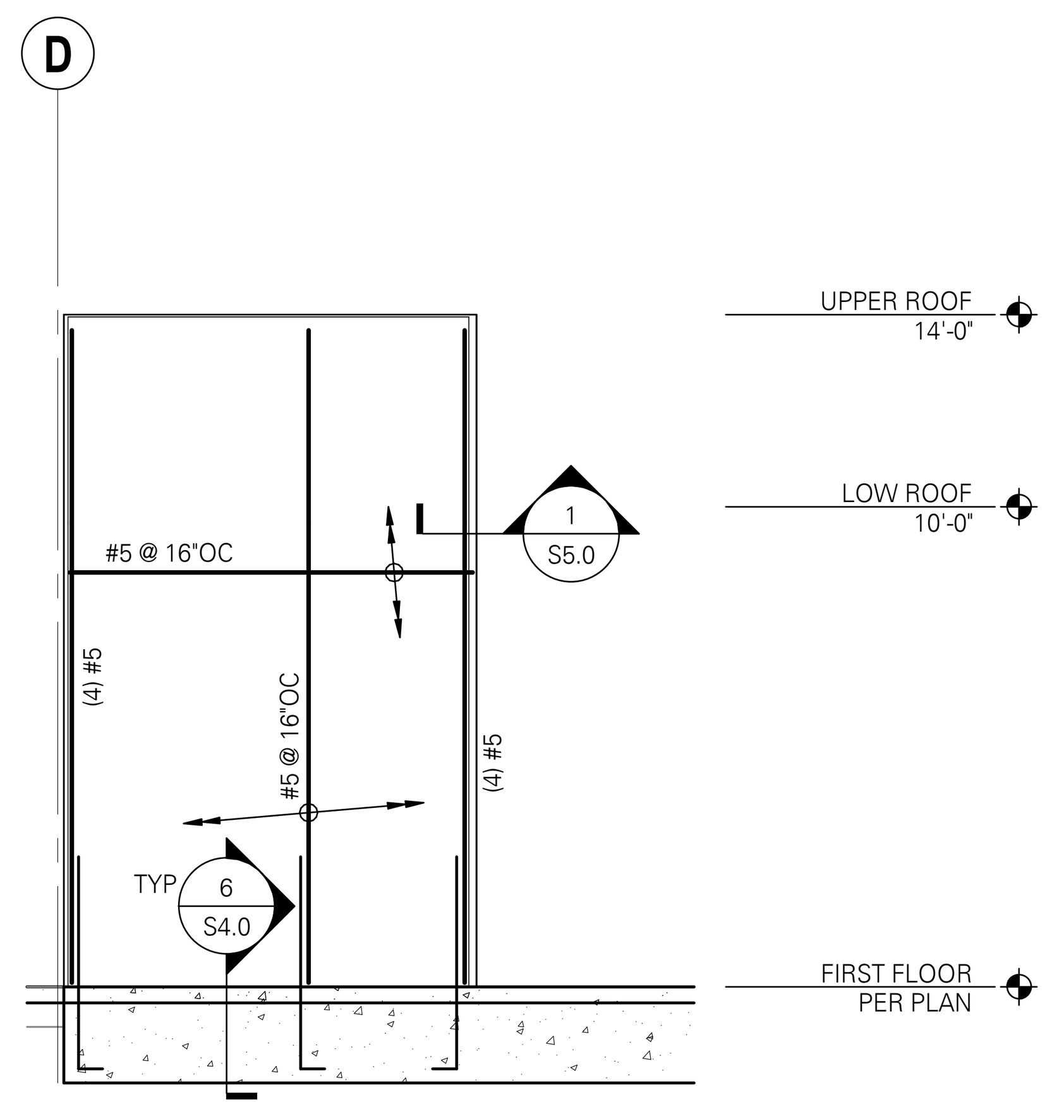
2 **GL 1 CONCRETE SHEAR WALL ELEVATION**

SCALE: 1/4" = 1'-0"



3 **GL D CONCRETE SHEAR WALL ELEVATION**

SCALE: 1/4" = 1'-0"



5 **GL 2 CONCRETE SHEAR WALL ELEVATION**

SCALE: 1/4" = 1'-0"

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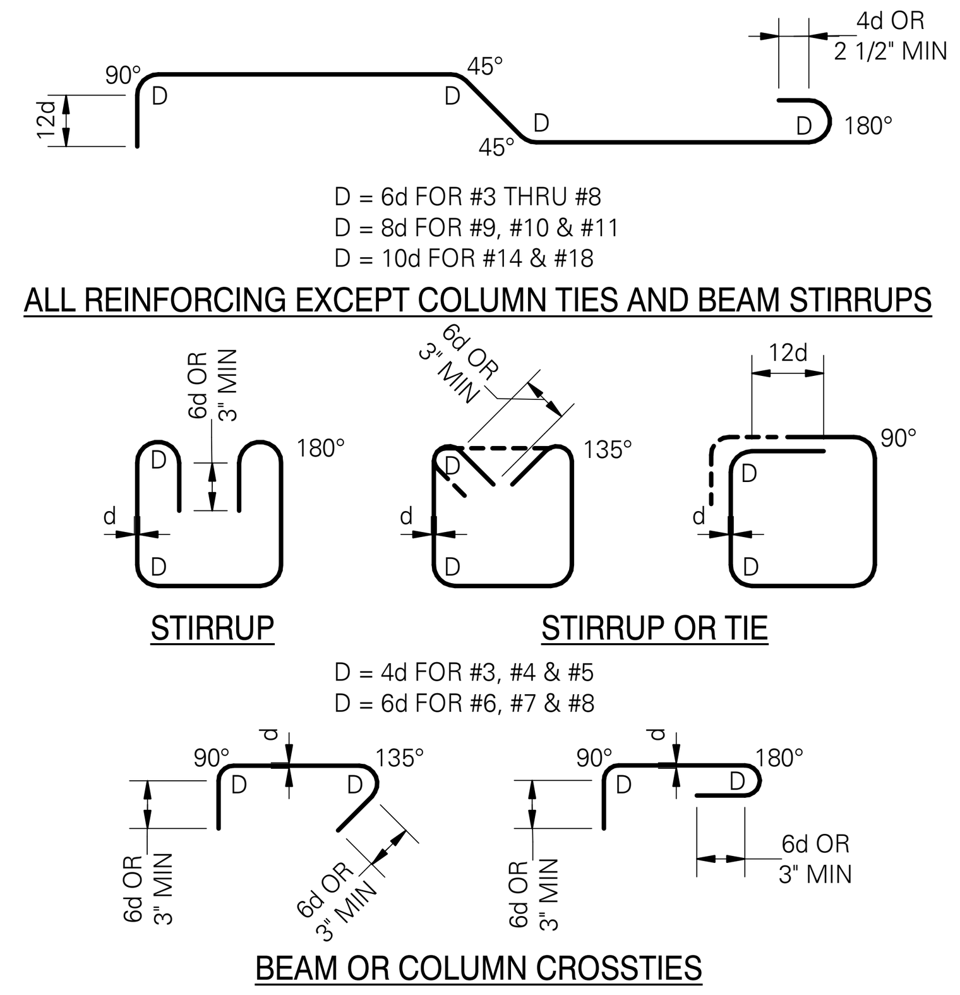
S3.0



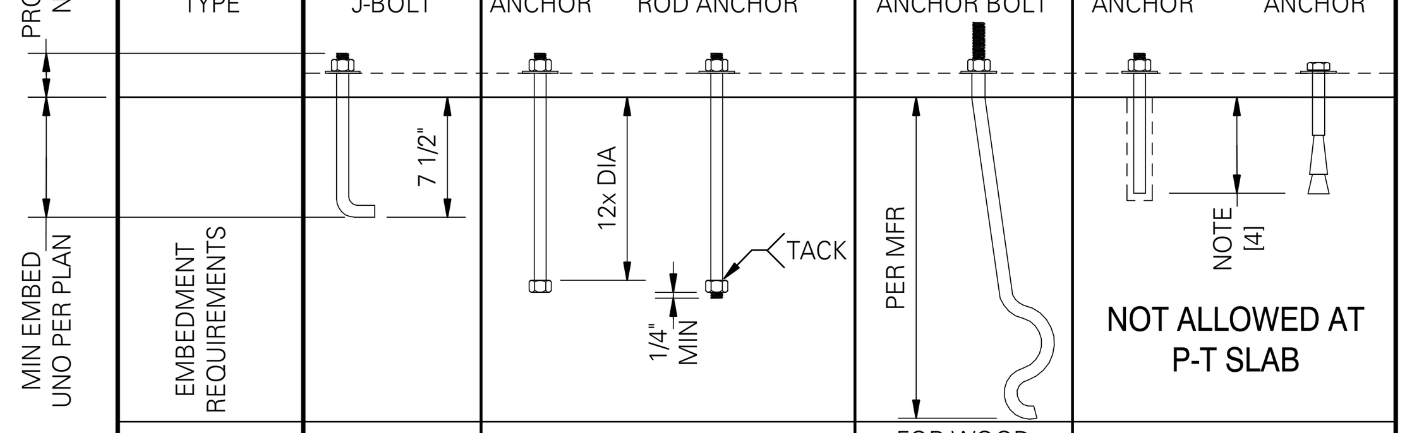
NOTE:

TIES AND CROSSTIES FOR SHEAR WALL BOUNDARY ELEMENTS SHALL BE DETAILED AS COLUMN TIES/CROSSTIES.

1 STANDARD HOOKS AND BENDS

SCALE: 3/4" = 1'-0" (03400)

01901	TYPICAL ANCHOR BOLT SCHEDULE				
INSTALLATION TYPE	CAST-IN-PLACE (PRE-AUTHORIZED) [2]			DRILL-IN OPTIONS (SUBMITTAL REQUIRED) [3]	
BOLT TYPE	STANDARD J-BOLT	HEADED ANCHOR / THREADED ROD ANCHOR	SIMPSON "SSTB" ANCHOR BOLT	ADHESIVE ANCHOR	EXPANSION ANCHOR
EMBEDMENT REQUIREMENTS	7 1/2"	12x DIA; 1/4" MIN; TACK	PER MFR	NOTE [4]; NOT ALLOWED AT P-T SLAB	
LIMITS	5/8"Ø MAX	5/8"Ø THRU 2 1/2"Ø	FOR WOOD FRAME ONLY	5/8"Ø THRU 1"Ø	
DIA = ANCHOR BOLT DIAMETER (NOMINAL)					



NOTES:

[1] CONTRACTOR SHALL DETERMINE THE REQUIRED THREAD PROJECTION SUITABLE FOR THE THICKNESS OF MATERIAL BEING FASTENED PLUS GROUT ALLOWANCE, IF ANY, AND CONSTRUCTION TOLERANCES, UNO.

[2] CONTRACTOR MAY SELECT APPROPRIATE CAST-IN-PLACE ANCHOR BOLT OPTION WITHOUT SUBMITTAL.

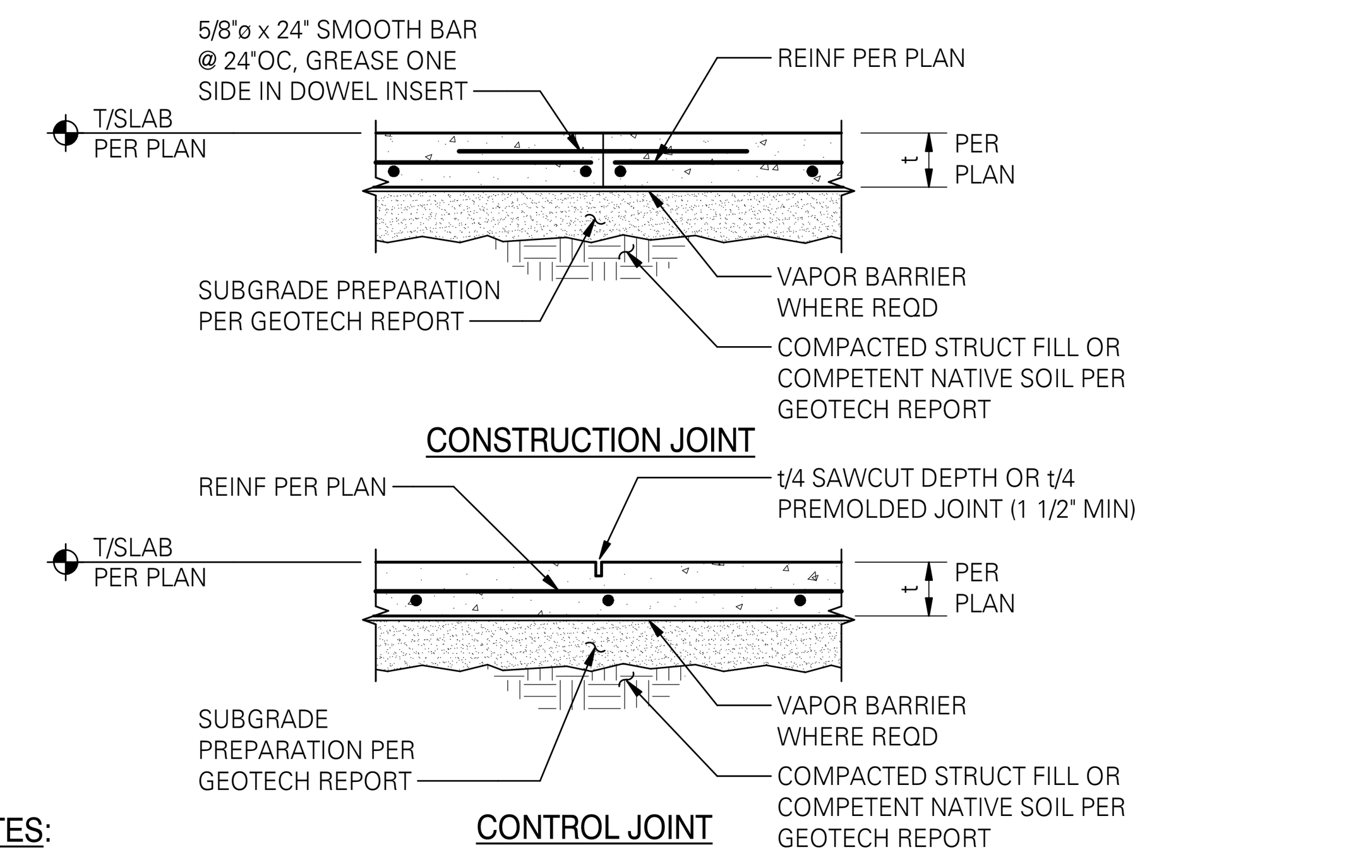
[3] DRILL-IN OPTIONS ARE NOT APPROPRIATE AT ALL CONDITIONS. IF DRILL-IN METHOD IS PREFERRED, SUBMIT MANUFACTURER'S INFORMATION, ALLOWABLE LOAD VS EMBEDMENT DATA AND LOCATIONS OF WHERE SUBSTITUTIONS ARE REQUESTED. ENGINEER WILL DETERMINE IF SUBSTITUTION IS APPROPRIATE FOR LOCATION AND LOADING.

[4] EMBEDMENT OF DRILL-IN ANCHORS SHALL BE PER ENGINEERS'S SUBMITTAL REVIEW COMMENTS. EMBEDMENT SHALL BE (9) NINE TIMES THE NOMINAL ANCHOR DIAMETER, UNO.

[5] AT PRESSURE TREATED SILLS, PROVIDE HOT-DIPPED GALVANIZED OR STAINLESS STEEL ANCHORS.

2 TYPICAL ANCHOR BOLT SCHEDULE

SCALE: 1" = 1'-0" (01901)



NOTES:

1. CONSTRUCTION JOINT IS A JOINT BETWEEN DIFFERENT POURS. CONTROL JOINT IS A CRACK CONTROL JOINT WITHIN THE SAME POUR.

2. USE "EARLY ENTRY DRY-CUT SAW" AS SOON AS POSSIBLE WITHOUT CAUSING RAVELING OF CONCRETE EDGES. SAWCUT ALONG SHORT DIRECTION OF POUR FIRST.

3. ALIGN A CONSTRUCTION OR CONTROL JOINT WITH RE-ENTRANT SLAB CORNERS, EACH WAY, TYPICAL.

4. CONSTRUCTION/CONTROL JOINT TO ENCLOSE APPROXIMATE SQUARE AREAS 225 SQUARE FEET MAXIMUM, WITH MAXIMUM PANEL ASPECT RATIO OF 1.3 TO 1.0.

5. CONTRACTOR TO SUBMIT CONSTRUCTION/CONTROL JOINT PLAN TO STRUCTURAL ENGINEER OF RECORD FOR REVIEW/APPROVAL.

3 TYPICAL SLAB ON GRADE JOINT DETAILS WITH REINFORCING

SCALE: 3/4" = 1'-0" (03201)

01400	GRADE 60 REINFORCING				
BAR SIZE	MISCELLANEOUS BARS		TOP BARS (see note #5)		HOOKED BARS
	Ld	Splice	Ld	Splice	Ldh
f'c = 3000psi					
#3	17	22	22	28	9
#4	22	29	29	38	11
#5	28	36	36	47	14
#6	33	43	43	56	17
#7	48	63	63	81	20
#8	55	72	72	93	22
#9	62	81	81	105	25
#10	70	91	91	118	28
#11	78	101	101	131	31
#14	93	N/A	121	N/A	38
#18	124	N/A	161	N/A	50
f'c = 4000psi					
#3	15	19	19	25	8
#4	19	25	25	33	10
#5	24	31	31	41	12
#6	29	37	37	49	15
#7	42	54	54	71	17
#8	48	62	62	81	19
#9	54	70	70	91	22
#10	61	79	79	102	25
#11	67	87	87	114	27
#14	81	N/A	105	N/A	33
#18	108	N/A	140	N/A	43
f'c = 5000psi					
#3	13	17	17	22	7
#4	17	23	23	29	9
#5	22	28	28	36	11
#6	26	34	34	44	13
#7	38	49	49	63	15
#8	43	56	56	72	17
#9	48	63	63	81	20
#10	54	71	71	92	22
#11	60	78	78	102	24
#14	72	N/A	94	N/A	29
#18	96	N/A	125	N/A	39

NOTES:

1. ALL TABULATED VALUES ARE IN INCHES.

2. VALUES FOR UNCOATED REINFORCING AND NORMAL WEIGHT CONCRETE WITH CLEAR SPACING > db, CLEAR COVER > db AND MINIMUM STIRRUPS OR TIES THROUGHOUT Ld OR CLEAR SPACING > 2db AND CLEAR COVER > db.

3. DEVELOP ALL REINFORCING IN STRUCTURAL SLABS WITH MINIMUM DEVELOPMENT LENGTH Ld.

4. Ldh = DEVELOPMENT LENGTH OF BAR WITH STANDARD HOOK.

5. TOP BAR = HORIZONTAL BAR WITH MORE THAN 12" OF FRESH CONCRETE BELOW OR AS NOTED ON DOCUMENTS AS "TOP BAR".

6. LAP SPLICE OF DIFFERENT SIZED BARS TO BE THE LARGER OF Ld OF THE LARGER BAR OR SPLICE LENGTH OF THE SMALLER BAR.

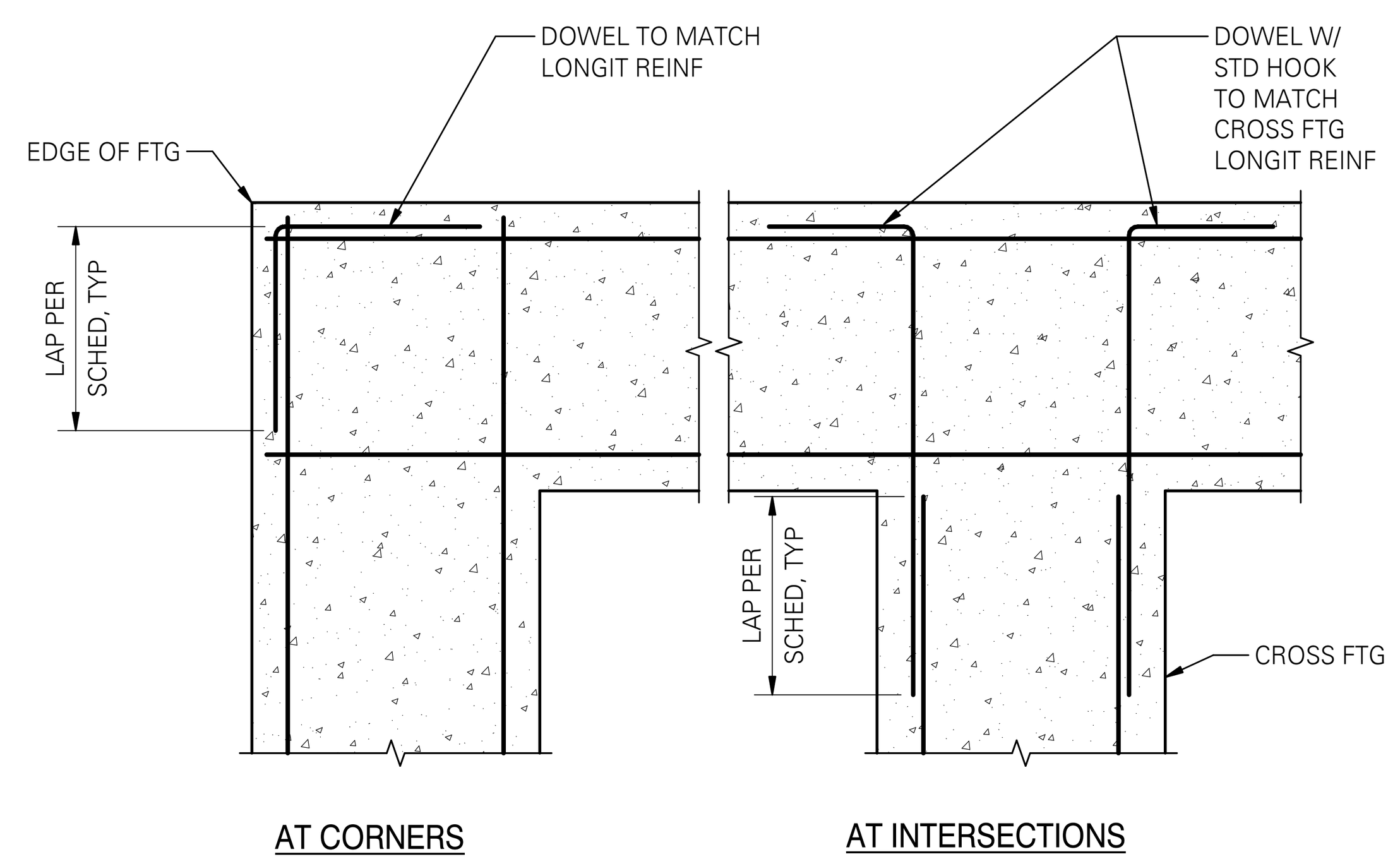
7. LAP SPLICE OF #14 AND #18 BARS IS NOT PERMITTED. LAP SPLICE OF SMALLER TO #14 AND #18 BARS IS NOT PERMITTED.

8. LAP SPLICE OF DIFFERENT GRADES OF REINFORCING TO BE THE LARGER OF Ld OF THE HIGHER GRADE BAR OR SPLICE LENGTH OF THE LOWER GRADE BAR.

9. SHEAR WALL REINFORCING LAP SPLICE SCHEDULE PER 8/S5.0.

8 TYPICAL LAP SPLICE AND DEVELOPMENT LENGTH SCHEDULE

SCALE: 3/4" = 1'-0" (01400M)



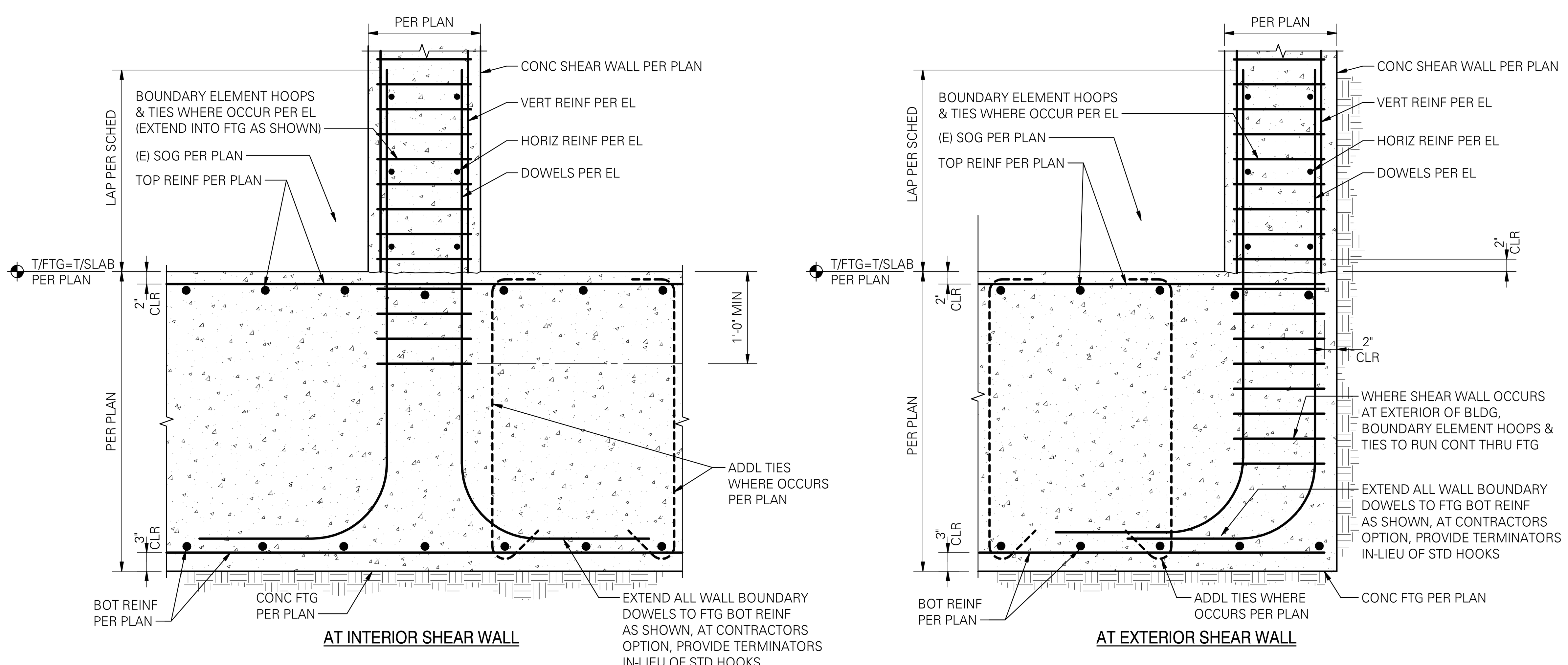
NOTE:

1. SPLICE LENGTHS PER LAP SPLICE AND DEVELOPMENT LENGTH SCHEDULE.

2. FOOTING REINFORCING PER PLAN OR ELEVATIONS, SECTIONS AND DETAILS.

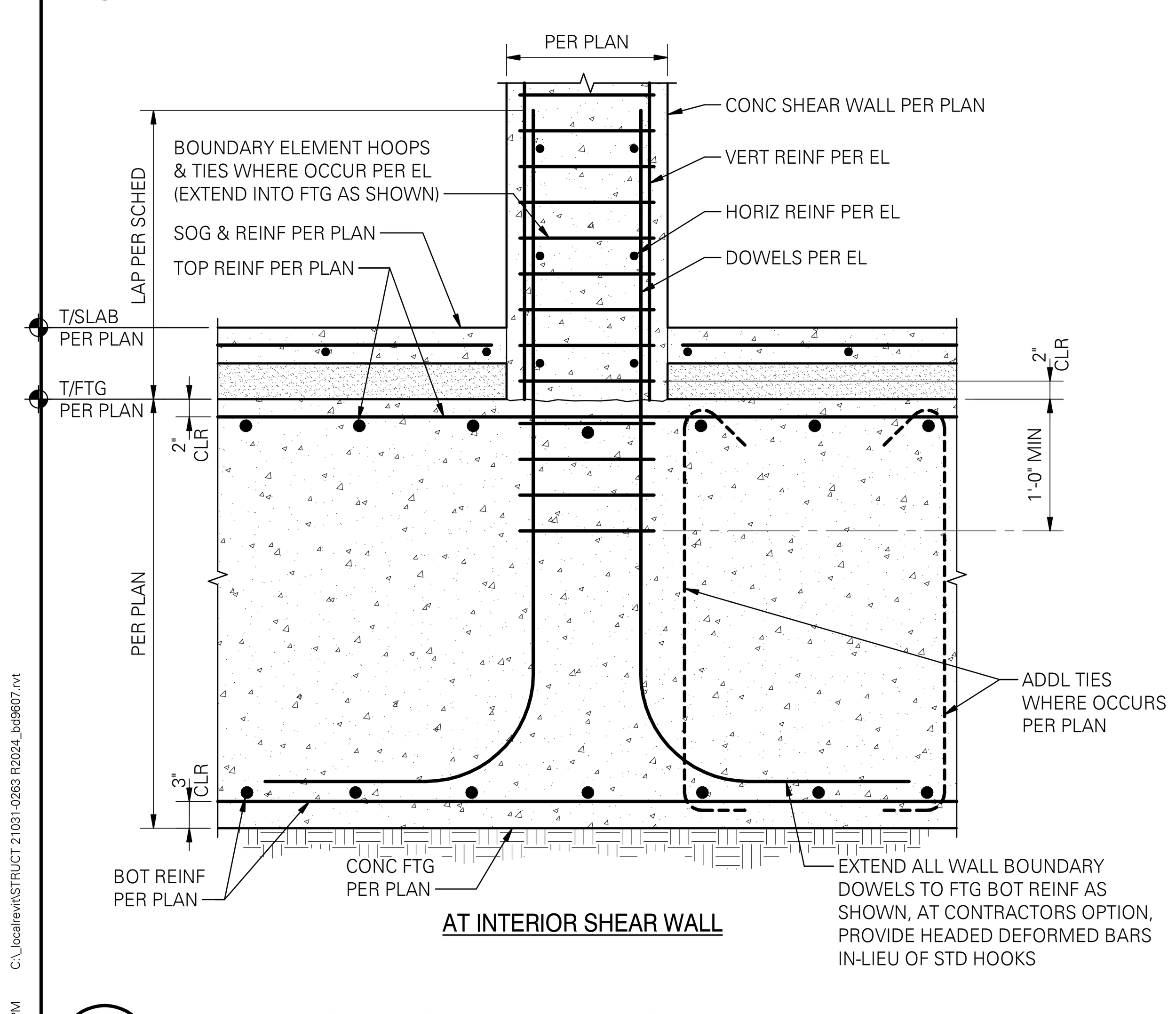
5 PLAN - TYPICAL CORNER REINFORCING AT CONCRETE FOOTINGS

SCALE: 3/4" = 1'-0" (03132)



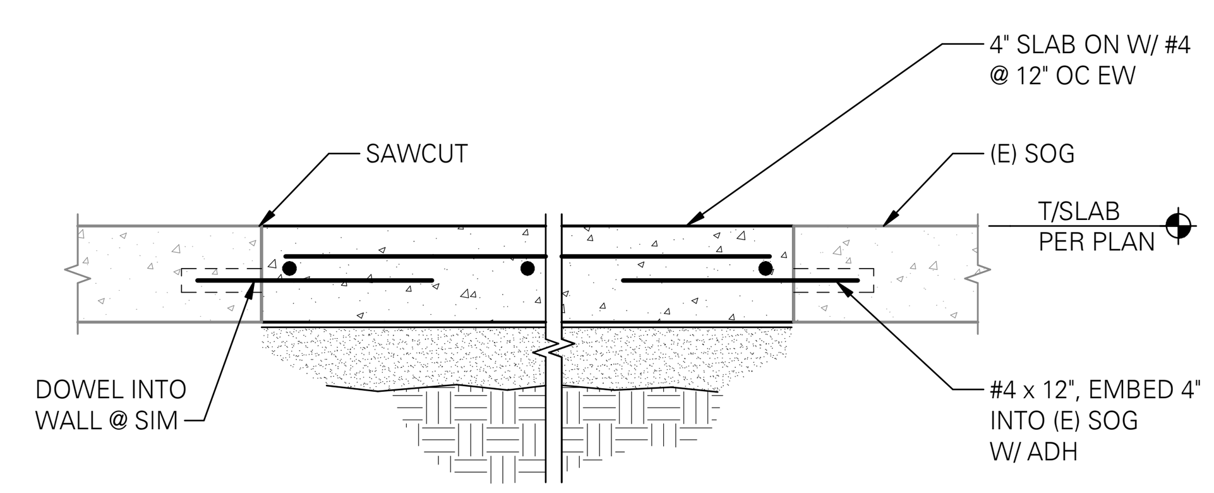
6 FOOTING AT CONCRETE SHEAR WALL

SCALE: 3/4" = 1'-0" (03136)



9 FOOTING AT CONCRETE SHEAR WALL

SCALE: 3/4" = 1'-0" (03136)



10 SLAB ON GRADE INFILL

SCALE: 1" = 1'-0"

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PROJECT NO.: 21031-0263

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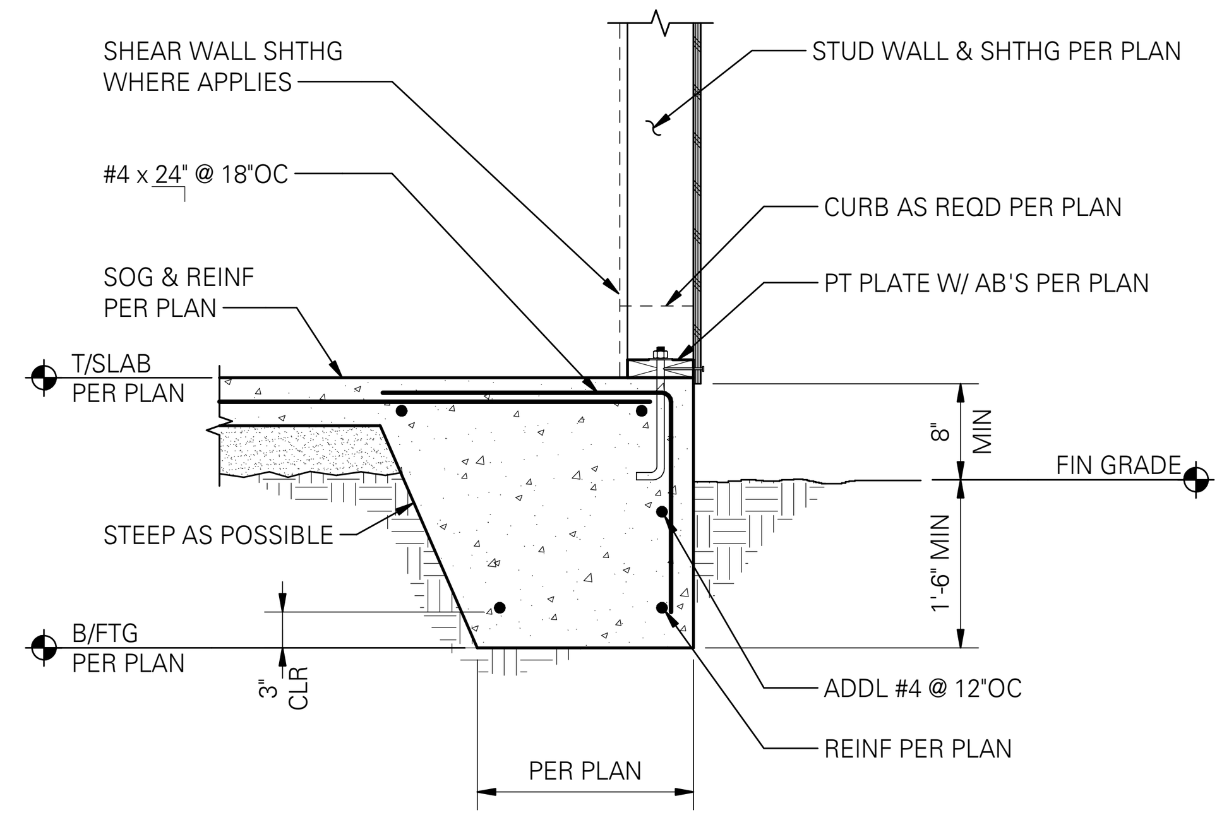
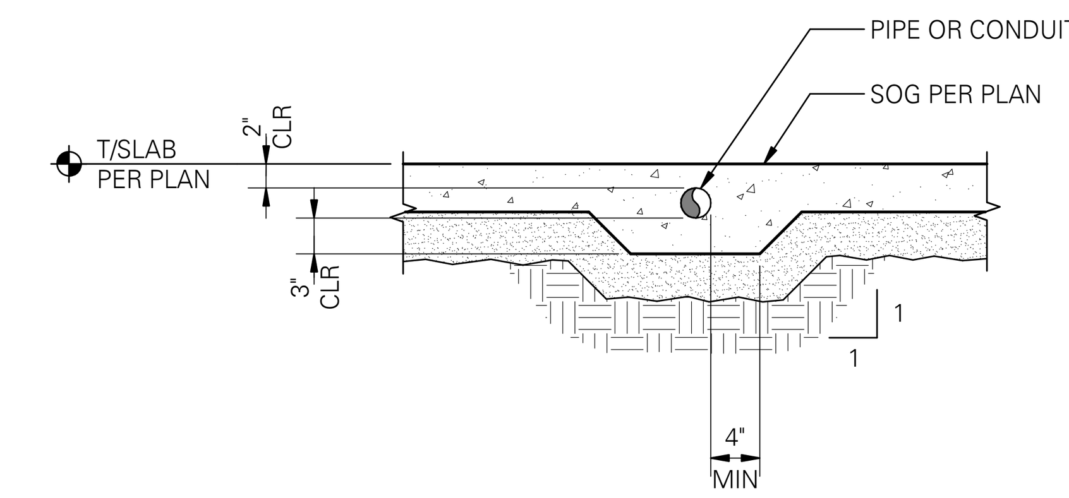


1 EXTERIOR THICKENED SLAB EDGE FOOTING AT STUD WALL

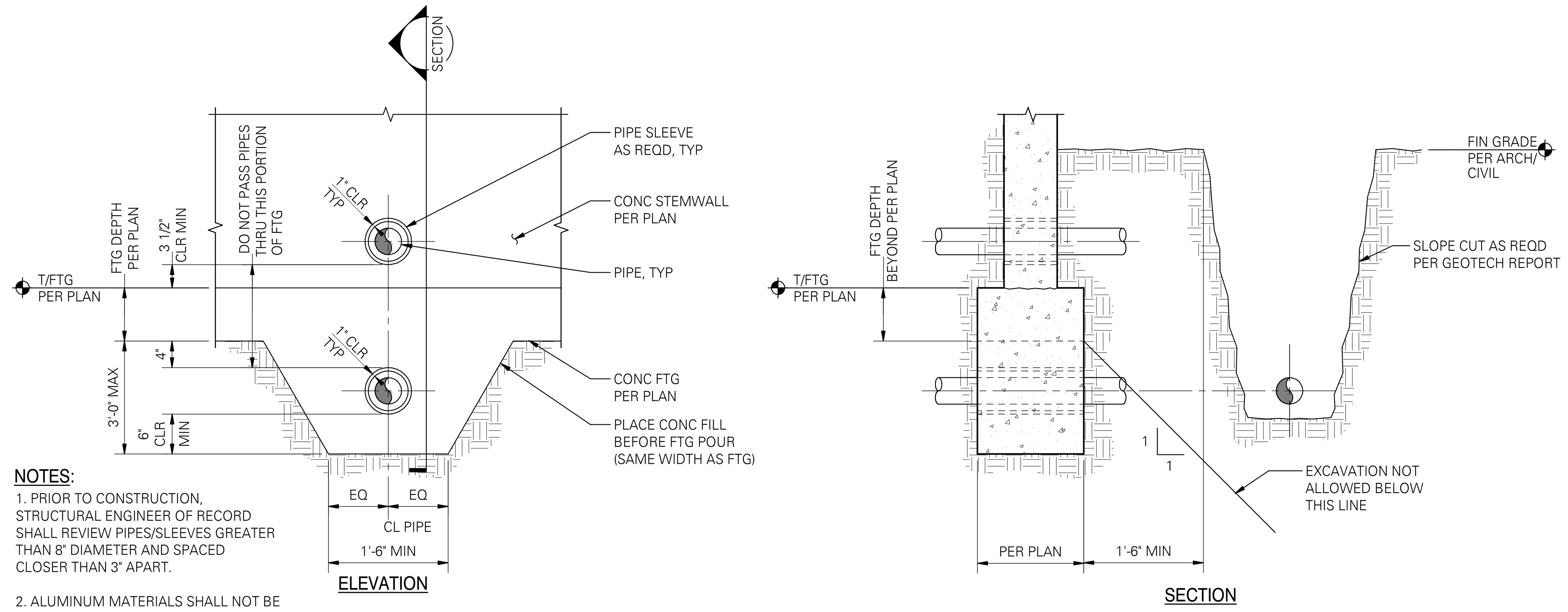
SCALE: 3/4" = 1'-0" (03030)



NOTE:
ALUMINUM MATERIALS SHALL NOT BE EMBEDDED IN CONCRETE.

4 PIPE OR CONDUIT EMBEDDED IN SLAB ON GRADE

SCALE: 3/4" = 1'-0" (03204)



NOTES:

1. PRIOR TO CONSTRUCTION, STRUCTURAL ENGINEER OF RECORD SHALL REVIEW PIPES/SLEEVES GREATER THAN 8" DIAMETER AND SPACED CLOSER THAN 3" APART.
2. ALUMINUM MATERIALS SHALL NOT BE EMBEDDED IN CONCRETE.

5 TYPICAL PIPE AND TRENCH LOCATIONS AT CONCRETE STEMWALL/FOOTING

SCALE: 3/4" = 1'-0" (03190)

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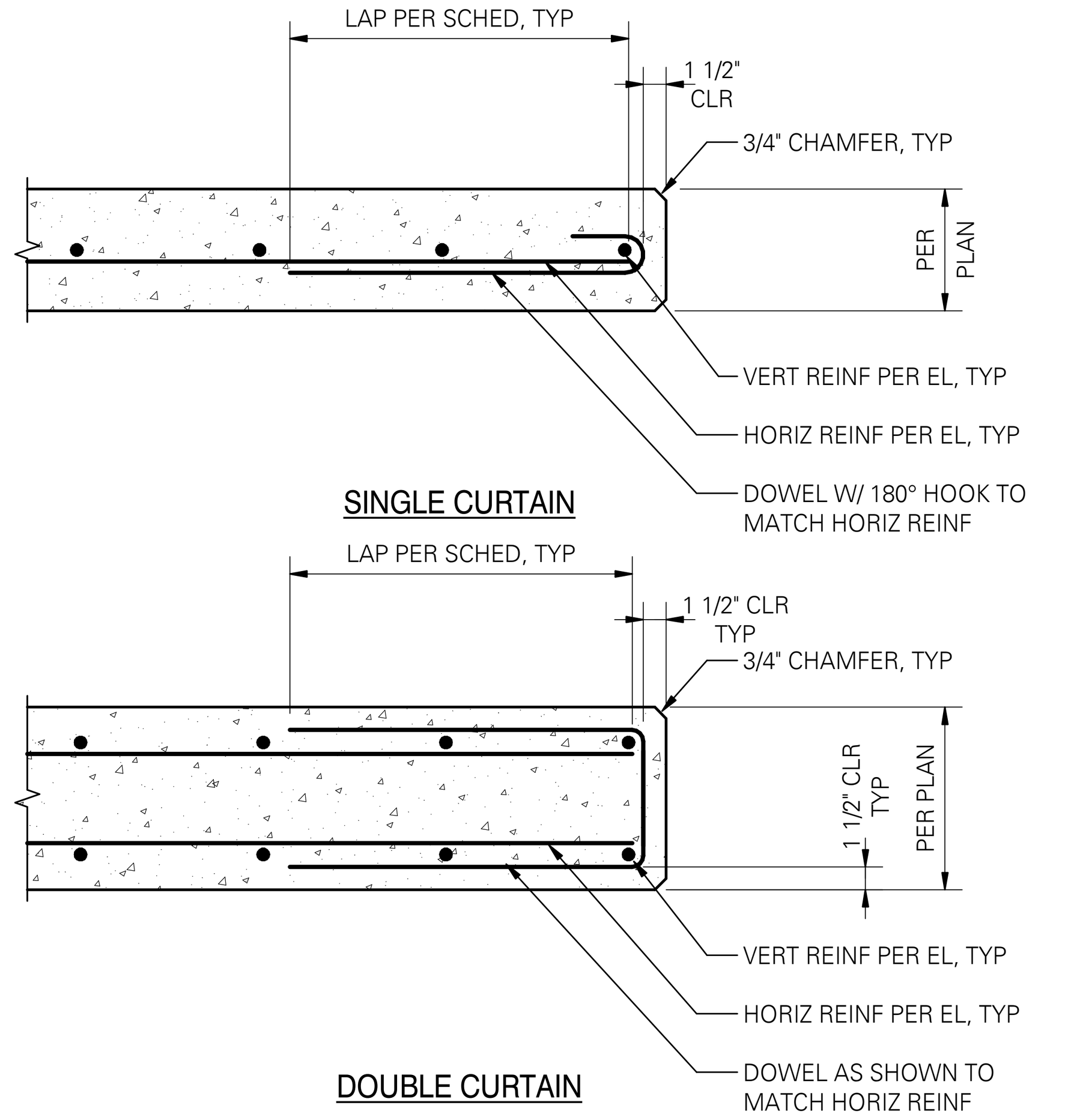
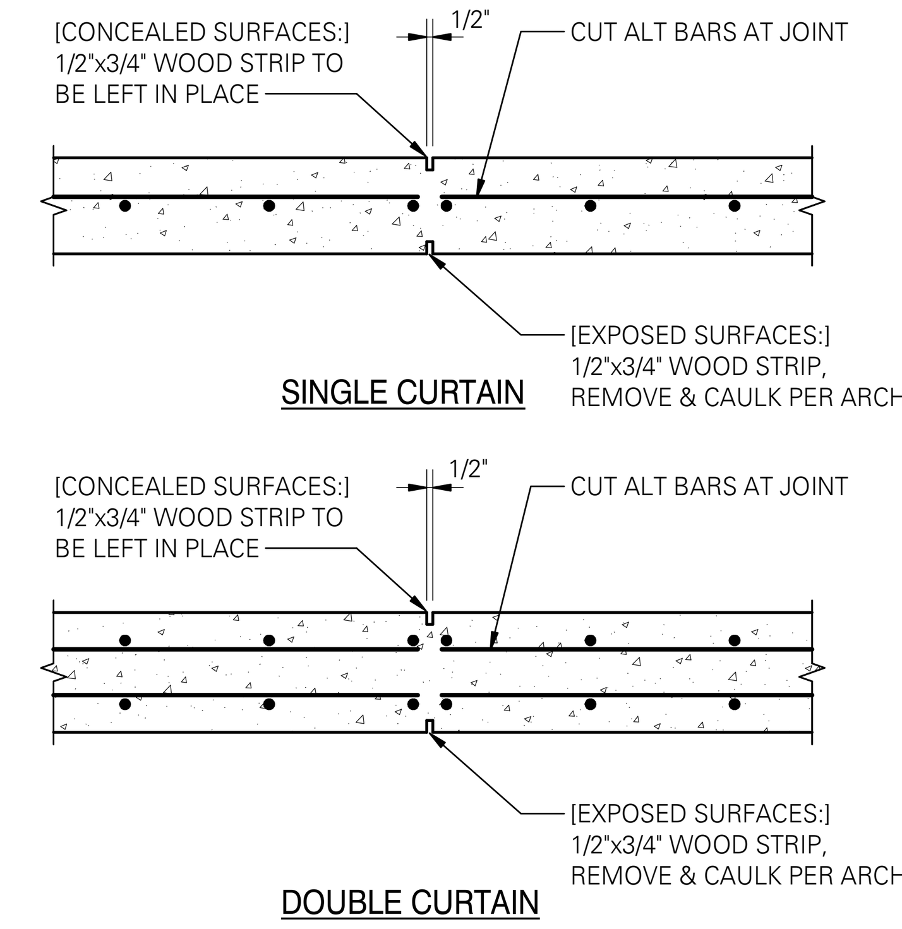
S4.1



1 PLAN - CONCRETE SHEAR WALL END

SCALE: 1" = 1'-0" (03410)



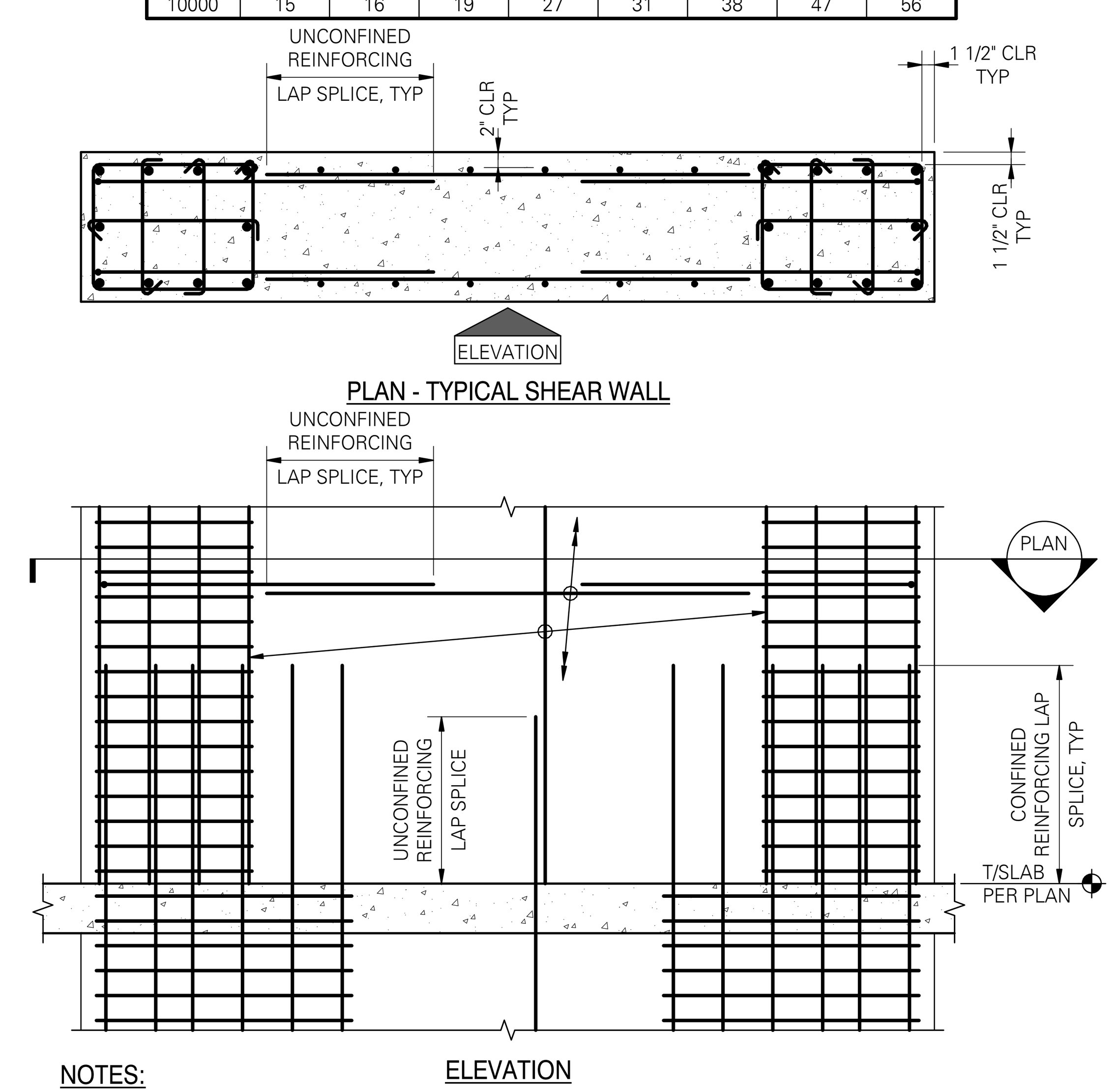
NOTES:
1. ESTABLISH CONTROL JOINTS AT 3x WALL HEIGHT APART (20'-0"OC MAXIMUM SPACING).
2. JOINT LOCATION TO BE APPROVED BY ARCHITECT.

3 PLANS - TYPICAL VERTICAL CONTROL JOINT AT CONCRETE WALL

SCALE: 3/4" = 1'-0" (03406)

01404 CONFINED REINFORCING								
f'c (psi)	#4	#5	#6	#7	#8	#9	#10	#11
4000	19	24	28	41	47	53	60	72
5000	17	21	25	37	42	47	54	65
6000	16	19	23	34	38	43	49	59
7000	15	18	21	31	35	40	45	55
8000	15	17	20	29	33	37	43	51
8500	15	16	20	28	32	36	41	50
9000	15	16	19	27	31	35	40	48
10000	15	15	18	26	30	33	38	46

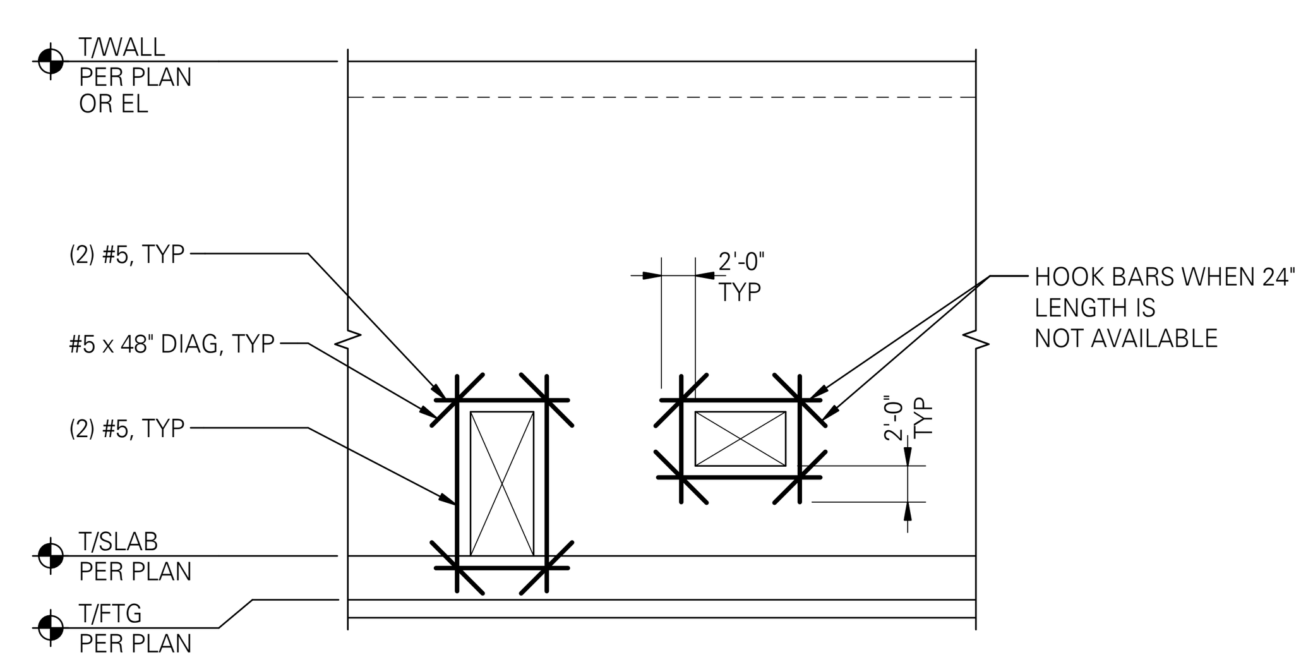
UNCONFINED REINFORCING								
f'c (psi)	#4	#5	#6	#7	#8	#9	#10	#11
4000	20	25	29	43	49	60	74	89
5000	18	22	26	38	44	54	66	80
6000	16	20	24	35	40	49	61	73
7000	15	19	22	32	37	46	56	67
8000	15	18	21	30	35	43	53	63
8500	15	17	20	29	33	41	51	61
9000	15	17	20	29	33	40	50	59
10000	15	16	19	27	31	38	47	56



NOTES:
1. TABULATED VALUES ARE IN INCHES.
2. LAP SPLICE NOT PERMITTED AT #14 OR #18 BARS.

8 SHEAR WALL REINFORCING LAP SPLICE SCHEDULE

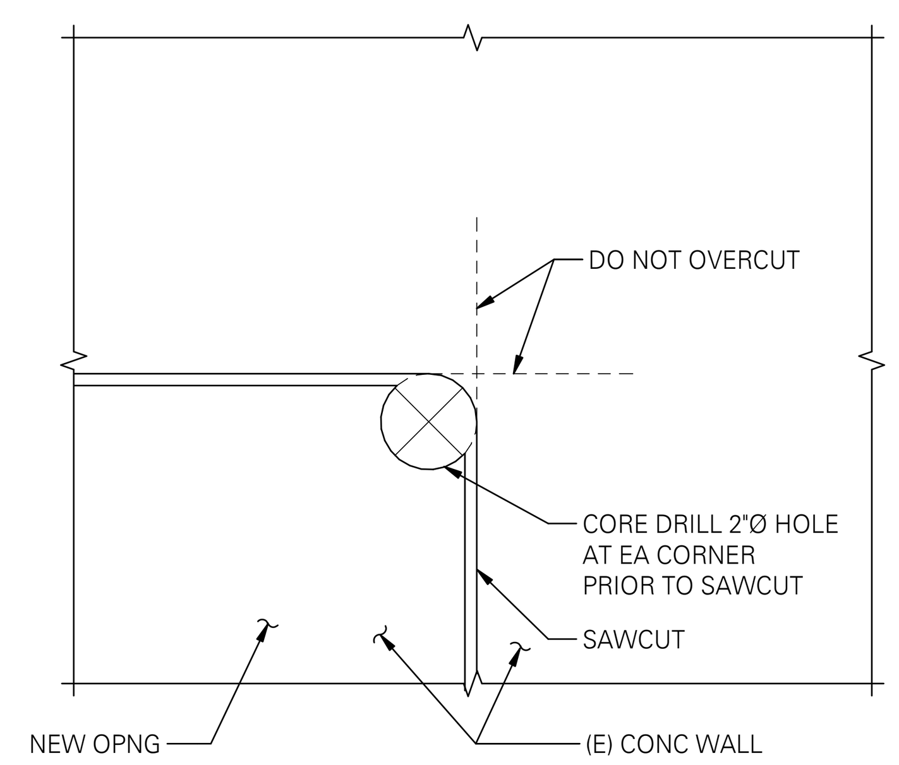
SCALE: 1/2" = 1'-0" (01404)



NOTE:
PROVIDE (2) #5 x 48" DIAGONAL BARS AT WALLS 10" OR THICKER.

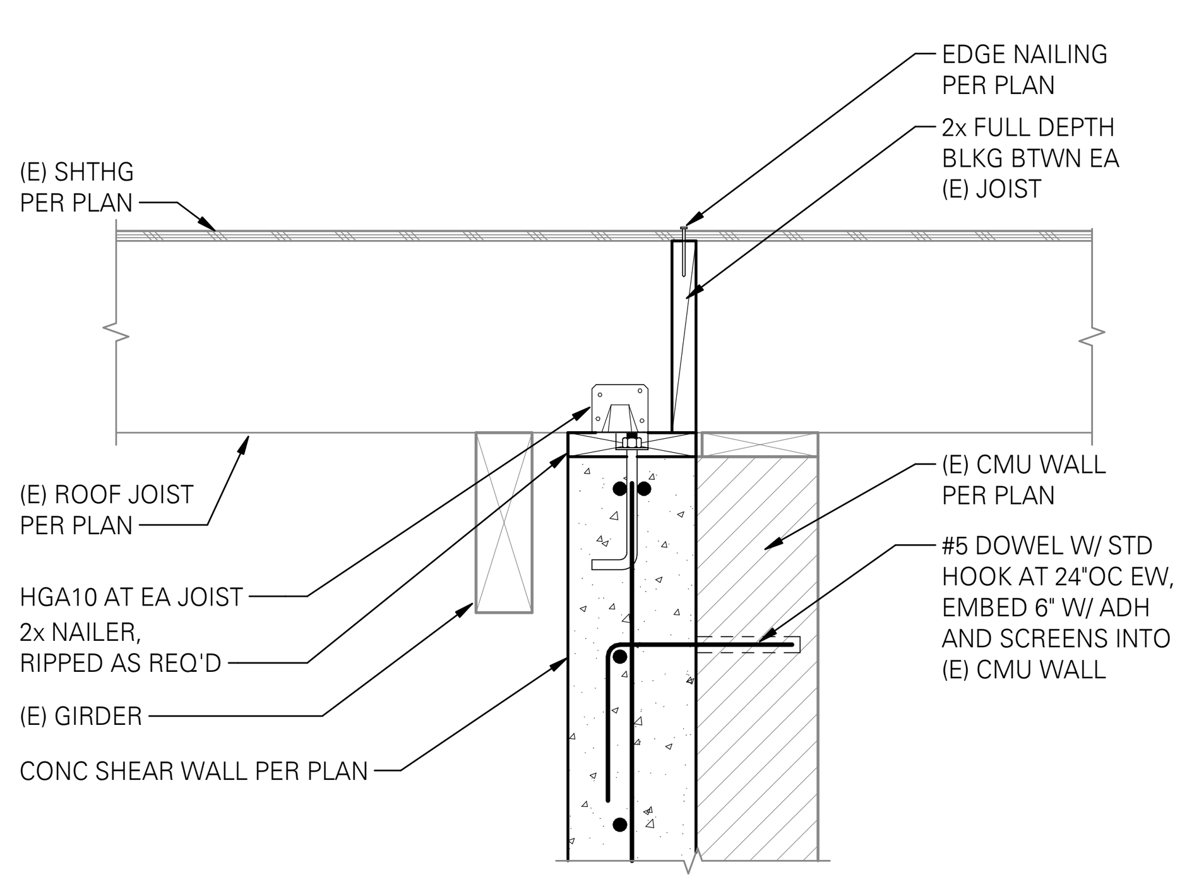
5 ELEVATION - TYPICAL CONCRETE WALL OPENING REINFORCING

SCALE: 3/4" = 1'-0" (03401)



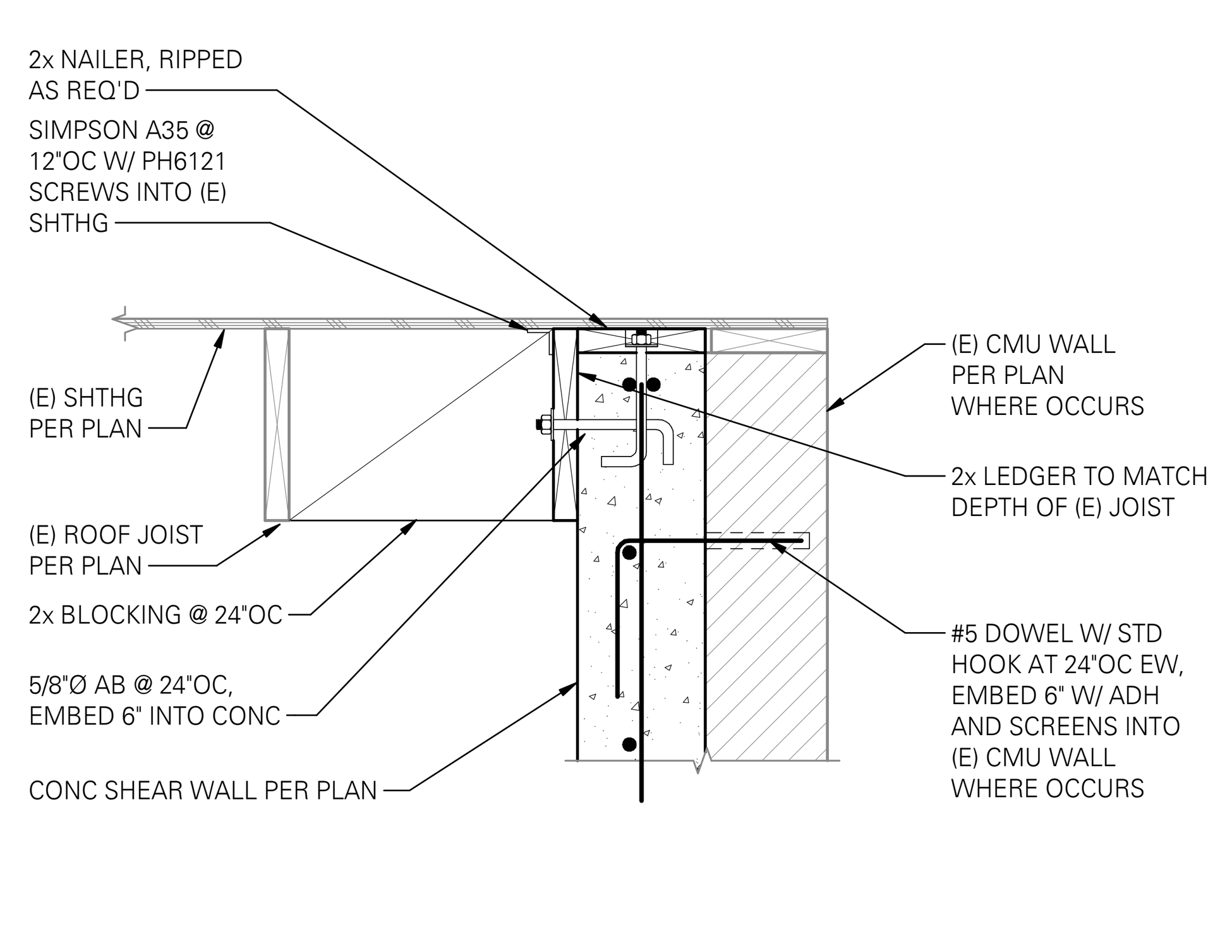
6 TYPICAL SAWCUT IN EXISTING CONCRETE WALL OR SLAB

SCALE: 3" = 1'-0" (03405)



9 CONCRETE SHEAR WALL PERPENDICULAR TO JOIST

SCALE: 1" = 1'-0"



10 CONCRETE SHEAR WALL PARALLEL TO JOIST

SCALE: 1" = 1'-0"

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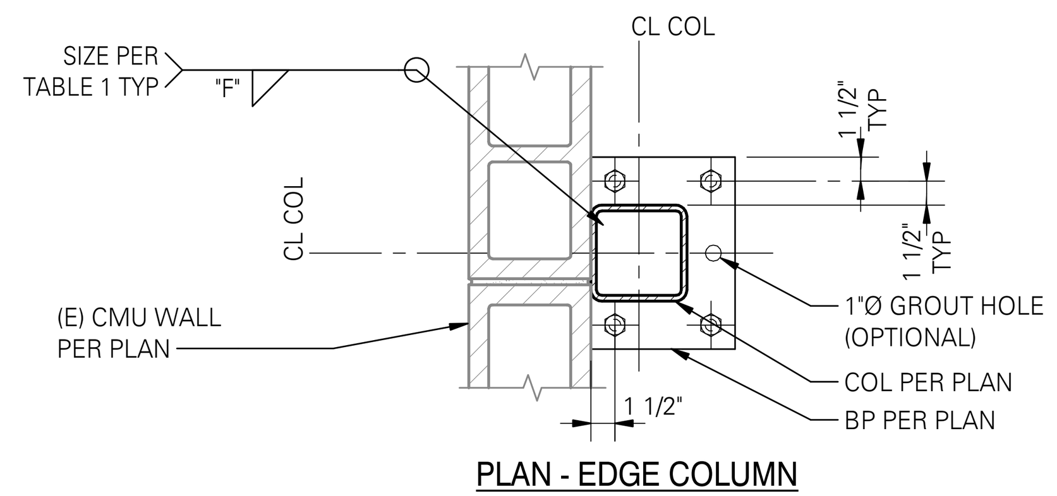
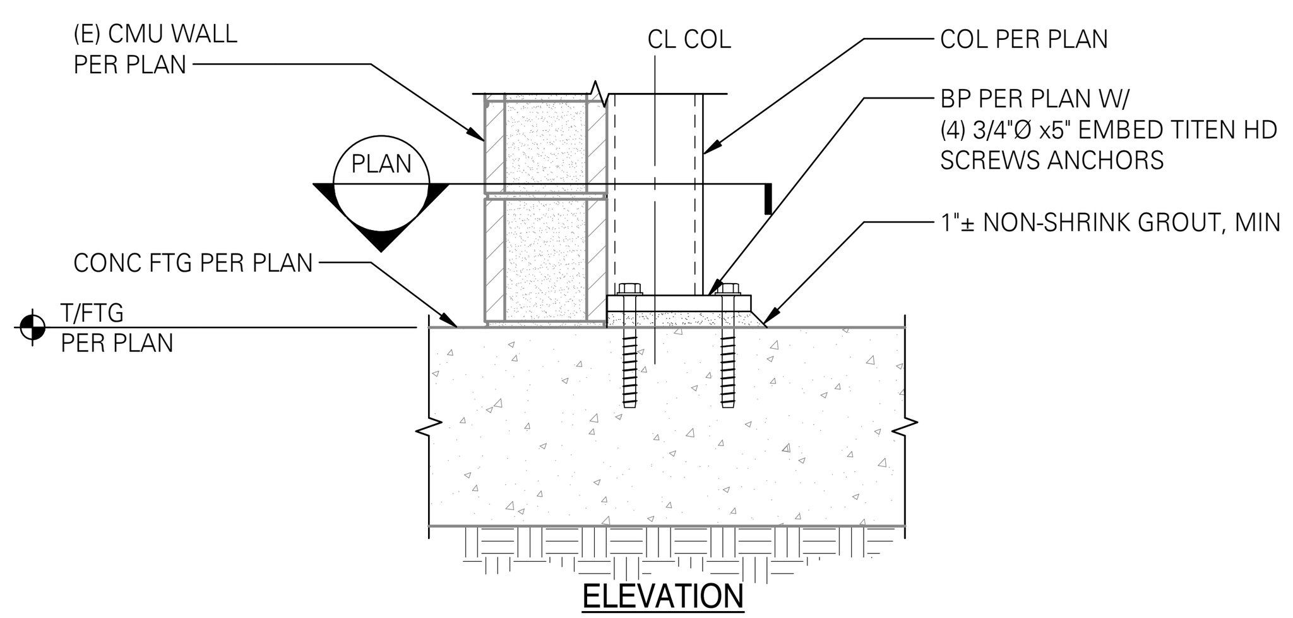
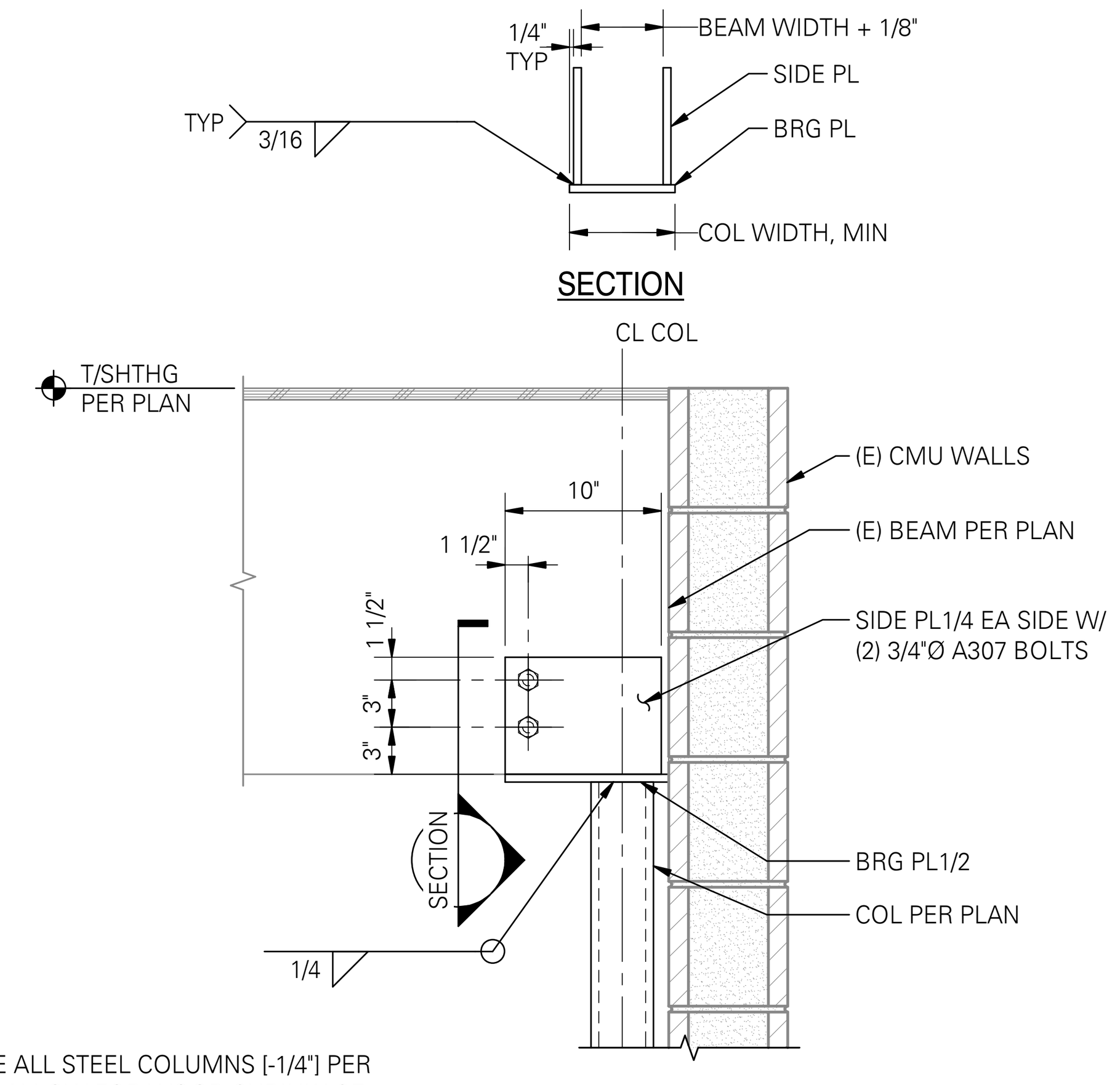


TABLE 1	
BASEPLATE THICKNESS	"F" FILLET SIZE
3/8" , 1/2"	3/16"
5/8" , 3/4"	1/4"
> = 7/8"	5/16"



1 TYPICAL BASEPLATE TO FOUNDATION CONNECTION - HSS COLUMN

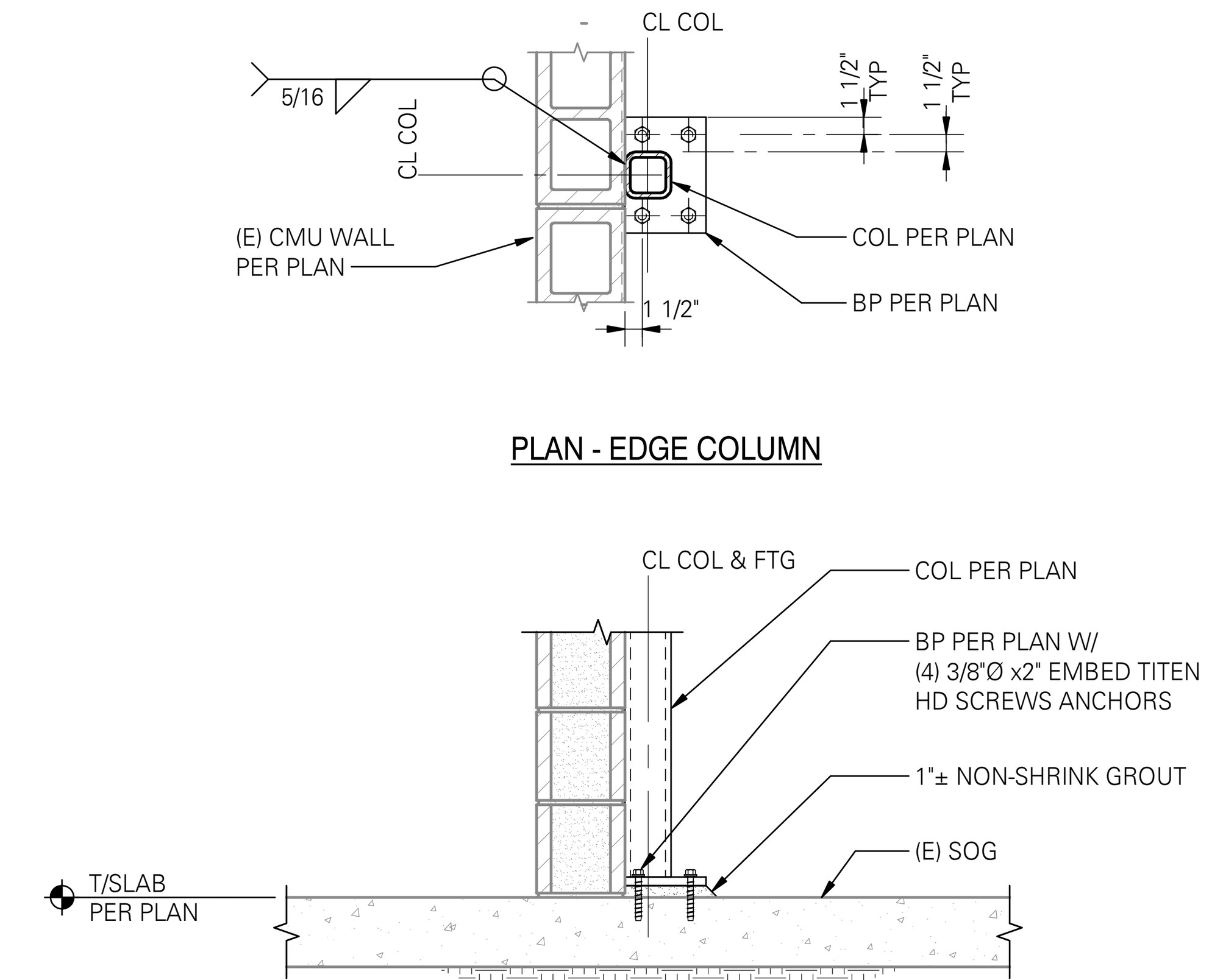
SCALE: 1" = 1'-0" (05030)



NOTE:
FABRICATE ALL STEEL COLUMNS [-1/4"] PER FLOOR TO ALLOW FOR WOOD SHRINKAGE.

2 TYPICAL BEAM SADDLE AT BEAM END - STEEL COLUMN

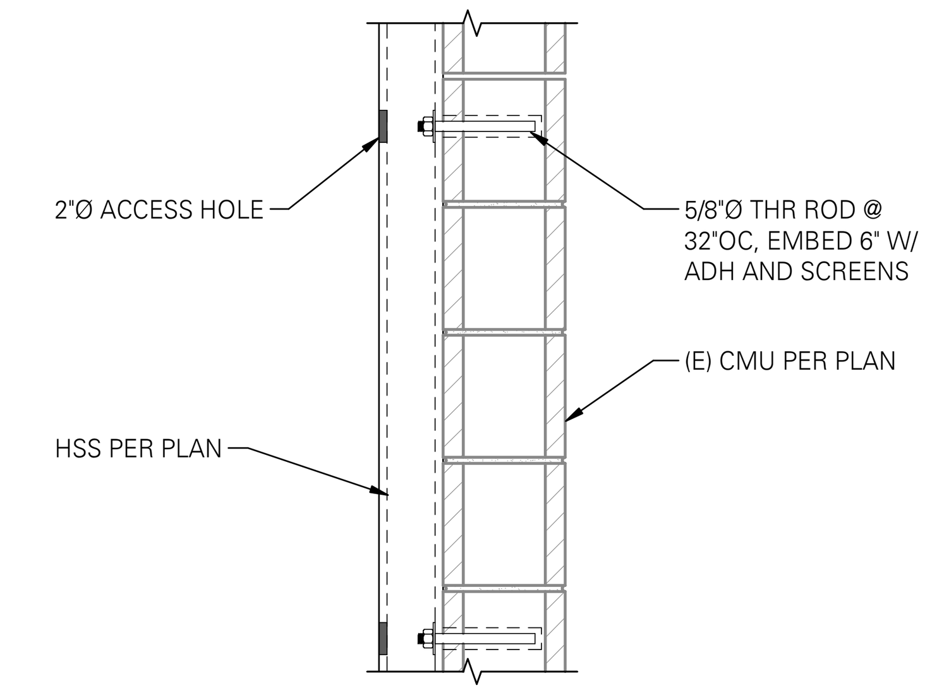
SCALE: 1" = 1'-0" (06201M)



NOTE:
DO NOT CUT REINFORCING DURING ADHESIVE ROD INSTALLATION.

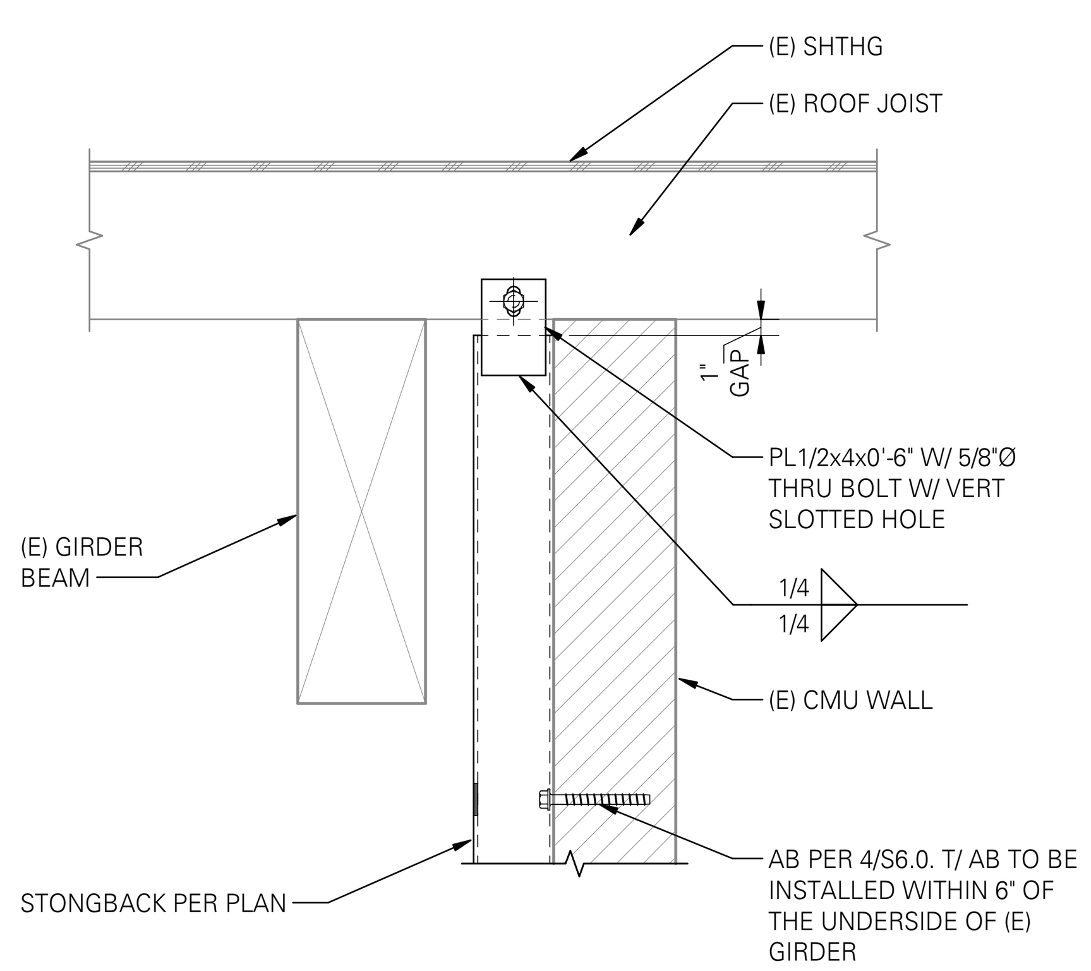
3 STRONGBACK BASE CONNECTION

SCALE: 3/4" = 1'-0" (03006M)



4 HSS TO CMU

SCALE: 1" = 1'-0"



5 TYPICAL TOP STRONGBACK CONNECTION

SCALE: 1" = 1'-0"

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PROJECT NO.: 21031-0263

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
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EXISTING

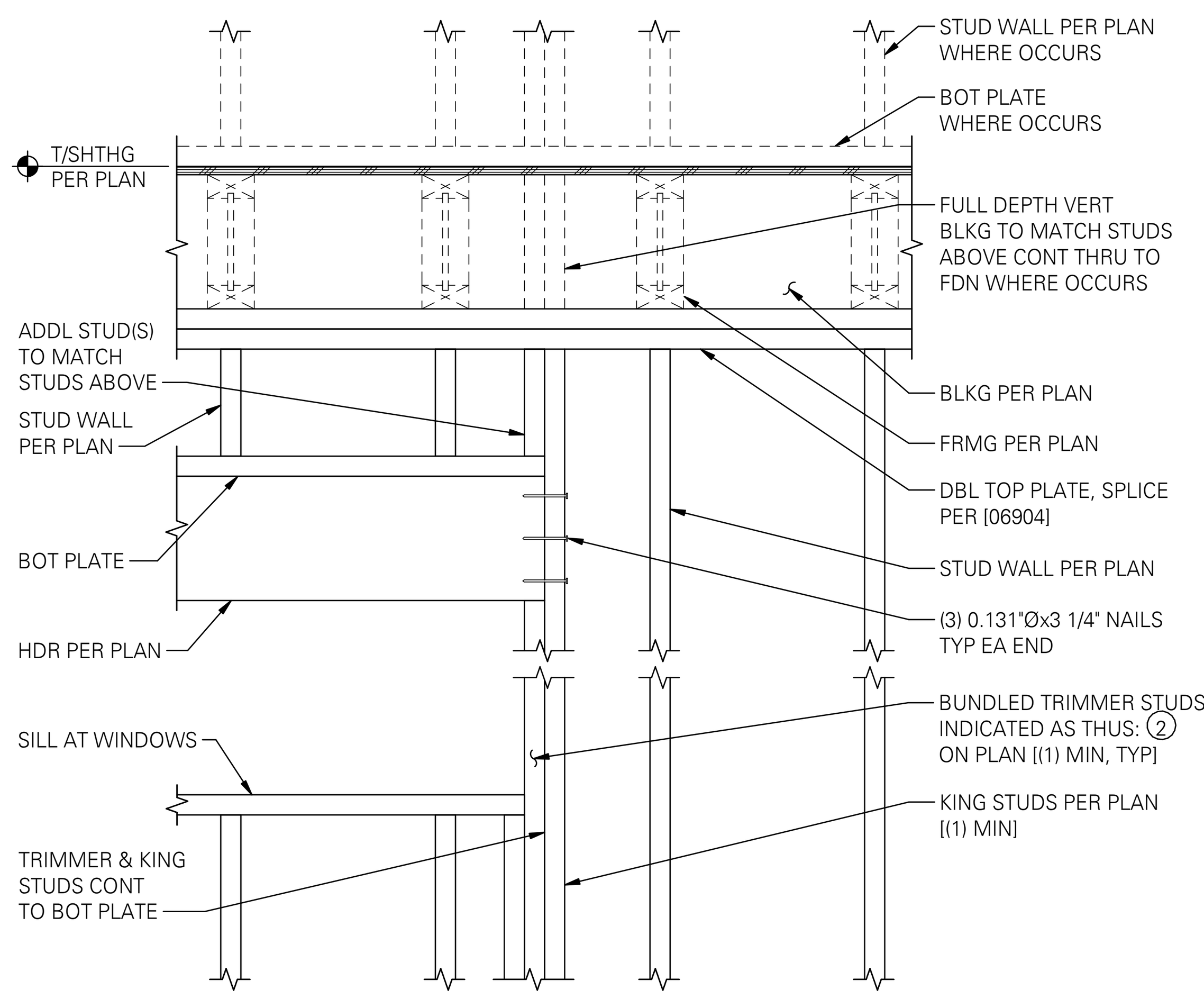
REVISIONS:

#	DATE	DESCRIPTION

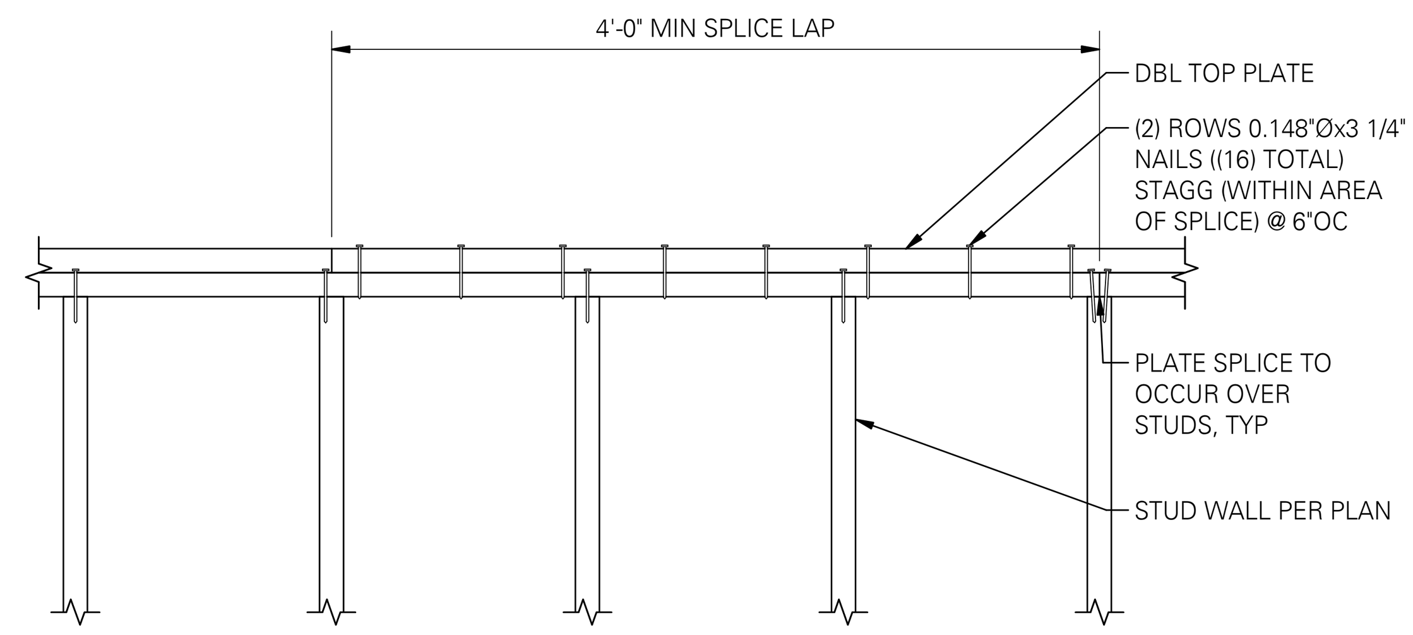
DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - STEEL FRAMING DETAILS

S6.0

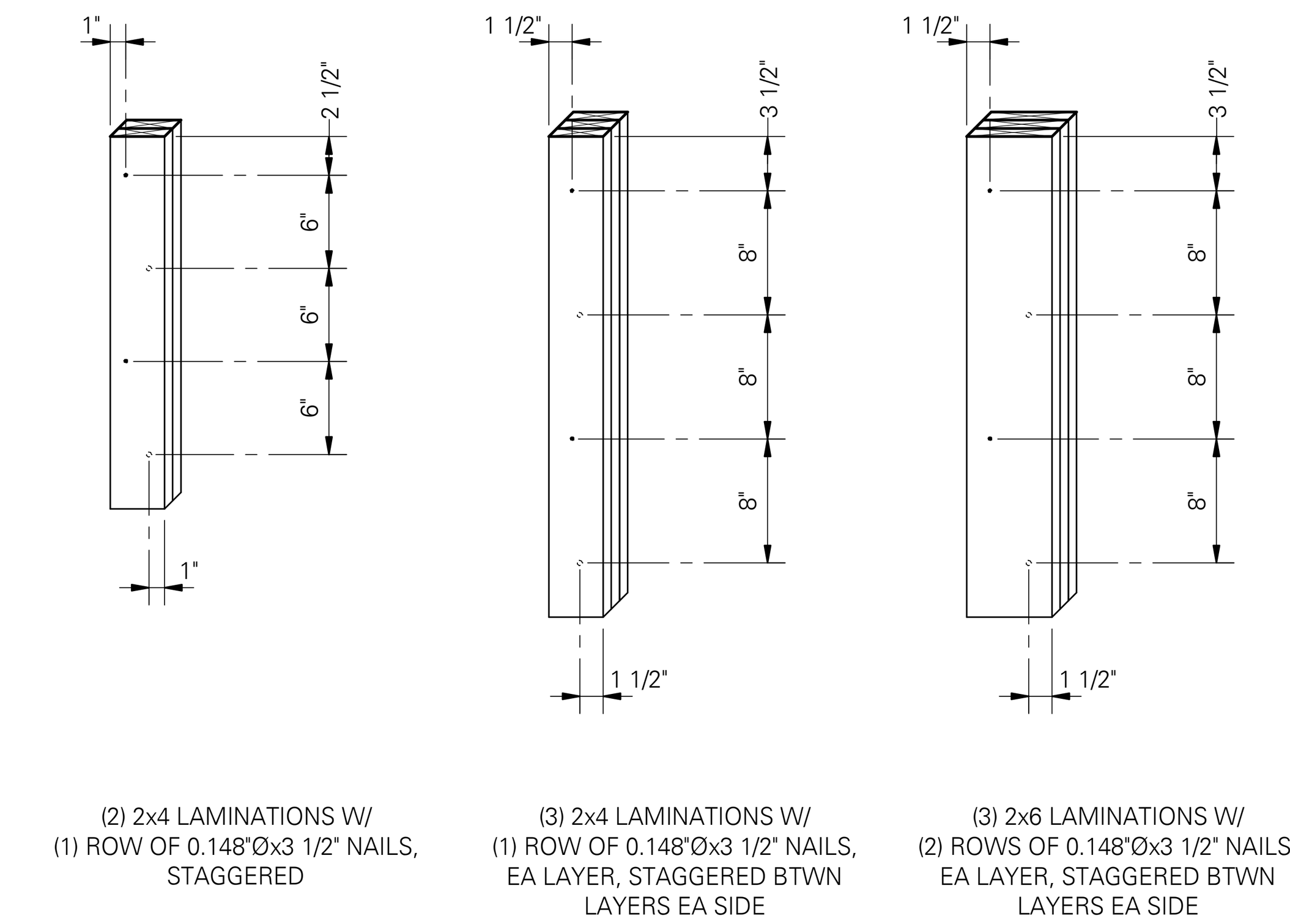


1 TYPICAL HEADER
SCALE: 1" = 1'-0" (06211)



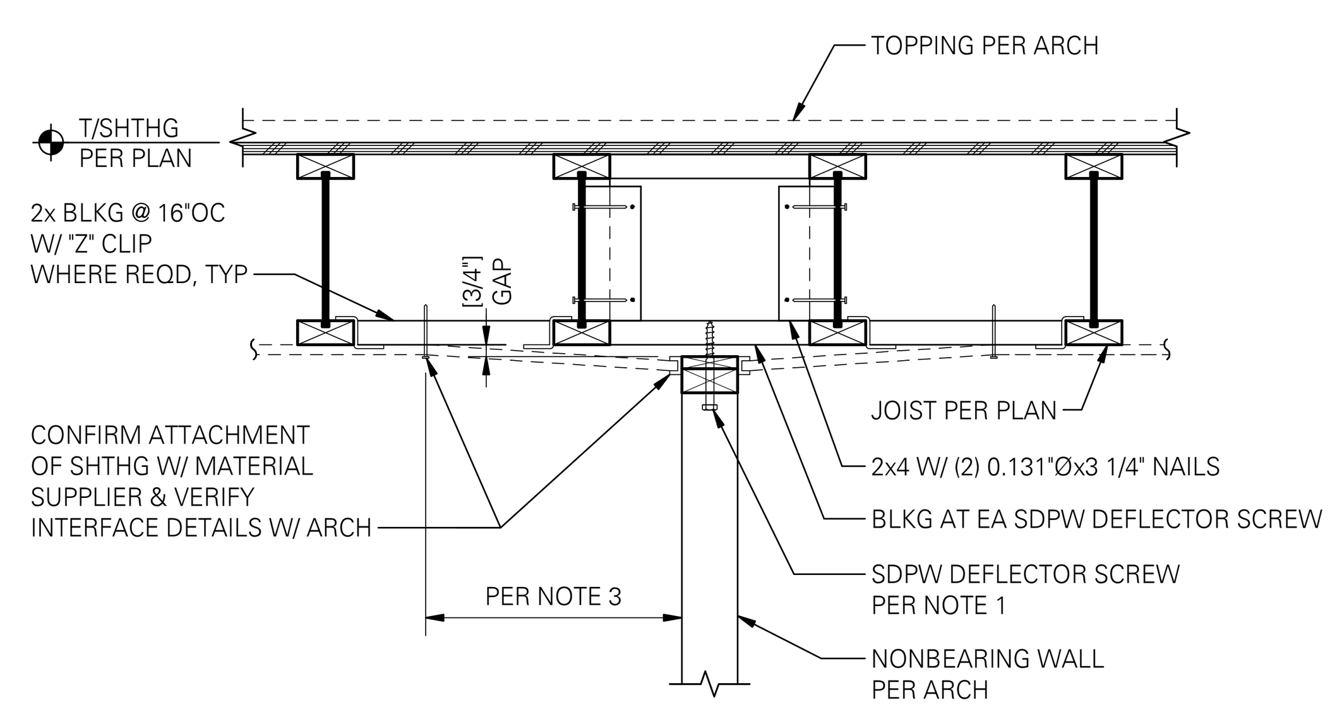
NOTE:
FLOOR/ROOF JOISTS NOT SHOWN FOR CLARITY.

2 TYPICAL PLATE SPLICE DETAIL
SCALE: 1" = 1'-0" (06904)

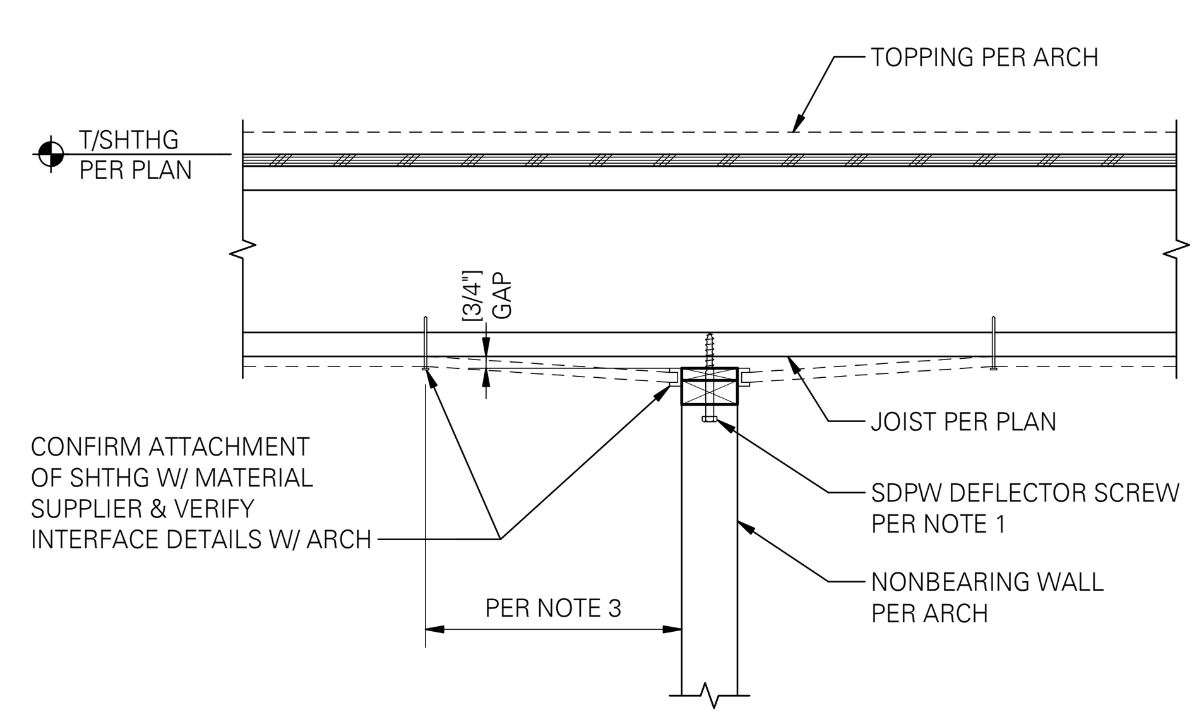


NOTE:
EACH ADDITIONAL LAMINATION OVER (3) LAMINATIONS SHALL BE NAILED TO THE ASSEMBLY W/ 0.148"Øx3 1/4" NAILS, STAGGERED EACH SIDE OF BUNDLED STUDS.

3 TYPICAL NAILING FOR BUNDLED STUDS
SCALE: 1" = 1'-0" (06914)



WALL PARALLEL TO FLOOR/ROOF FRAMING



WALL PERPENDICULAR TO FLOOR/ROOF FRAMING

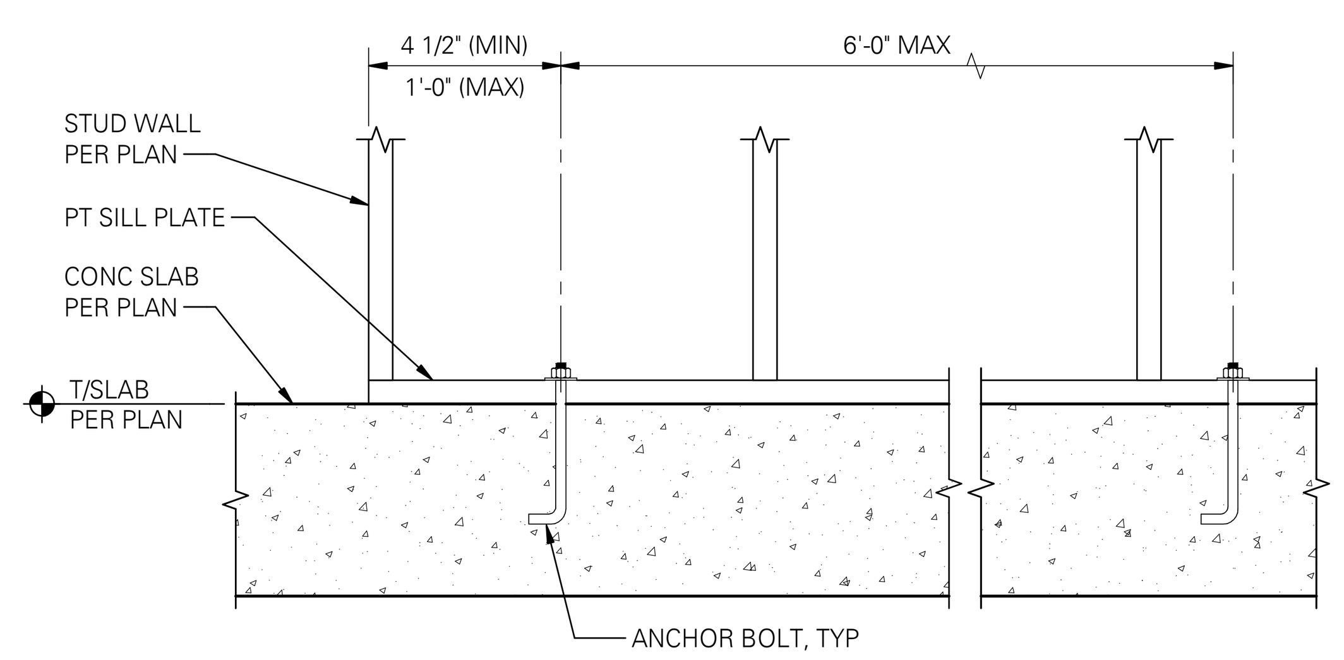
NOTES:
1. CONTRACTOR TO REFERENCE SDPW DEFLECTOR SCREW SPECIFICATIONS IN REGARDS TO PRODUCT TYPE AT 1x AND 2x TOP PLATES OR (2) 2x TOP PLATES. FOR 1x AND 2x TOP PLATES A MAXIMUM SPACING EQUALS 42"OC AND FOR (2) 2x TOP PLATES A MAXIMUM SPACING EQUALS 48"OC. SDPW DEFLECTOR SCREWS MUST BE PLACED AT MINIMUM 6" FROM EACH END OF WALL. INSTALL WITH 3/4" OFFSET BELOW BOTTOM OF PLATE AND HEAD OF SDPW DEFLECTOR SCREW.

2. DCI ENGINEERS IS ONLY RESPONSIBLE FOR THE POSITIVE CONNECTION (SDPW DEFLECTOR SCREW) FROM THE NONBEARING WALL TO THE PRIMARY STRUCTURE. THE CONTRACTOR IS TO CONFIRM THE ATTACHMENT OF THE CEILING SHEATHING TO THE NONBEARING WALL WITH THE SUB-CONTRACTOR PERFORMING THE FRAMING, THE ARCHITECT, THE MATERIAL SUPPLIER, AND THE ACOUSTICAL CONSULTANT AS THERE ARE VARIOUS CONSIDERATIONS INCLUDING MATERIAL ATTACHMENT SPECIFICATIONS, PREFERENTIAL FRAMING TECHNIQUES BY THE SUB-CONTRACTOR, FIRE RATING AND ACOUSTICAL CAULKING REQUIREMENTS, AND NONBEARING WALL FINISH INTERFACE REQUIREMENTS. ALL OF WHICH ARE OUTSIDE OF DCI ENGINEERS EXPERTISE.

3. 16" MINIMUM, 24" MAXIMUM. CONFIRM SPACING OF CONNECTION WITH MATERIAL SUPPLIER, MATERIAL SUPPLIER TO ACCOUNT FOR MOVEMENT OF STRUCTURE.

4. 1x TOP PLATE CAN BE REMOVED GIVEN THE GAP IS 1/2" MAXIMUM.

9 NON-STRUCTURAL PARTITION WALL CONNECTION
SCALE: 1" = 1'-0" (06905)



NOTES:
1. USE 5/8"Ø ANCHOR BOLTS WITH 7" MINIMUM EMBEDMENT INTO CONCRETE SLAB.

2. EACH SILL PLATE PIECE SHALL HAVE (2) BOLTS MINIMUM. HOLD-DOWN ANCHORS ARE NOT TO BE CONSIDERED AN ANCHOR BOLT.

3. LOCATE BOLTS WITHIN 1'-0" OF SILL PLATE PIECE ENDS AND AT 6'-0"OC MAXIMUM.

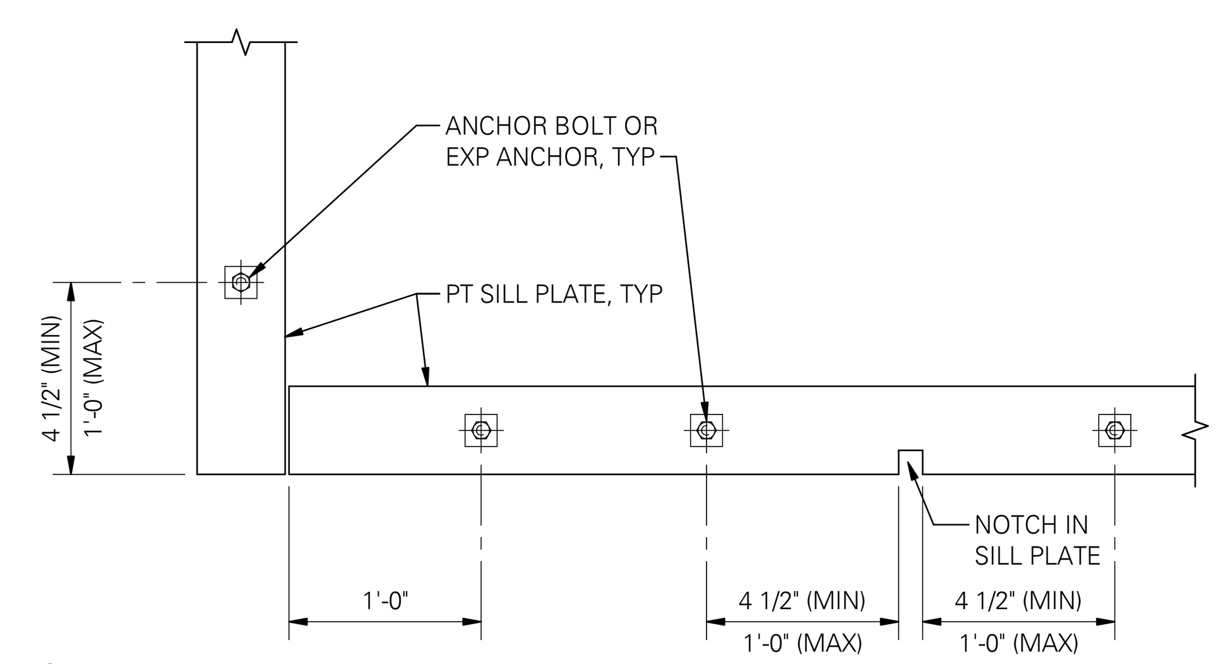
4. USE PLATE WASHER PER SHEAR WALL SCHEDULE AT EACH BOLT. STANDARD CUT WASHERS ARE ACCEPTABLE AT NON-SHEAR WALLS.

5. DO NOT DRILL OVERSIZE HOLES THRU SILL PLATE. USE 11/16"Ø DRILL BIT.

6. SILL PLATE THICKNESS AND FASTENING AT SHEAR WALLS PER SHEAR WALL SCHEDULE.

7. CONTACT THE ENGINEER-OF-RECORD FOR POST INSTALLED ANCHOR OPTIONS.

6 TYPICAL SILL PLATE ANCHORAGE TO CONCRETE
SCALE: 1" = 1'-0" (06910)



NOTES:
1. USE 5/8"Ø ANCHOR BOLT PER [06910].

2. EACH SILL PLATE PIECE SHALL HAVE (2) BOLTS MINIMUM. HOLD-DOWN ANCHORS ARE NOT TO BE CONSIDERED AN ANCHOR BOLT.

3. LOCATE BOLTS WITHIN 1'-0" OF SILL PLATE PIECE ENDS AND AT 6'-0"OC MAXIMUM.

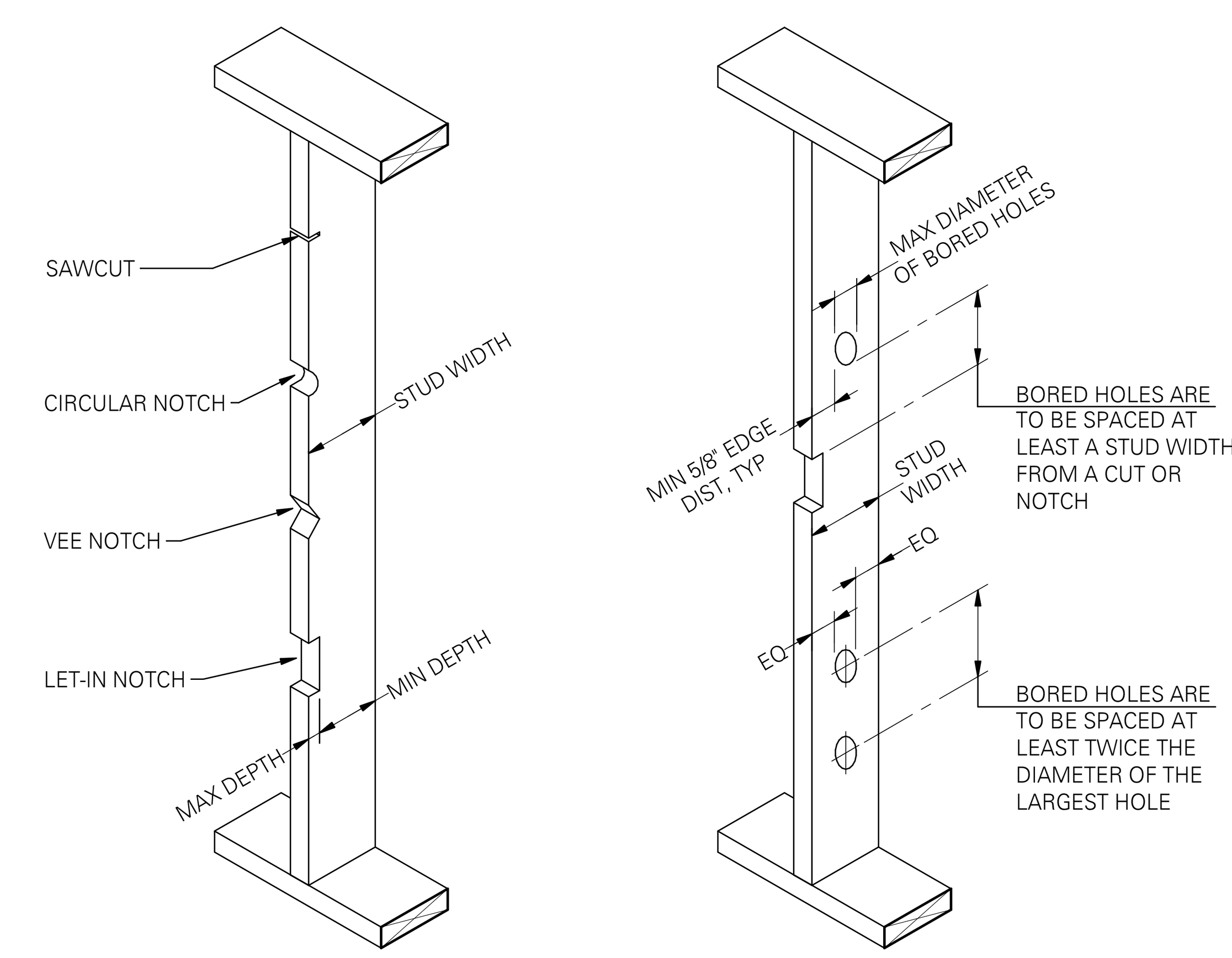
4. USE PLATE WASHER PER SHEAR WALL SCHEDULE AT EACH BOLT. STANDARD CUT WASHERS ARE ACCEPTABLE AT NON-SHEAR WALLS.

5. DO NOT DRILL OVERSIZE HOLES THRU SILL PLATE. USE 11/16"Ø DRILL BIT.

6. SILL PLATE THICKNESS AND FASTENING AT SHEAR WALLS PER SHEAR WALL SCHEDULE.

7. CONTACT THE ENGINEER-OF-RECORD FOR POST INSTALLED ANCHOR OPTIONS.

10 PLAN - TYPICAL SILL PLATE ANCHORAGE TO CONCRETE
SCALE: 1" = 1'-0" (06911)



BEARING WALL STUDS		
STUD SIZE	MAX DEPTH OF EDGE CUT OR NOTCH	MIN DEPTH REMAINING AFTER CUT OR NOTCH
2x4	7/8"	2 5/8"
2x6	1 3/8"	4 1/8"

NON-BEARING WALL STUDS		
STUD SIZE	MAX DEPTH OF EDGE CUT OR NOTCH	MIN DEPTH REMAINING AFTER CUT OR NOTCH
2x4	1 3/8"	2 1/8"
2x6	2 3/16"	3 3/8"

CUTTING AND NOTCHING WOOD STUDS

NOTE:
DO NOT NOTCH MORE THAN THREE ADJACENT STUDS WITHOUT REVIEW BY ENGINEER.

BEARING WALL STUDS		
STUD SIZE	MAX DIAMETER OF BORED HOLE	MIN DEPTH REMAINING AFTER BORED HOLE
2x4	1 3/8"	5/8" EA SIDE OF HOLE
2x6	2 3/16"	5/8" EA SIDE OF HOLE

NOTE:
STUDS MAY NOT BE BORED IN EXCESS OF 40% OF THE STUD, IF STUDS ARE DOUBLED, BORINGS MAY BE INCREASED TO 60% OF STUD WIDTH PROVIDED NOT MORE THAN (2) SUCCESSIVE STUDS ARE BORED. BORINGS SHALL NOT BE MADE AT THE SAME SECTION WHERE CUT OR NOTCH HAS BEEN MADE.

NON-BEARING WALL STUDS		
STUD SIZE	MAX DIAMETER OF BORED HOLE	MIN DEPTH REMAINING AFTER BORED HOLE
2x4	2 1/16"	5/8" EA SIDE OF HOLE
2x6	3 1/4"	5/8" EA SIDE OF HOLE

NOTE:
STUDS MAY NOT BE BORED IN EXCESS OF 60% OF THE STUD. BORINGS SHALL NOT BE MADE AT THE SAME SECTION WHERE CUT OR NOTCH HAS BEEN MADE.

BORED HOLES IN WOOD STUDS

NOTE:
BORED HOLE NOT PERMITTED IN MORE THAN THREE ADJACENT STUDS WITHOUT REVIEW BY ENGINEER.

11 TYPICAL HOLES & NOTCHES IN WOOD STUDS
SCALE: 1" = 1'-0" (06908)

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STRUCTURAL REGISTERED PROFESSIONAL ENGINEER 74858PE OREGON MARCH 23, 2011 SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 21031-0263
NORTH BAY FIRE SEISMIC GRANT
NORTH BAY FIRE DISTRICT
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NORTH BEND, OR 97220

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REVISIONS:

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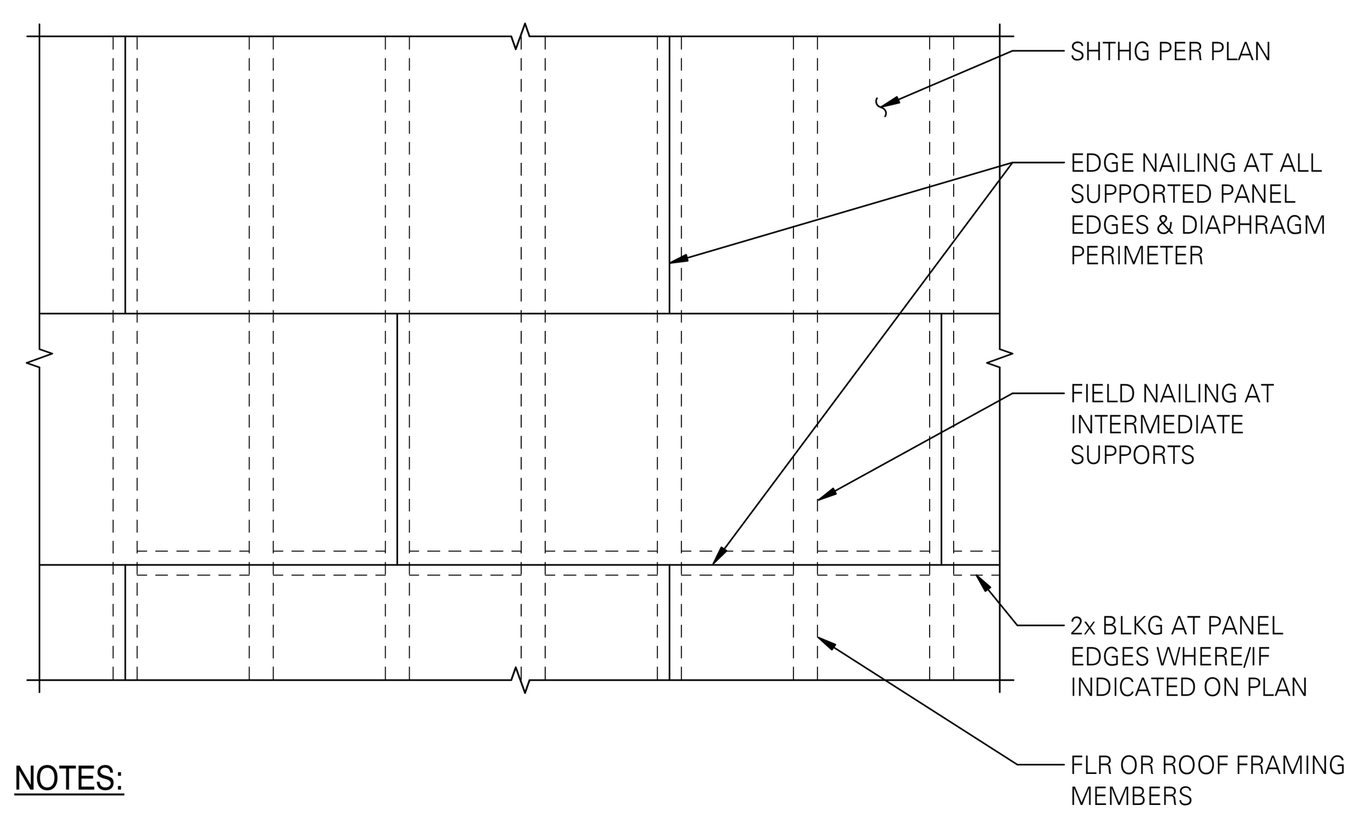
DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - WOOD FRAMING DETAILS

S7.0

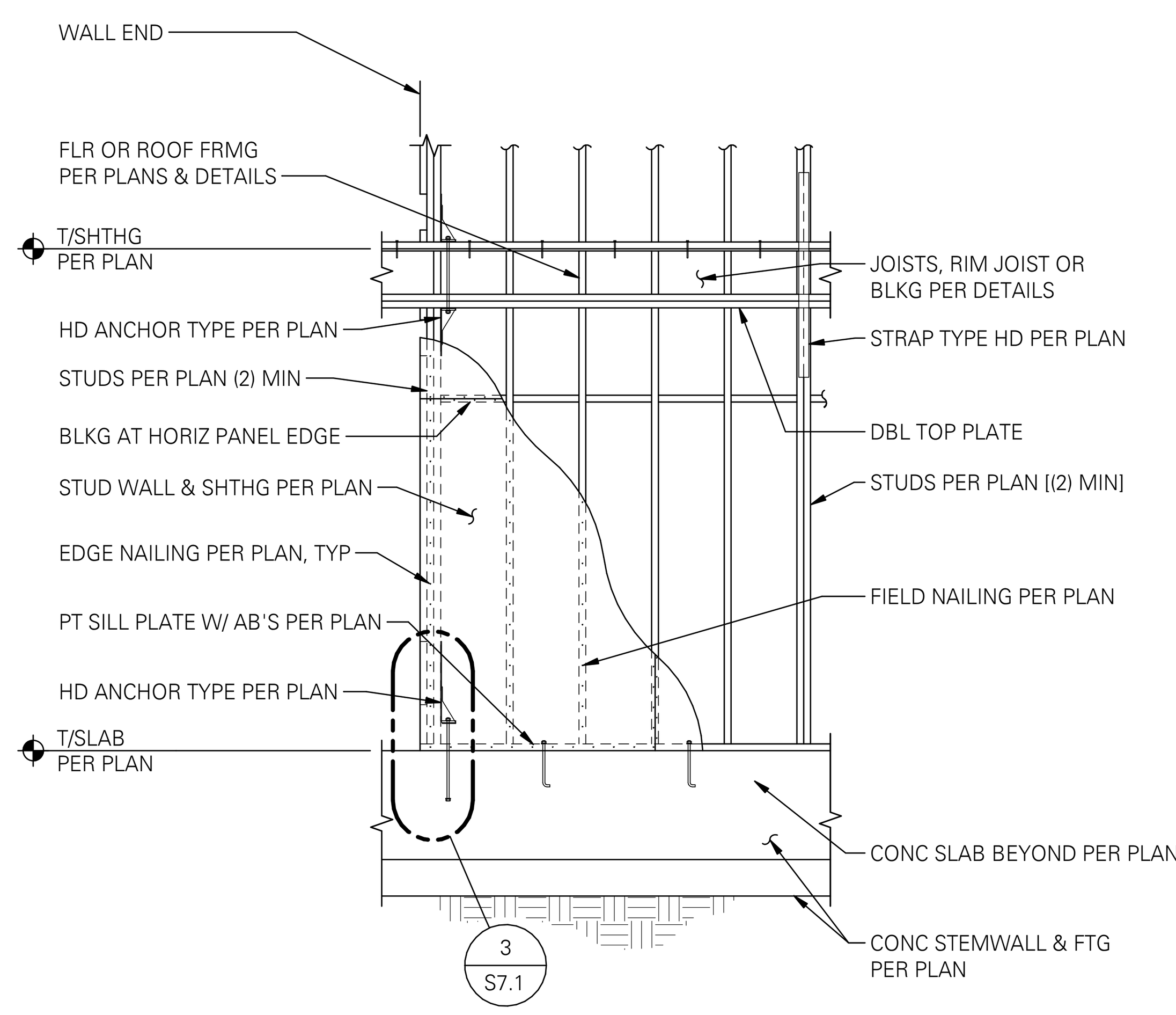


NOTES:

1. MINIMUM EDGE DISTANCE FOR NAILS SHALL BE 3/8".
2. MINIMUM SHEATHING SHEET SIZE SHALL BE 24"x48".
3. NAILS SHALL NOT BE OVERDRIVEN.
4. NAILS SHALL BE COMMON WIRE TYPE OR APPROVED EQUAL.
5. ADDITIONAL INFORMATION PER STRUCTURAL GENERAL NOTES.

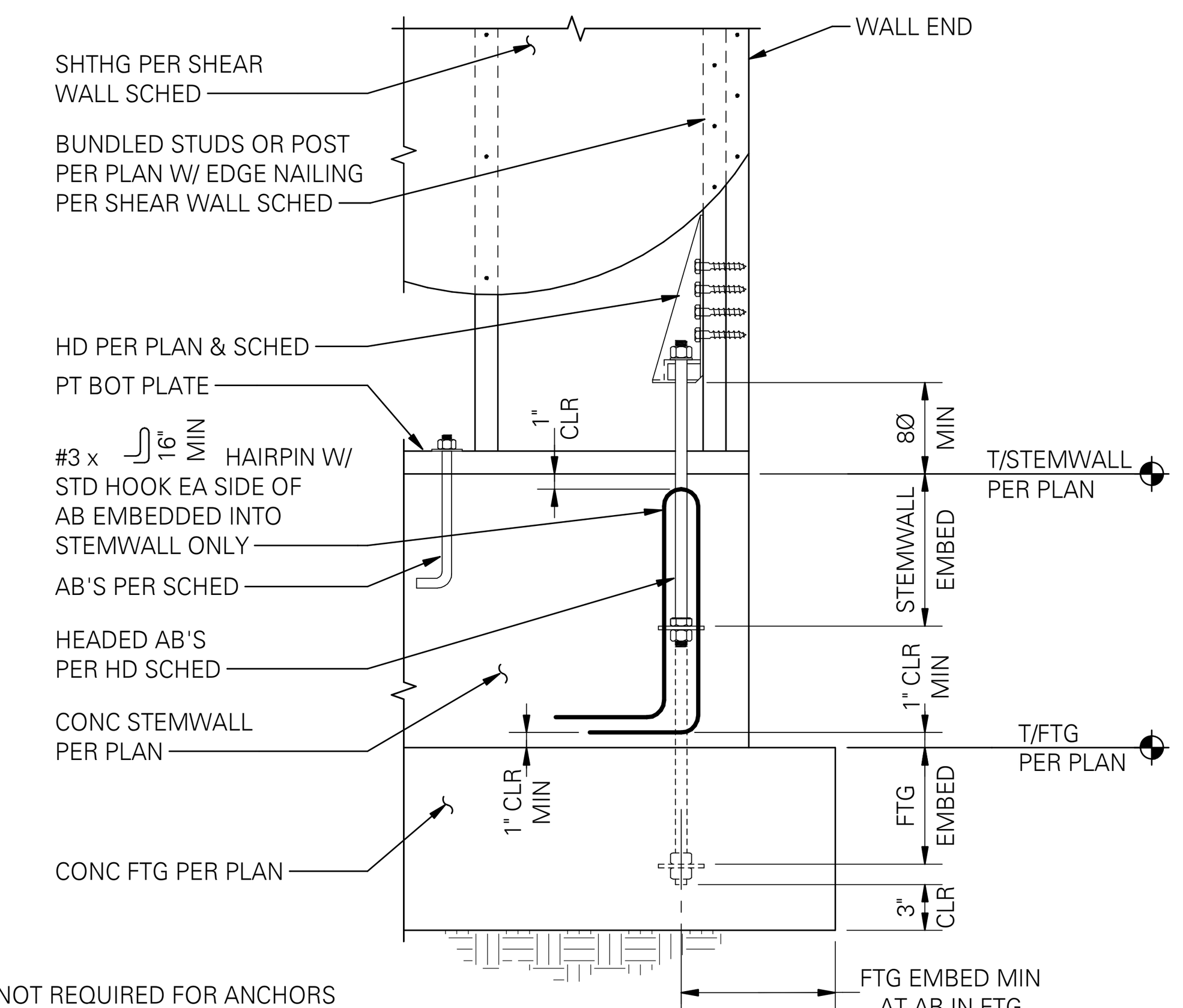
1 PLAN - TYPICAL FLOOR AND ROOF SHEATHING ATTACHMENT

SCALE: 1" = 1'-0" (06230)



2 TYPICAL SHEAR WALL ELEVATION

SCALE: 1" = 1'-0" (06090)



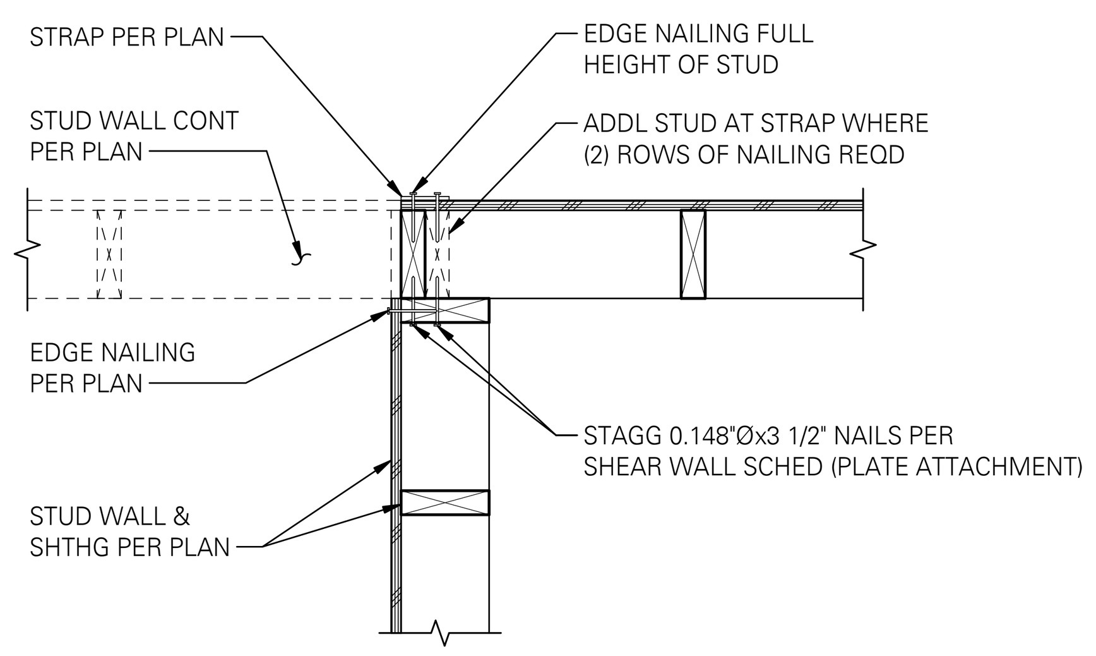
AT HDU HOLD-DOWNS

NOTES:

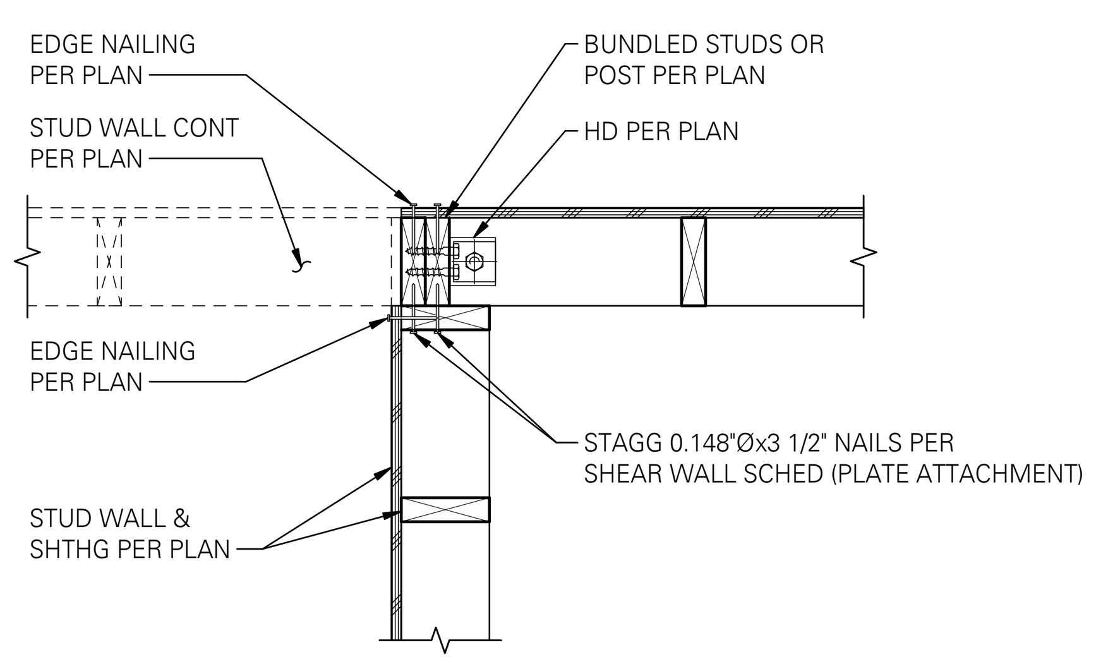
1. HAIRPINS NOT REQUIRED FOR ANCHORS EMBEDDED INTO FOOTING PER 1/S7.2.
2. MINIMUM FOOTING SIZE FOR ANCHORS EMBEDDED INTO FOOTING IS 2x EMBED SQUARE WITH DEPTH AS INDICATED.

3 TYPICAL HOLD-DOWN AT FOUNDATION - CONCRETE STEMWALL

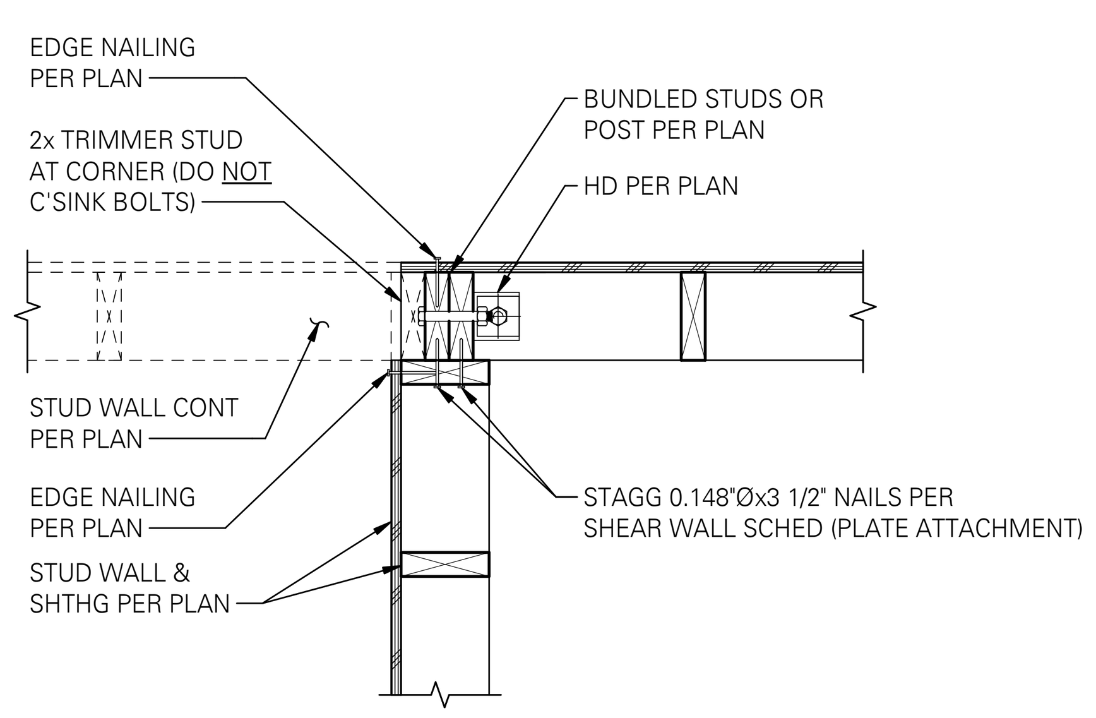
SCALE: 1" = 1'-0" (06091)



WITH OR WITHOUT STRAP TIE



HDU HOLD-DOWNS



HD HOLD-DOWNS

9 PLAN - INTERSECTING SHEAR WALLS

SCALE: 1" = 1'-0" (06110)

01430

SHEAR WALL SCHEDULE W6 FOR 0.131"Øx 2 1/2" NAILS IN DOUG-FIR LARCH (2018 IBC) [15]

SOME SHEAR WALL TYPES NOTED MAY NOT BE USED ON THIS PROJECT.

WALL TYPE	WALL SHEATHING APA-RATED [1, 2, 11, 12]	NAIL SIZE & SPACING AT ALL PANEL EDGES [4, 5]	BLOCKING & STUD SIZE AT ADJOINING PANEL EDGES [3, 6, 13]	RIM JOIST OR BLOCKING CONN TO TOP PLATE BELOW [7, 8]	2x PLATE ATTACHMENT	SILL PLATE ATTACHMENT		SHEAR CAPACITY LBS/FT
					NAILING TO WOOD RIM JOIST OR BLOCKING BELOW	ANCHOR BOLT TO CONCRETE BELOW [9]	SILL PLATE AT FOUNDATION [10] [18]	
W6	15/32"	0.131"Øx2 1/2" @ 6"OC	2x	CLIP @ 16"OC	0.148"Øx3 1/4" @ 8"OC	5/8"Ø @ 48"OC	2x	260
W4	15/32"	0.131"Øx2 1/2" @ 4"OC	2x	CLIP @ 16"OC	0.148"Øx3 1/4" @ 6"OC	5/8"Ø @ 48"OC	2x	350
W3	15/32"	0.131"Øx2 1/2" @ 3"OC STAGGERED	3x	CLIP @ 12"OC	0.148"Øx3 1/4" @ 4"OC	5/8"Ø @ 32"OC	2x	490
W2	15/32"	0.131"Øx2 1/2" @ 2"OC STAGGERED	3x	CLIP @ 16"OC EACH SIDE	0.148"Øx3 1/4" @ 6"OC (2) ROWS [9]	5/8"Ø @ 24"OC	2x	640
2W3	15/32" BOTH SIDES	0.131"Øx2 1/2" @ 3"OC STAGGERED	3x	CLIP @ 12"OC EACH SIDE	0.148"Øx3 1/4" @ 4"OC (2) ROWS [9]	5/8"Ø @ 16"OC	3x [17]	980

NOTES:

[1] INSTALL PANELS EITHER HORIZONTALLY OR VERTICALLY.

[2] WHERE SHEATHING IS APPLIED ON BOTH SIDES OF WALL, PANEL EDGE JOINTS ON 2x FRAMING SHALL BE STAGGERED SO THAT JOINTS ON THE OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUD.

[3] BLOCKING IS REQUIRED AT ALL PANEL EDGES.

[4] PROVIDE SHEAR WALL SHEATHING AND NAILING FOR THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. ENDS OF FULL HEIGHT WALLS ARE DESIGNATED BY WINDOWS OR DOORWAYS OR AS DESIGNATED ON PLANS. HOLD-DOWN REQUIREMENTS PER PLANS. (ALTERNATE NOTE: WALLS SHOWN WITH HORIZONTAL STRAPS BELOW AND/OR ABOVE OPENINGS REQUIRE SHEATHING, SHEAR WALL NAILING, ETC ABOVE AND BELOW ALL OPENINGS).

[5] SHEATHING EDGE NAILING IS REQUIRED AT ALL HOLD-DOWN POSTS. EDGE NAILING MAY ALSO BE REQUIRED TO EACH STUD USED IN BUILT-UP HOLD-DOWN POSTS. ADDITIONAL INFORMATION PER HOLD-DOWN DETAILS.

[6] INTERMEDIATE FRAMING TO BE 2x MINIMUM MEMBERS. ATTACH SHEATHING TO INTERMEDIATE FRAMING WITH 0.131"Øx2 1/2" NAILS AT 12"OC WHERE STUDS ARE SPACED AT 16"OC AND 0.131"Øx2 1/2" NAILS AT 6"OC WHERE STUDS ARE SPACED AT 24"OC.

[7] BASED ON 0.131"Øx1 1/2" NAILS USED TO ATTACH FRAMING CLIPS DIRECTLY TO FRAMING. USE 0.131"Øx2 1/2" NAILS WHERE INSTALLED OVER SHEATHING.

[8] FRAMING CLIPS: A35 OR LTP5 OR APPROVED EQUIVALENT.

[9] ANCHOR BOLTS SHALL BE PROVIDED WITH HOT-DIPPED GALVANIZED STEEL PLATE WASHERS 0.229"x3"x3" MINIMUM. THE HOLE IN THE PLATE WASHER MAY BE DIAGONALLY SLOTTED 13/16"x1 3/4" PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND NUT. PLATE WASHER TO EXTEND TO WITHIN 1/2" OF THE EDGE OF THE SILL PLATE ON THE SIDE(S) WITH SHEATHING. AT 2x6 WALLS WITH SHEATHING ON BOTH SIDES USE PLATE WASHER 0.229"x4 1/2"x4 1/2" MINIMUM. EMBED ANCHOR BOLTS 7" MINIMUM INTO THE CONCRETE.

[10] PRESSURE TREATED MATERIAL CAN CAUSE EXCESSIVE CORROSION IN THE FASTENERS. PROVIDE HOT-DIPPED GALVANIZED (ELECTRO-PLATING IS NOT ACCEPTABLE) NAILS AND CONNECTOR PLATES (FRAMING ANGLES, ETC) FOR ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED FRAMING MEMBERS. ADDITIONAL INFORMATION PER STRUCTURAL GENERAL NOTES.

[11] 7/16" APA-RATED SHEATHING (OSB) MAY BE USED IN PLACE OF 15/32" SHEATHING PROVIDED THAT ALL STUDS ARE SPACED AT 16"OC MAXIMUM.

[12] WHERE WOOD SHEATHING (W) IS APPLIED OVER GYPSUM SHEATHING (G), CONTACT THE ENGINEER OF RECORD FOR ALTERNATE NAILING REQUIREMENTS.

[13] AT ADJOINING PANEL EDGES, (2) 2x STUDS NAILED TOGETHER MAY BE USED IN PLACE OF SINGLE 3x STUD. DOUBLE 2x STUDS SHALL BE CONNECTED TOGETHER BY NAILING THE STUDS TOGETHER WITH 3" LONG NAILS OF THE SAME SPACING AND DIAMETER AS THE PLATE NAILING, PER SECTION.

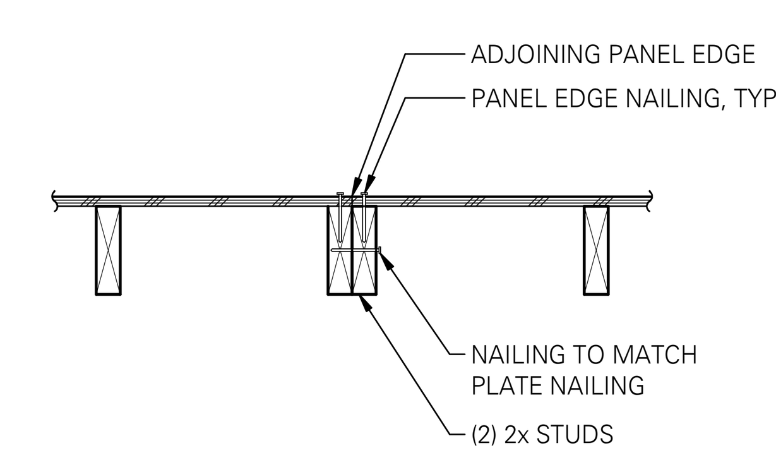
[14] CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR ADHESIVE OR EXPANSION BOLT ALTERNATIVES TO CAST-IN-PLACE ANCHOR BOLTS. SPECIAL INSPECTION MAY BE REQUIRED.

[15] WX WHERE "W" INDICATES WOOD SHEATHING AND "X" INDICATES EDGE NAIL SPACING.

[16] EDGE NAILS SHALL BE LOCATED 3/8" FROM PANEL EDGES.

[17] NAIL STUDS TO 3x SILL PLATES WITH EITHER (2) 0.148"Øx4" END NAILS OR (4) 0,131"Øx2 1/2" TOENAILS.

[18] INSTALL 5/8"Ø TITEN HD W/ 5" MIN EMBED AT EXISTING WOOD FRAMED WALLS, WITH PLATE WASHER PER [9].



PLAN

11 SHEAR WALL SCHEDULE - DOUG-FIR LARCH

SCALE: 1" = 1'-0" (01430M)

FOR PERMIT

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STRUCTURAL REGISTERED PROFESSIONAL ENGINEER 74858PE OREGON MARCH 23, 2011 SHIRLEY CHALUPA

EXPIRES: 12-31-25

PROJECT NO.: 21031-0263

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97220

EXISTING

REVISIONS:

#	DATE	DESCRIPTION

DATE: APRIL-11

SHEET TITLE: STRUCTURAL - WOOD FRAMING DETAILS

S7.1

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01420

HOLD-DOWN/STRAP SCHEDULE - DOUG-FIR STUDS

[1, 2, 7, 11] ← INDICATES FOOTNOTES

	TYPE	NUMBER OF STUDS/POST [3, 12]	NAILS, SCREWS OR BOLTS	ANCHOR [4] DIAMETER [10]	CONCRETE EMBEDMENT/CAPACITY: STEMWALL [5] EMBED CIP [6, 13]	STEMWALL [5] CAPACITY	FOOTING EMBED CIP [6]	FOOTING CAPACITY	NOTES
WOOD TO CONCRETE	HDU2	(2) 2x	(6) SDS1/4x2 1/2	5/8"Ø	10"	3.1k	8"	3.1k	----

NOTES:

[1] SOME HOLD-DOWN TYPES MAY NOT BE USED ON THIS PROJECT.

[2] TYPICAL HOLD-DOWN DETAILS PER 3/S7.1, 6/S7.1, 10/S7.1. ANCHOR REINFORCEMENT REQUIRED AT STEMWALLS.

[3] PROVIDE PANEL EDGE NAILING PER SHEAR WALL SCHEDULE AT HOLD-DOWN STUDS/POSTS.

[4] BASED ON MINIMUM f'c = 3000 PSI CONCRETE.

[5] STEMWALLS SHALL BE 8" WIDE x 18" TALL MINIMUM.

[6] CAST-IN-PLACE (CIP) TYPE THREADED RODS AT HOLD-DOWNS SHALL HAVE TWO HEX HEAD NUTS WITH OVERSIZED WASHERS.

[7] INCLUDES 1.6 LOAD DURATION INCREASE FOR WOOD.

[8] BASED ON 11 7/8" DEEP FLOOR JOIST.

[9] TOTAL NAILS SPECIFIED, USE HALF THE NAILS AT THE STUDS ABOVE AND BELOW LEVEL BEING CONNECTED.

[10] AT PRESSURE TREATED SILLS, USE HOT DIPPED GALVANIZED BOLTS.

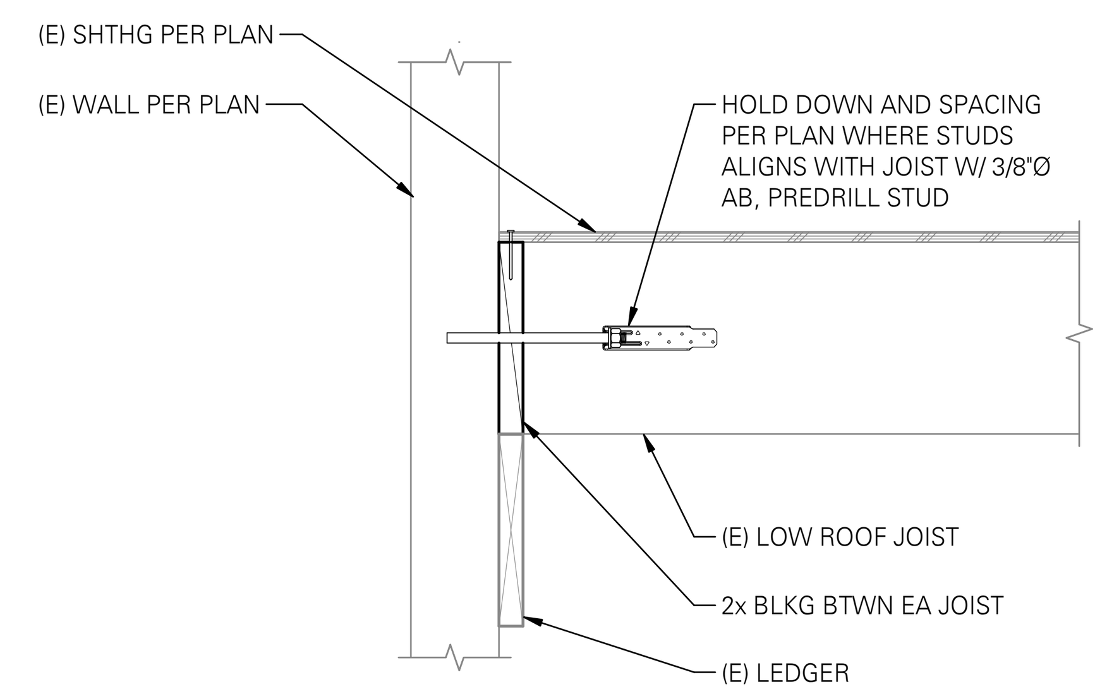
[11] POST INSTALLED HOLD-DOWN OPTIONS MAY BE AVAILABLE AT SOME CONDITIONS. CONTACT ENGINEER OF RECORD PRIOR TO CONSTRUCTION.

[12] NAIL LAMINATE MULTIPLE 2x STUDS WITH PLATE NAILING PER SHEAR WALL SCHEDULE.

[13] STUD WALLS SHALL BE 2x6, CENTER HOLD-DOWN IN STUD WALL.

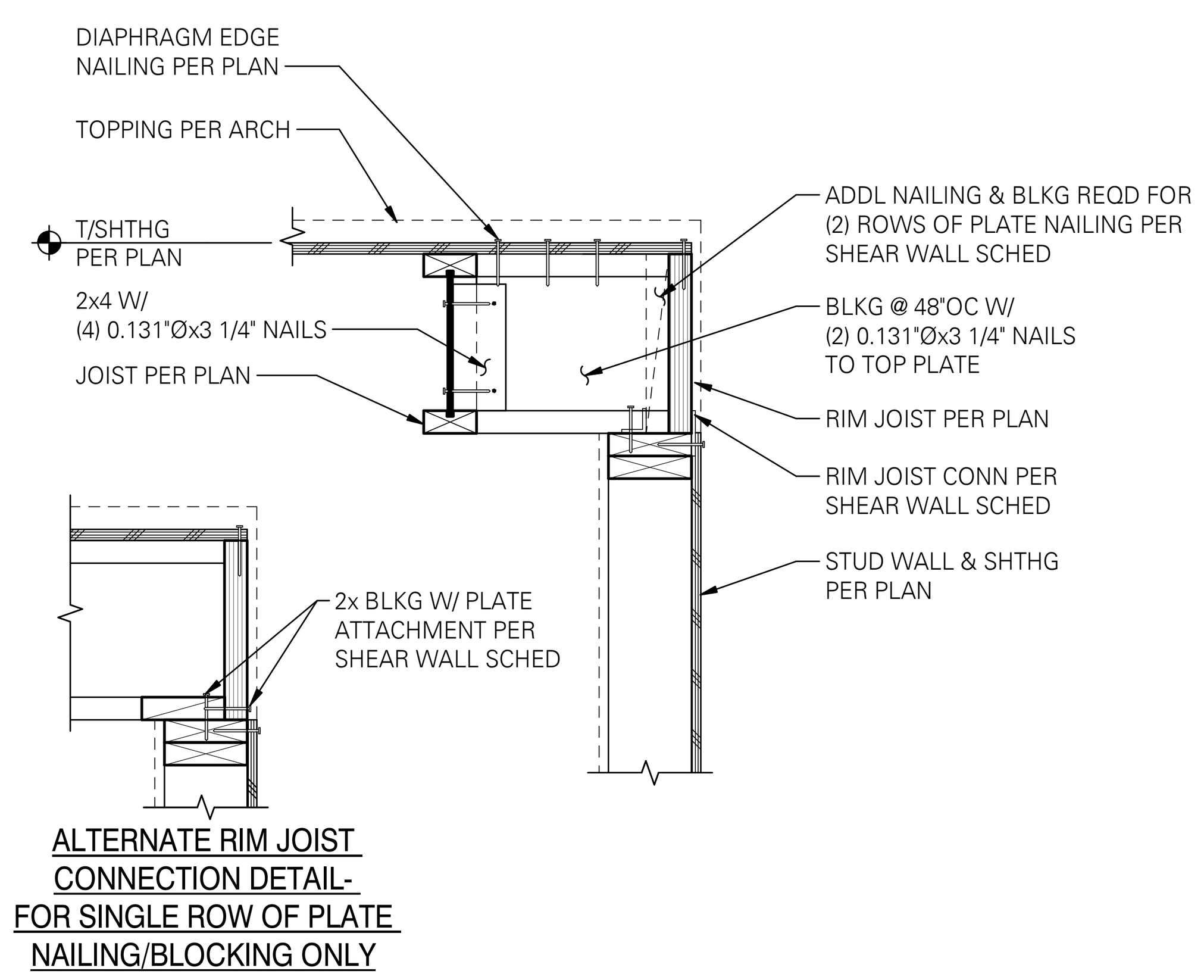
1 HOLD-DOWN/STRAP SCHEDULE - DOUG-FIR STUDS

SCALE: 1" = 1'-0" (01420M)



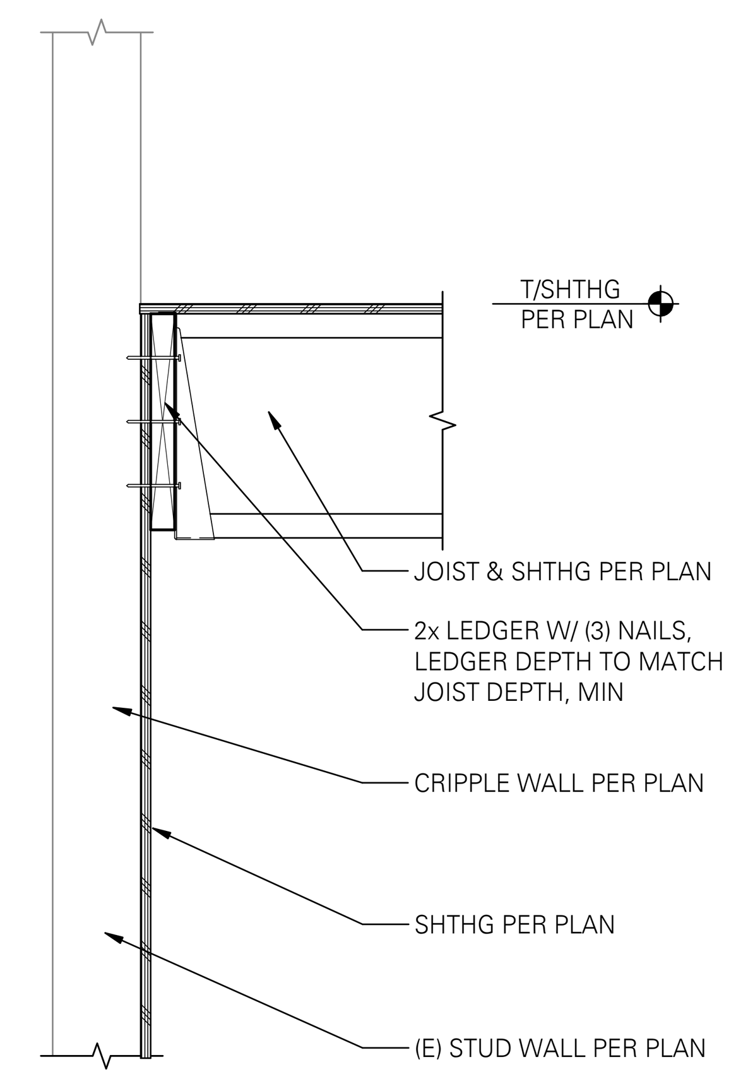
3 HOLD DOWN AT WOOD WALL AND (E) LOW ROOF PARALLEL TO JOIST

SCALE: 1" = 1'-0"



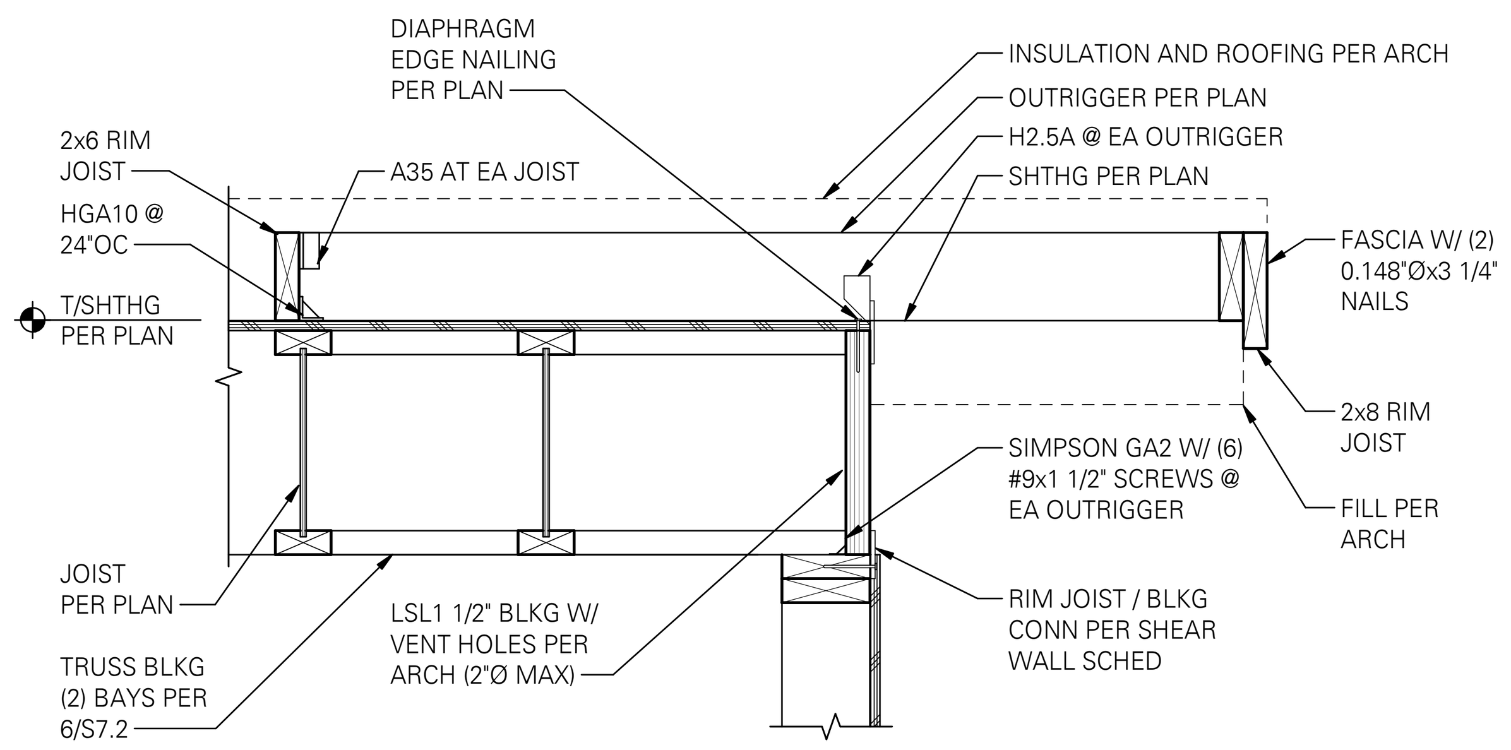
6 EXTERIOR WALL PARALLEL TO FLOOR JOISTS

SCALE: 1" = 1'-0" (06001)



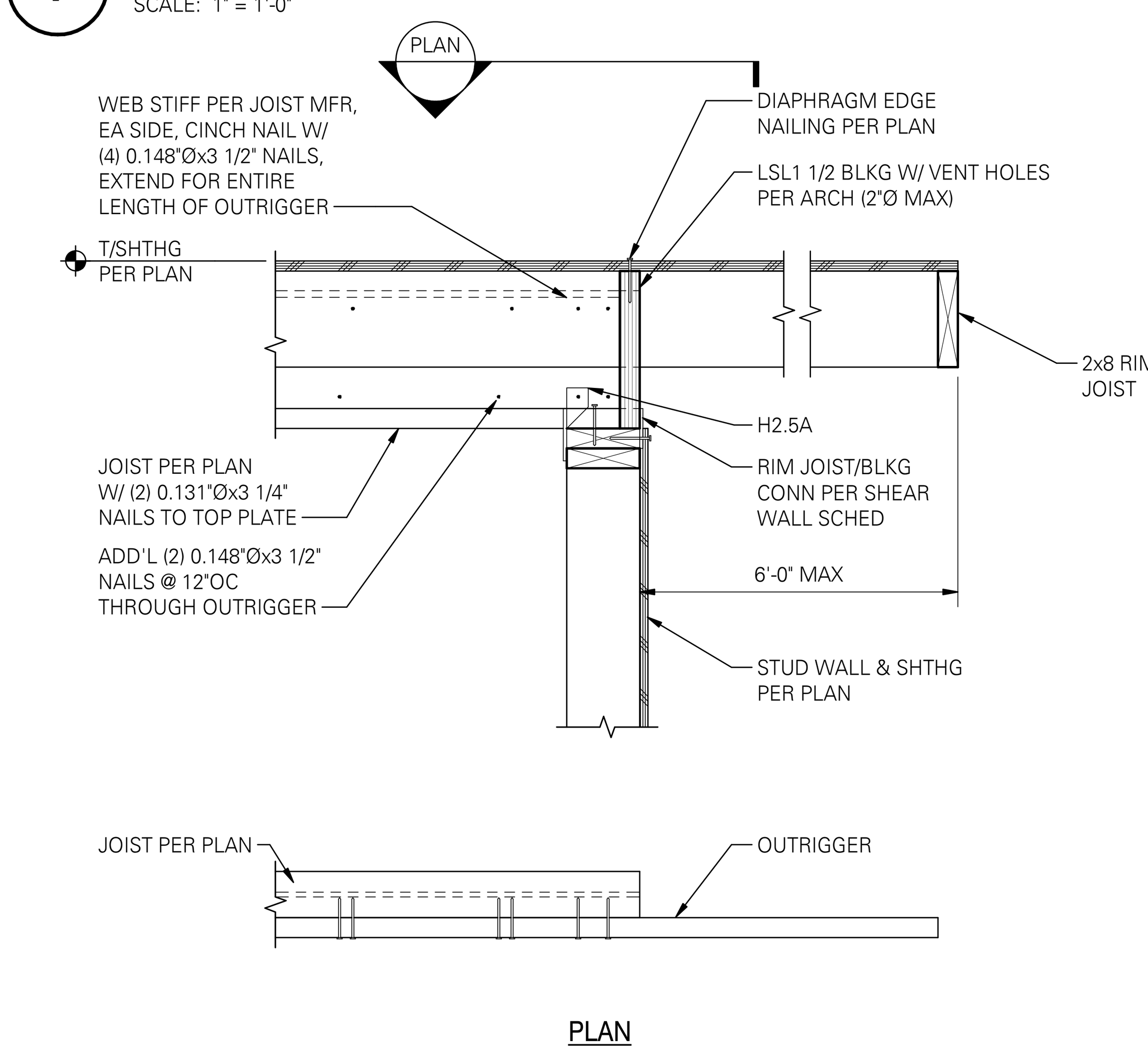
7 ROOF ADDITION

SCALE: 1" = 1'-0"



9 FASCIA TO RAFTER CONNECTION

SCALE: 1" = 1'-0"



10 EXTERIOR WALL PERPENDICULAR TO ROOF JOIST WITH OVERHANG - NEW ADDITION

SCALE: 1" = 1'-0" (06061B)

FOR PERMIT
The Contractor shall not use these drawings for construction until Contractor receives written approval for use in construction by the authority having jurisdiction and DCI Engineers.

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CIVIL / STRUCTURAL

STRUCTURAL REGISTERED PROFESSIONAL ENGINEER 74858PE OREGON MARCH 23, 2011 SHIRLEY CHALUPA
EXPIRES: 12-31-25

PROJECT NO.: 21031-0263

NORTH BAY FIRE SEISMIC GRANT

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97220

EXISTING

REVISIONS:

#	DATE	DESCRIPTION

DATE: APRIL-11

SHEET TITLE:
STRUCTURAL - WOOD FRAMING DETAILS

S7.2

MECHANICAL LEGEND

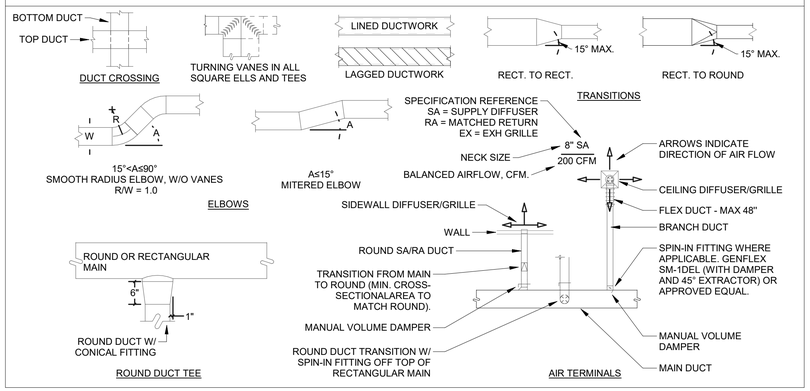
- SUPPLY AIR DIFFUSER
- RETURN AIR GRILLE
- EXHAUST AIR GRILLE
- PERFORATED RETURN AIR PANEL
- OR — DIRECTIONAL AIR FLOW
- MANUAL VOLUME DAMPER
- SUPPLY OR OUTSIDE AIR DUCT UP & DOWN
- RETURN AIR DUCT UP & DOWN
- EXHAUST AIR DUCT UP & DOWN
- SUPPLY OR OUTSIDE AIR DUCT UP & DOWN
- RETURN AIR DUCT UP & DOWN
- EXHAUST AIR DUCT UP & DOWN
- VAV TERMINAL UNIT
- VVT TERMINAL UNIT
- (E) — EXISTING
- CONNECT TO EXISTING
- T AC-4 — THERMOSTAT OR TEMP. SENSOR
- ? — NOTE
- XX / # — EQUIPMENT DESIGNATOR
- BALL VALVE
- GATE VALVE
- CHECK VALVE
- BALANCING VALVE
- THERMOMETER
- DIRECTION OF FLOW
- PUMP
- STRAINER
- PRESSURE GAUGE
- PETE'S PLUG
- DOUBLE CHECK ASSEMBLY
- PRESSURE REDUCING VALVE
- UNION
- 2-WAY CONTROL VALVE
- 3-WAY CONTROL VALVE
- CAP
- S — SMOKE DETECTOR
- MOTORIZED DAMPER

AFF	-	ABOVE FINISH FLOOR
AHU	-	AIR HANDLING UNIT
B.D.	-	BOTTOM OF DUCT
BHP	-	BRAKE HORSEPOWER
BTU	-	BRITISH THERMAL UNITS
CFM	-	CUBIC FEET PER MINUTE
CONN.	-	CONNECTION
CONT.	-	CONTINUATION
CW	-	DOMESTIC COLD WATER
DB	-	DRY BULB
DIA.	-	DIAMETER
DIST.	-	DISTRIBUTION
EA	-	EXHAUST AIR
EDB	-	ENTERING DRY BULB TEMPERATURE
EWB	-	ENTERING WET BULB TEMPERATURE
EWT	-	ENTERING WATER TEMPERATURE
FF	-	FINISH FLOOR
FIXT.	-	FIXTURE
FPM	-	FEET PER MINUTE
FPS	-	FEET PER SECOND
FT.	-	FEET / FOOT
GA.	-	GAUGE
GPM	-	GALLONS PER MINUTE
H	-	HEIGHT
HP	-	HORSEPOWER
I.D.	-	INSIDE DIAMETER
IN.	-	INCHES
L	-	LENGTH
LBS.	-	POUNDS
LDB	-	LEAVING DRY BULB
LWB	-	LEAVING WET BULB
LWT	-	LEAVING WATER TEMPERATURE
MAX.	-	MAXIMUM
MBH	-	THOUSANDS OF BTUs PER HOUR
MIN.	-	MINIMUM
NC	-	NOISE CRITERIA
N.C.	-	NORMALLY CLOSED
N.I.M.	-	NOT IN MECHANICAL
NO.	-	NUMBER
N.O.	-	NORMALLY OPEN
O.A.	-	OUTSIDE AIR
P	-	PERSON
PSI	-	POUNDS PER SQUARE INCH
P/T	-	PRESSURE / TEMPERATURE
R.A.	-	RETURN AIR
R	-	REFRIGERANT
RECT.	-	RECTANGULAR
REQ'D	-	REQUIRED
S.A.	-	SUPPLY AIR
S.P.	-	STATIC PRESSURE
SQ.	-	SQUARE
TEMP.	-	TEMPERATURE
TYP.	-	TYPICAL
VAV	-	VARIABLE AIR VOLUME
W	-	WIDTH
WB	-	WET BULB
WPD	-	WATER PRESSURE DROP
Ø	-	DIAMETER

PIPING ABREVIATIONS.

(E)	-	EXISTING
(D)	-	DEMOLISH
COND	-	CONDENSATE
G	-	GAS
HWS	-	HEATING WATER SUPPLY
HWR	-	HEATING WATER RETURN
R	-	REFRIGERANT
RAD	-	RADON

AIR DISTRIBUTION DETAILS



GENERAL NOTES

A. THE DRAWINGS ARE DIAGRAMMATIC. PROVIDE ALL MATERIAL (NEW AND UNDAMAGED) AND LABOR FOR A COMPLETE AND OPERABLE SYSTEM. VERIFY ALL BUILDING MEASUREMENTS DIMENSIONS AND EQUIPMENT LOCATIONS BEFORE PROCEEDING WITH ANY OF THE WORK.

B. ALL INSTALLATIONS SHALL COMPLY WITH APPLICABLE FEDERAL AND STATE CODES INCLUDING, 2019 OREGON STRUCTURAL SPECIALTY CODE (OSSC), 2019 OREGON MECHANICAL SPECIALTY CODE (OMSC), 2019 OREGON ZERO ENERGY READY COMMERICAL CODE (OZERCC). NATIONAL FIRE PROTECTION ASSOCIATION (NFPA). WHERE TWO CODES DIFFER THE MORE STRICT OF THE TWO SHALL BE FOLLOWED.

C. OBTAIN ALL NECESSARY PERMITS AND INSPECTIONS REQUIRED BY THE GOVERNING AUTHORITIES HAVING JURISDICTION. SUBMIT ALL CERTIFICATES PRIOR TO ACCEPTANCE.

D. COORDINATE WITH OTHER CRAFTS AS REQUIRED TO COMPLETE WORK IN ACCORDANCE WITH CONSTRUCTION SCHEDULE.

E. PROVIDE OWNER INSTRUCTION BY QUALIFIED PERSONNEL ON EQUIPMENT AND SYSTEMS AT OWNER'S REQUEST.

F. CONTRACTOR TO PROVIDE TESTING, ADJUSTING AND BALANCING REPORT FOR THE AREAS AFFECTED BY THE REMODEL FOR ENGINEER'S REVIEW. SEE SPECS FOR ADDITIONAL TAB REQ'T.

G. CONTRACTOR TO PROVIDE HVAC AS-BUILT DRAWINGS AND OPERATION AND MAINTENANCE MANUALS WTIHIN 90 DAYS OF SYSTEM ACCEPTANCE.

H. CONTRACTOR TO TEXT HVAC CONTROL SYSTEM TO ENSURE PROPER OPERATION, CALIBRATION AND ADJUSTMENT OF CONTROLS.

SPLIT SYSTEMS

INDOOR UNIT MARK/ NUMBER	IHP 1.1	IHP 1.2
HEAT PUMP/COOLING ONLY	HEAT PUMP	HEAT PUMP
LOCATION	LIVING	CONFERENCE
TYPE	CEILING CASSETTE	WALL MOUNTED
COOLING/HEATING CAPACITY (Btu/h)	18,000	18,000
COOLING SUPPLY CFM	448/378/275	583/484
OSA CFM	NOTE 2	NOTE 2
VOLT/PHASE	240/1, NOTE 1	240/1, NOTE 1
CONDENSATE PUMP	YES	YES
WEIGHT (lbs)	39	31
BASIS OF DESIGN - DAIKIN	FFQ18W2VJU9	FTXS18WVJU9
OUTDOOR UNIT MARK/ NUMBER	OHP 1	
# OF INDOOR UNITS	2	
EFFICIENCY (HSPF, SEER)	12.2/17.7	
NOMINAL TONS	3T	
NOMINAL COOLING CAPACITY (Btu/h)	36,000	
NOMINAL HEATING CAPACITY (Btu/h)	36,000	
REFRIGERANT	R410A	
MAX PIPE LENGTH (FEET)	230	
MAX PIPE HEIGHT (FEET)	50	
VOLTS/PHASE	240/1	
MCA/MOP	23.9/25	
COMPRESSOR	INVERTOR	
WEIGHT (lbs)	140	
BASIS OF DESIGN - DAIKIN	MXS36RMVJUA	

NOTES:
1. POWERED FROM OUTDOOR UNIT
2. OPERABLE WINDOWS

ELECTRIC HEATER SCHEDULE

EQUIPMENT MARK/ NUMBER	CH 1	CH 2	CH 3	CH 4
WATTS	1200			
VOLTS/PH	240/1			
AMPS	5.0			
LENGTH (IN)	94			
LOCATION	12" BELOW CEILNG			
MOUNTING	WALL BRACKET			
OPERATING WEIGHT (LBS)	10			
BASIS OF DESIGN: QMARK	12008C			
NOTES	1,2			

NOTES:
1. BUILT IN THERMOSTAT
2. HANGING BRACKET

EXHAUST FAN SCHEDULE

EXHAUST FAN MARK/ NUMBER	EF 1	EF 2	EF 3
TYPE	CEILING CABINET	CEILING CABINET	CEILING CABINET
SYSTEM	SHOWER	TOILET	LAUNDRY
CFM	100	100	100
WHEEL TYPE	BI	BI	BI
EXT. STATIC PRESS (IN WC)	.30"	.30"	.30"
RPM	1023	1023	1023
MOTOR HP	12 WATTS	12 WATTS	12 WATTS
CONTROLLED BY	WALL SWITCH WITH 5 MINUTE DELAY	WALL SWITCH WITH 5 MINUTE DELAY	WITH INTEGRAL HUMIDSTAT
INTERLOCK WITH	--	--	--
BACKDRAFT DAMPER	YES	YES	YES
MOTORIZED DAMPER	NO	NO	NO
MAXIMUM INLET SONES	1.5	1.5	1.5
VOLTAGE/PH	115/1	115/1	115/1
OPERATING WEIGHT (LBS)	20	20	20
BASIS OF DESIGN - GREENHECK	SP-A90-130-VG	SP-A90-130-VG	SP-A90-130-VG

MECHANICAL DRAWING INDEX

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REGISTERED PROFESSIONAL ENGINEER
70996PE
OREGON
JULY 9, 2002
TAKAKO BAKER
EXPIRES: 12/31/25

PROJECT NO.: **21.59**

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH MAY FIRE DISTRICT
67577 EAST BAY RD.
NORTH BEND, OR 97459

Project Status

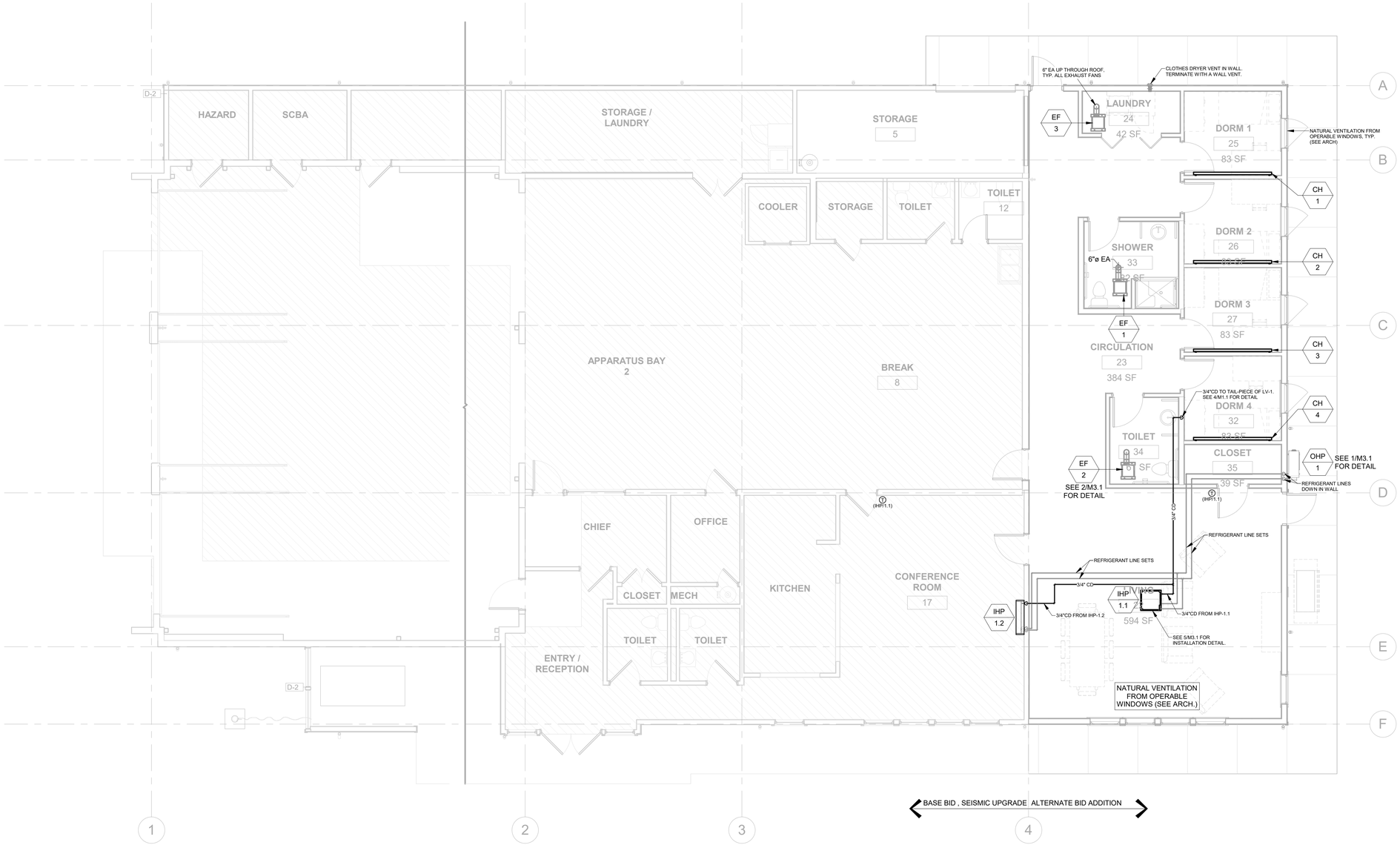
REVISIONS:

#	DATE	DESCRIPTION

DATE: Issue Date

SHEET TITLE:
MECHANICAL SCHEDULES AND NOTES

M1.1



1 MECHANICAL FLOOR PLAN - ALTERNATE BID
3/16" = 1'-0"

NORTH

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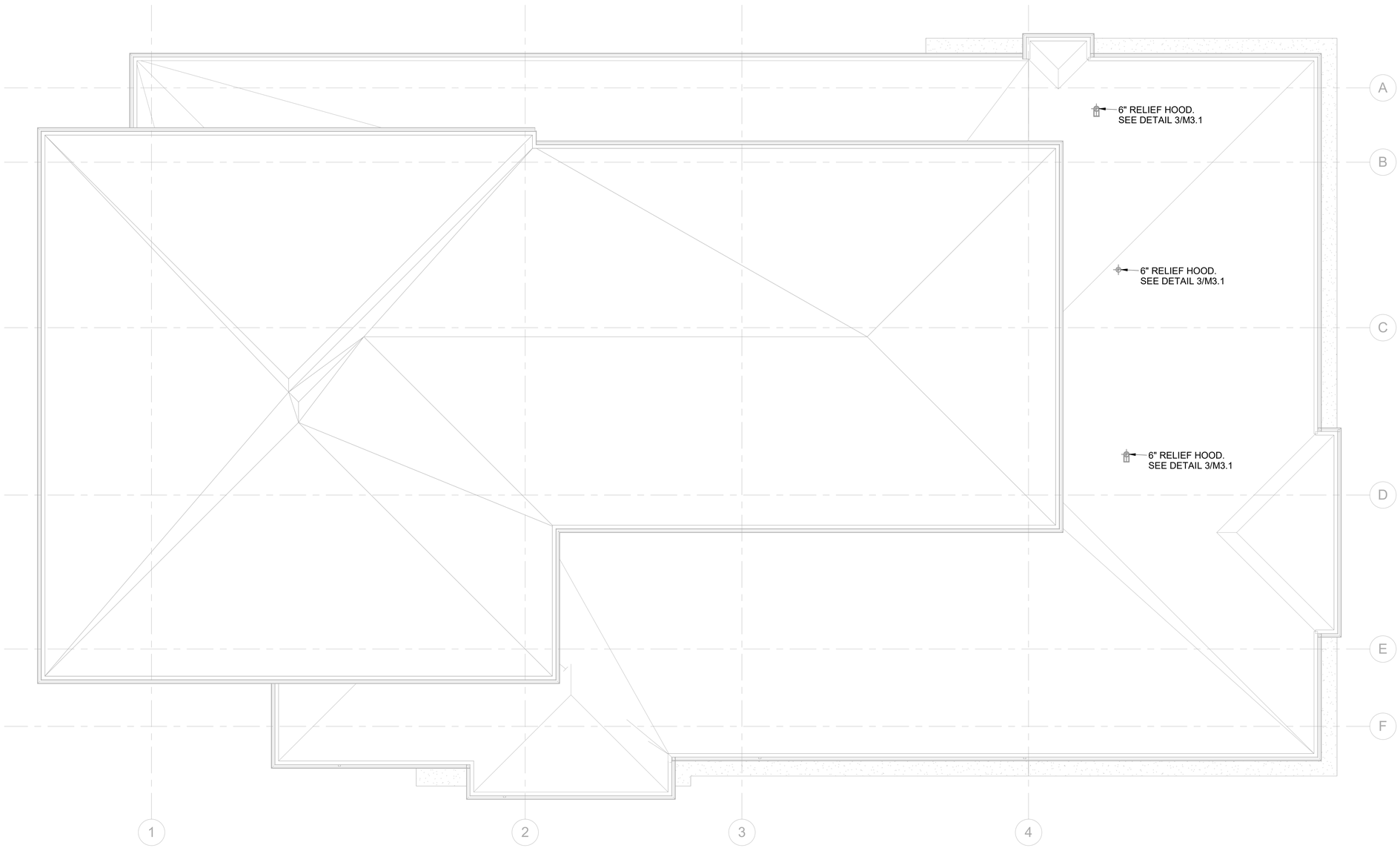
SHEET TITLE:
MECHANICAL FLOOR PLAN

M2.1



1 MECHANICAL ROOF PLAN
3/16" = 1'-0"

NORTH

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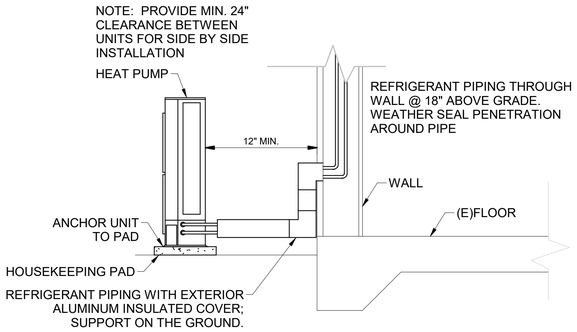
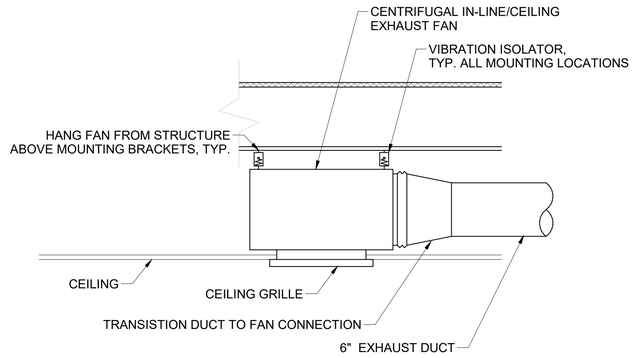
DATE: Issue Date

SHEET TITLE:
MECHANICAL ROOF PLAN

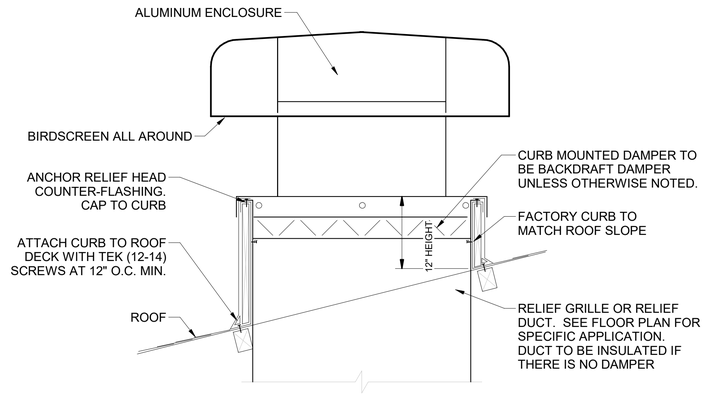
M2.2



1 DETAIL - OUTDOOR UNIT
1" = 1'-0"



2 DETAIL - RESTROOM EXHAUST FAN



3 DETAIL - RELIEF/INTAKE/EXHAUST HEAD
1" = 1'-0"

4 DETAIL - FIXED AIR GAP
1" = 1'-0"

5 DETAIL - CEILING CASSETTE

6 DETAIL - DRYER OUTLET

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SHEET TITLE:
MECHANICAL DETAILS

M3.1

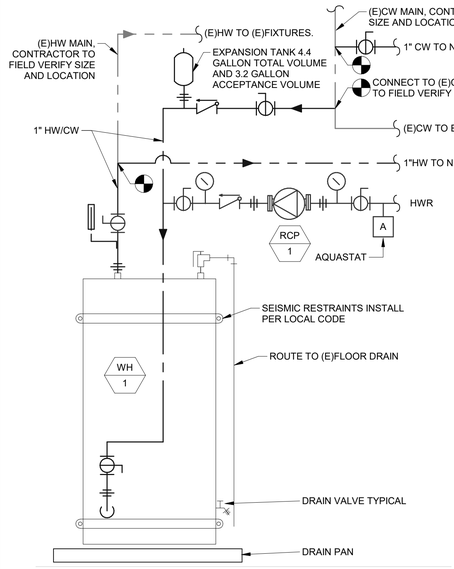
PLUMBING LEGEND

AFF	-----	ABOVE FINISHED FLOOR
ARCH	-----	ARCHITECTURAL
B.G.	-----	BELOW GRADE
BTU	-----	BRITISH THERMAL UNIT
CAP.	-----	CAPACITY
C.I.	-----	CAST IRON
CO	-----	CLEANOUT
COMP.	-----	COMPARTMENT
CONT.	-----	CONTINUATION
CU.	-----	CUBIC
DF	-----	DRINKING FOUNTAIN
DI	-----	DEIONIZED (WATER)
DIA.	-----	DIAMETER
ELEV.	-----	ELEVATION
EWC	-----	ELECTRIC WATER COOLER
FD	-----	FLOOR DRAIN
FDC	-----	FIRE DEPARTMENT CONNECTION
F.F.	-----	FINISH FLOOR
FLG.	-----	FLANGE
FT	-----	FOOT / FEET
G	-----	GAS
GA.	-----	GAUGE
GALV.	-----	GALVANIZED
GPM	-----	GALLONS PER MINUTE
G.V.	-----	GATE VALVE
HP	-----	HORSEPOWER
HR.	-----	HOUR
I.E.	-----	INVERT ELEVATION
kW	-----	KILOWATT
LAV	-----	LAVATORY
LBS	-----	POUNDS
MAX.	-----	MAXIMUM
MBH	-----	THOUSANDS OF BTUs PER HOUR
MIN.	-----	MINIMUM
M.J.	-----	MECHANICAL JOINT
N.I.M.	-----	NOT IN MECHANICAL
OS&Y	-----	OUTSIDE STEM & YOKE
PROT.	-----	PROTECTION
PRV	-----	PRESSURE REDUCING VALVE
PSI, PSIG	-----	POUNDS PER SQUARE INCH
P/T	-----	PRESSURE / TEMPERATURE
RD	-----	RAIN DRAIN / STORM DRAIN
REQ'D	-----	REQUIRED
RPBP	-----	REDUCED PRESSURE BACKFLOW PREVENTER
RPM	-----	REVOLUTIONS PER MINUTE
TYP.	-----	TYPICAL
UR	-----	URINAL
VTR	-----	VENT THROUGH ROOF
WC	-----	WATER CLOSET
WCO	-----	WALL CLEANOUT

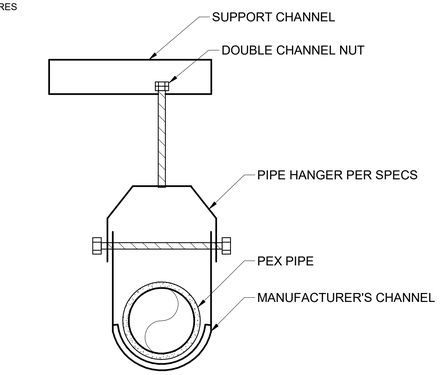
- (E)CW - EXISTING DOMESTIC COLD WATER PIPE
- CW - DOMESTIC COLD WATER PIPE
- (E)HW - EXISTING DOMESTIC HOT WATER PIPE
- HW - DOMESTIC HOT WATER PIPE
- HWR - DOMESTIC HOT WATER RECIRCULATION PIPE
- RD - RAIN DRAINAGE PIPE
- W - DOMESTIC SANITARY WASTE PIPE
- V - DOMESTIC SANITARY VENT PIPE

- CLEANOUT
- HOSE BIBB
- FLOOR DRAIN
- PRESSURE/TEMP RELIEF VALVE
- BUTTERFLY VALVE
- GAS PRESSURE REGULATING VALVE
- TOP CONNECTION
- BOTTOM CONNECTION
- PIPE TURNED UP, PIPE TURNED DOWN
- GATE VALVE
- BALL VALVE
- BALANCING VALVE
- CHECK VALVE
- UNION
- DOUBLE CHECK ASSEMBLY

- XXX — FIXTURE MARK
- XX / # — EQUIPMENT MARK NUMBER
- # — NOTE
- CAP
- TEE
- ELBOW



1 DETAIL - WATER HEATER



NOTE: INSULATE OVER MANUFACTURER'S CHANNEL

4 DETAIL - PEX SUPPORT

2 DETAIL - NON-SEISMIC PIPE SUPPORT

3 DETAIL - DOWNSPOUT CONNECTION
3/4" = 1'-0"

WATER HEATER SCHEDULE

EQUIPMENT MARK/ NUMBER	WH 1
TYPE	ELECTRIC
CAPACITY (GAL)	50
POWER (kW)	4.5
RECOVERY CAP. @100F TR (GPH)	18
ELECTRICAL (V/PH)	240V/1
SHIPPING WEIGHT (LBS)	125
BASIS OF DESIGN: AO SMITH	DEN-40

PUMP SCHEDULE

EQUIPMENT MARK/ NUMBER	RCP 1
SERVICE	HW RECIRC. (HWR)
TYPE	CIRCULATION
CONTROLLED BY	AQUASTAT
ARRANGEMENT	IN-LINE
FLOW RATE (GPM)	3.0
HEAD (FT)	15
MOTOR HP	90 WATTS
ELECTRICAL	115/1
DESIGN WEIGHT	20

PLUMBING CALCULATIONS - 2021 OPSC APPENDIX A

			DOMESTIC WATER SERVICE				SANITARY WASTE	
FIXTURE TYPE	NUMBER OF FIXTURES	WATER FIXTURE UNITS	TOTAL WSFU	TOTAL CW FIXTURE UNITS	TOTAL HW FIXTURE UNITS		DRAINAGE FIXTURE UNITS	TOTAL DFU
CLOTHES WASHER	1	4	4	3	3		3	3
LAVATORY (SINGLE)	2	1	2	1.5	1.5		1	2
SHOWER (STALL)	1	2	2	1.5	1.5		2	2
WATER CLOSET (1.6 GPF TANK-GENERAL)	2	2.5	5	5	0		4	8
HOSE BIBB (FIRST ONE)	1	2.5	2.5	2.5	0		---	---
TOTAL	6		15.5	13.5	6			15
		GPM	11.5	10.25	5	**WASTE SIZE**		4"
		SUPPLY SIZE	1"	1"	3/4"			
		METER SIZE	--	**PER OREGON W-4**				

PLUMBING CONNECTION SCHEDULE

FIXTURE TAG	FIXTURE	W	V	CW	HW	REMARKS
WC-1	WATER CLOSET	4"	2"	1/2"	--	FLOOR MOUNT, TANK STYLE
LV-1	LAVATORY	1-1/2"	1-1/2"	1/2"	1/2"	COUNTER MOUNT, SEE SPECS
WF-1	WASHER FITTING	2"	1-1/2"	1/2"	1/2"	SEE SPECS
SH-1	SHOWER	2"	1-1/2"	1/2"	1/2"	ADA SHOWER, SEE SPECS
HB-1	HOSE BIBB	--	--	3/4"	--	FROST FREE, SEE SPECS

PLUMBING DRAWING INDEX

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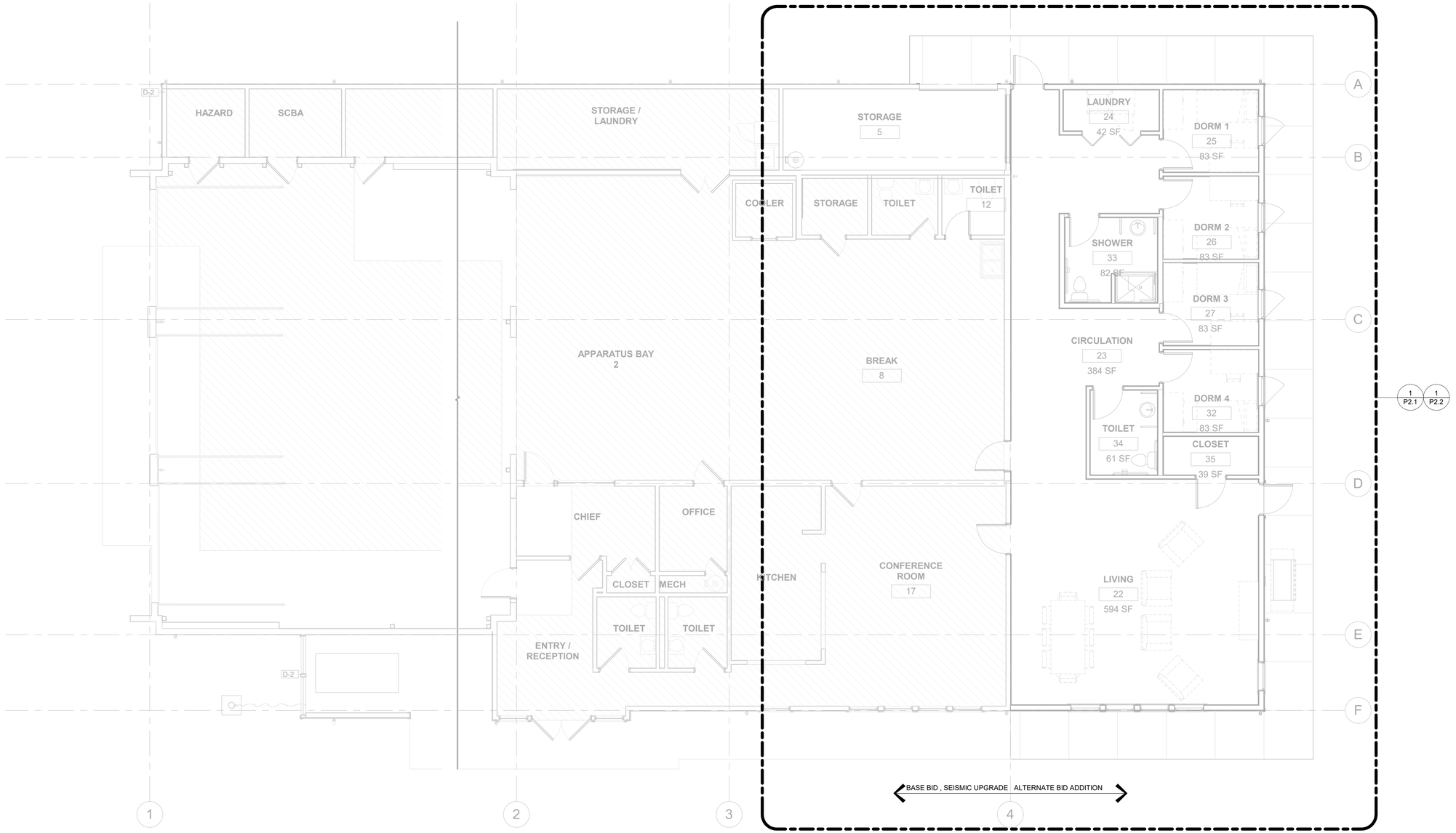
PLUMBING SCHEDULES AND DETAILS

P0.1



1 PLUMBING OVERALL FLOOR PLAN
3/16" = 1'-0"

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NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH MAY FIRE DISTRICT
67577 EAST BAY RD.
NORTH BEND, OR 97459

PROJECT NO.: 21.59

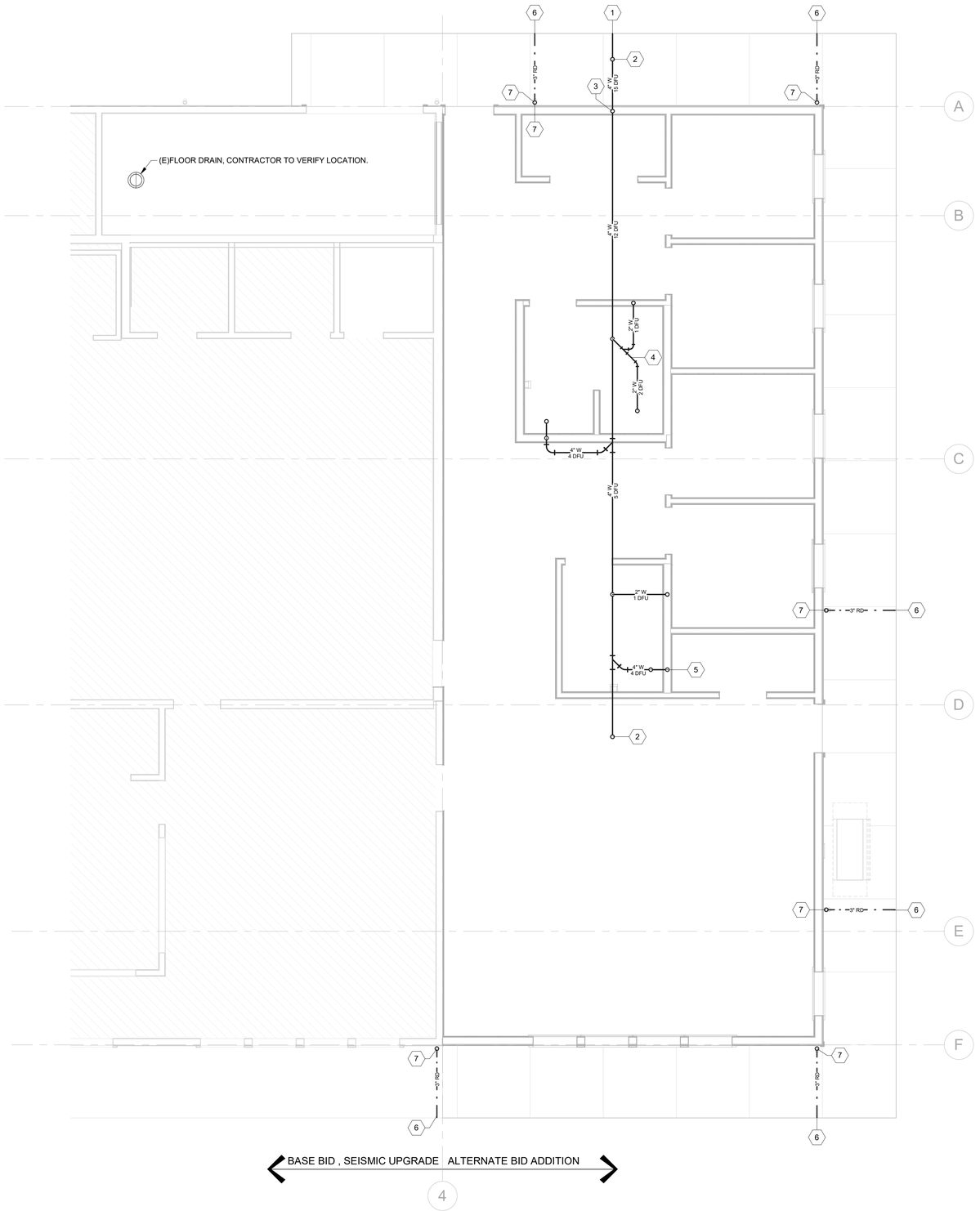
Project Status

REVISIONS:

DATE: Issue Date

SHEET TITLE:
PLUMBING OVERALL FLOOR PLAN

P1.0



1 PLUMBING ENLARGED UNDERSLAB PLAN - ALTERNATE BID
1/4" = 1'-0"

KEYNOTES

1 4"W, SEE SITE PLAN FOR CONTINUATION.

2 WASTE PIPE DOWN FROM CLEANOUT.

3 2"W DOWN FROM LEVEL ABOVE.

4 WET VENTED PIPE.

5 2"V UP TO NEXT LEVEL.

6 3"RD, SEE SITE PLAN FOR CONTINUATION.

7 3"RD DOWN FROM LEVEL ABOVE. CONNECT TO DOWNSPOUT, SEE 3/P0.1 FOR DETAIL.

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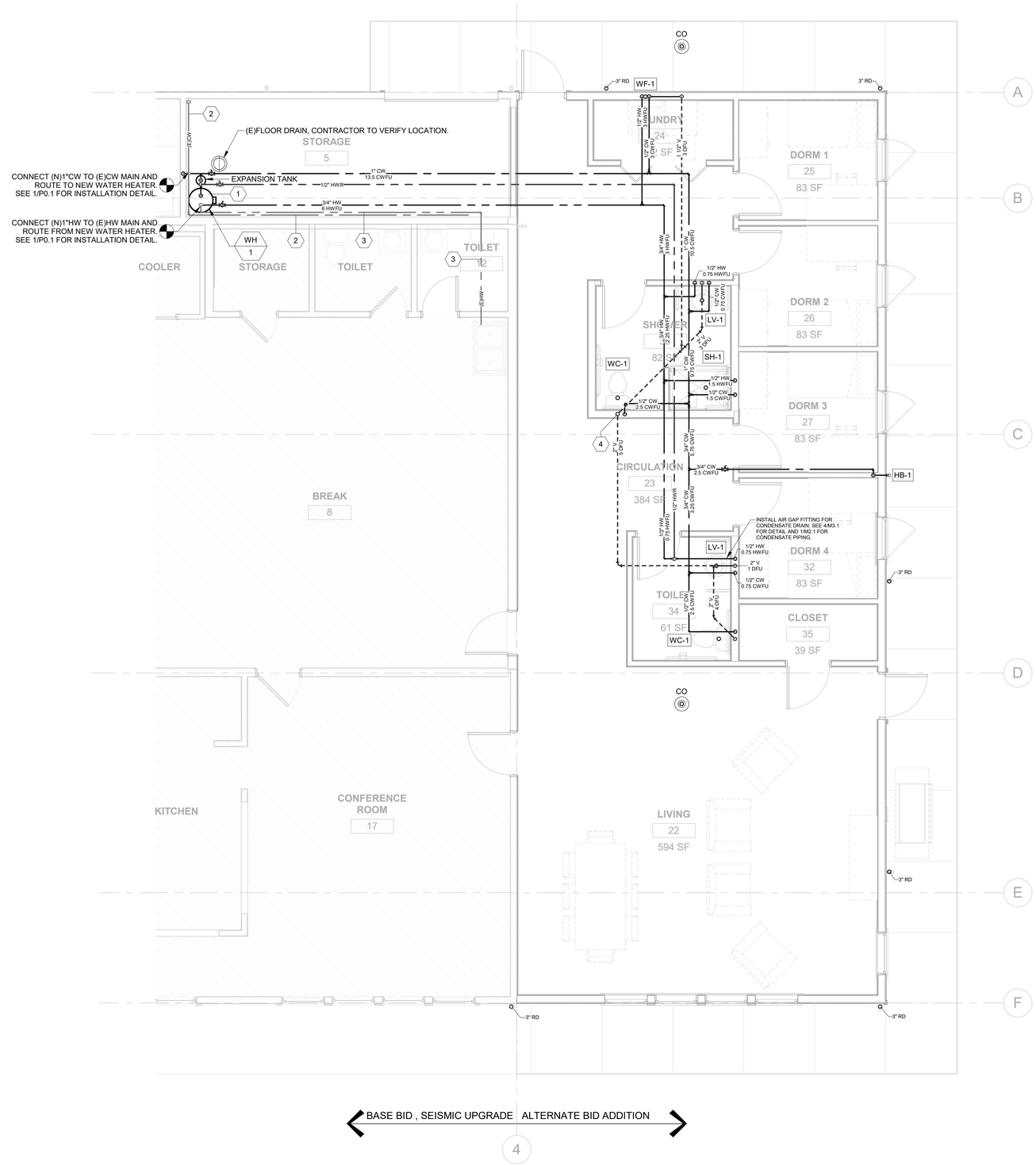
SHEET TITLE:
PLUMBING ENLARGED UNDERSLAB PLAN

P2.1



BASE BID , SEISMIC UPGRADE ALTERNATE BID ADDITION

4

1 PLUMBING ENLARGED FLOOR PLAN - ALTERNATE BID
1/4" = 1'-0"

KEYNOTES

1 NEW WH-1 & RCP-1, SEE 1/P0.1 FOR DETAIL.

2 ASSUMED LOCATION OF (E)CW MAIN. CONTRACTOR TO VERIFY LOCATION.

3 ASSUMED LOCATION OF (E)HW MAIN. CONTRACTOR TO VERIFY LOCATION.

4 4"VTR.

GENERAL NOTES:

1. SEE M1.1 FOR CONDENSATE PIPIPNG PLAN.

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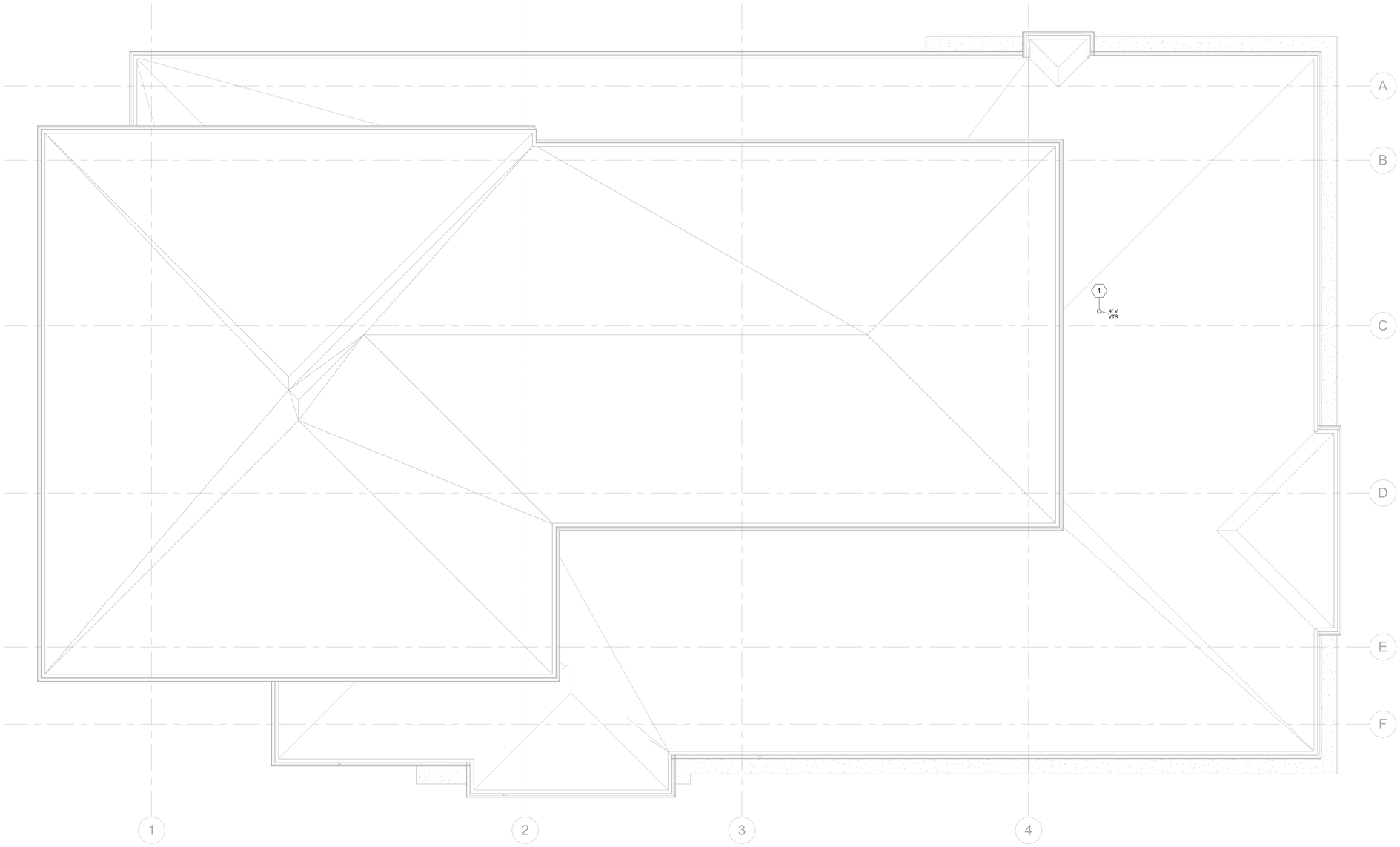
PLUMBING ENLARGED FLOOR PLAN

P2.2

KEYNOTES

1 4"VTR.



1 PLUMBING ROOF PLAN
3/16" = 1'-0"

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PLUMBING ROOF PLAN

P2.3

ELECTRICAL SYMBOL SCHEDULE

SYMBOLS	ONELINE DIAGRAM	NOTES
	MOLDED CASE CIRCUIT BREAKER	
	TRANSFORMER	
	CURRENT TRANSFORMER(S)	
M	METER, TYPE AS NOTED	
	GROUND	
BN	NEUTRAL BUS	
2	MOTOR WITH MOTOR NUMBER (SEE EQUIPMENT SCHEDULE)	
D	COMBINATION FIRE SMOKE DAMPER	2
	EQUIPMENT NUMBER (SEE EQUIPMENT SCHEDULE)	
	NON-FUSED DISCONNECT SWITCH	
	FUSED DISCONNECT SWITCH (FUSES SIZED PER EQUIPMENT MANUFACTURERS RECOMMENDATIONS UNO.)	
	COMBINATION MOTOR STARTER / FUSED DISCONNECT SWITCH	
	SUB-DISTRIBUTION PANELBOARD OR SWITCHBOARD	
	BRANCH CIRCUIT PANELBOARD	
	MISCELLANEOUS PANEL AS NOTED	
	MAIN DISTRIBUTION PANELBOARD	
T	TRANSFORMER	

SYMBOLS	RACEWAYS	NOTES
	BRANCH CIRCUIT INSTALLED CONCEALED FROM FINISH SPACES . PROVIDE GROUND CONDUCTOR AS INDICATED IN PANEL SCHEDULE . GROUND CONDUCTOR NOT INCLUDED IN HASH MARK INDICATION .	
	BRANCH CIRCUIT INSTALLED IN OR BELOW FLOOR . PROVIDE GROUND CONDUCTOR AS INDICATED IN PANEL SCHEDULE . GROUND CONDUCTOR NOT INCLUDED IN HASH MARK INDICATION .	
L4-13.5	BRANCH CIRCUIT HOME RUN TO PANEL . HASH MARKS INDICATES NUMBER OF CONDUCTORS . PROVIDE GROUND CONDUCTOR AS INDICATED IN PANEL SCHEDULE . GROUND CONDUCTOR NOT INCLUDED IN HASH MARK INDICATION .	
	LOW VOLTAGE EMPTY CONDUIT WITH PULL STRING - ¾" UNO	
PB	PULL BOX , 6" x 6" x 4" UNLESS NOTED OTHERWISE	
J	JUNCTION BOX , 4" SQUARE UNLESS OTHERWISE NOTED	
	4" CONDUIT SLEEVE WITH BUSHINGS AT BOTH ENDS . LOCATE AT 6" ABOVE ACCESSIBLE CEILING . FIRESTOP WITH UL APPROVED SYSTEM .	
	CONDUIT STUB-OUT, CAP & MARK WITH APPROVED MARKER	
	CONDUIT , UP	
	CONDUIT , DOWN	

SYMBOLS	RECEPTACLES	NOTES
•	WHEN ADDED TO A SYMBOL , INDICATES OUTLET MOUNTED WITH BOTTOM OF OUTLET AT 2" ABOVE COUNTER TOP OR BACK SPLASH UNO.	
	DUPLEX CONVENIENCE OUTLET	+ 18"
	GFI DUPLEX CONVENIENCE OUTLET	+ 18"
	DUPLEX OUTLET WITH USB OUTLETS	+ 18"
	DOUBLE DUPLEX CONVENIENCE OUTLET	+ 18"
	SINGLE PHASE SPECIAL PURPOSE OUTLETS, AS NOTED	+ 18" UNO
	THREE PHASE SPECIAL PURPOSE OUTLETS, AS NOTED	
	FLUSH FLOOR OUTLET AS SHOWN	

SYMBOLS	TELEPHONE / DATA	NOTES
•	WHEN ADDED TO SYMBOL , INDICATES OUTLET MOUNTED WITH BOTTOM OF OUTLET AT 2" ABOVE COUNTER TOP OR BACKSPLASH UNO	
	TELE/DATA. PROVIDE (2) CAT6 CABLES UNO	+ 18" 1
W	'W' ADDED TO SYMBOL INDICATES WALL MOUNTED	+ 60" 1
	FLUSH FLOOR OUTLET AS SHOWN	1
TTB	TELEPHONE TERMINAL BOARD , 8" HIGH (WIDTH AS SHOWN) , ¾" FIRE RESISTIVE PLYWOOD WITH * 6 CU GND	
	WIRELESS ACCESS PORT. PROVIDE (1) CAT6 CABLES	1
	TELE/DATA. PROVIDE (1) CAT6 CABLES UNO	+ 18" 1

NOTES

1. ALL SYMBOLS MAY NOT APPLY DIRECTLY TO THIS JOB.
2. ALL MOUNTING HEIGHTS SHOWN ARE TO CENTERLINE OF DEVICE.
3. ALL MOUNTING HEIGHTS ARE TYPICAL ON PLANS.

KEYED NOTES

1. PROVIDE 1" CONDUIT FROM OUTLET BOX TO ACCESSIBLE LOCATION ABOVE CEILING, UNLESS NOTED OTHERWISE. TERMINATE CONDUITS WITH BLUE INSULATED BOX CONNECTORS AND LABEL SYSTEM, SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION . ROUTE CAT6 or 6a CABLE(S) TO DATA RACK

SYMBOLS	LIGHT FIXTURES	NOTES
	WHEN ADDED TO LIGHT FIXTURE SYMBOL - INDICATES WALL OR BRACKET MOUNTED LIGHT FIXTURE	
FD 2. 4	SURFACE OR PENDANT MOUNTED LIGHT FIXTURE OUTLET . (NUMBER INDICATES CIRCUIT , CAPITAL LETTER INDICATES FIXTURE TYPE , LOWER CASE LETTER INDICATES SWITCHING CONTROL , TYPICAL FOR ALL LIGHT FIXTURES)	
	RECESSED CEILING LIGHT FIXTURE	
	RECESSED WALL WASHER , UNSHADED SIDE INDICATES DIRECTION OF WALL WASHING	
	FLUORESCENT LIGHT FIXTURE	
	FLUORESCENT STRIP LIGHT FIXTURE	
E E	SINGLE FACE EXIT SIGN WITH NUMBER OF DIRECTIONAL ARROWS AS SHOWN , CEILING MOUNTED , SOLID QUADRANT INDICATES FACE.	

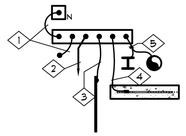
SYMBOLS	SWITCHES	NOTES
$	SINGLE POLE LIGHT SWITCH	+ 46"
$3	THREE WAY LIGHT SWITCH	+ 46"
$M	MOTOR RATED SWITCH	+ 46"
MC MLU	OCCUPANCY SENSOR - C=CEILING W=WALL MOUNTED	
P	PHOTOELECTRIC SWITCH	

SYMBOLS	SECURITY	NOTES
	SECURITY CAMERA. PROVIDE J-BOX WITH CAT 6 CABLE	1
L	ELECTRONICALLY CONTROLLED LOCK	
	DOOR POSITION SWITCH	
M	MOTION DETECTOR (OMNI DIRECTIONAL)	
CR	CARDREADER	+ 44"
KP	KEYPAD	+ 44"

SYMBOLS	AUDIO / VISUAL	NOTES
S	CEILING SPEAKER	
S	WALL MOUNTED SPEAKER	+ 80"
V	WALL MOUNTED SPEAKER HORN	+ 80"
TV	TELEVISION (VIDEO) OUTLET	+ 18"
IR	INTERCOM REQUEST STATION (SPEAKER & PUSH BUTTON)	+ 44"

SYMBOLS	FIRE ALARM	NOTES
M	MANUAL PULL STATION	+ 44"
V	COMBINATION VISUAL / AUDIBLE ALARM	+ 80" AFFB
V	VISUAL STROBE ALARM	+ 80" AFFB
P	PHOTOELECTRIC SMOKE DETECTOR (CEILING MOUNTED UNO)	
I	IONIZATION SMOKE DETECTOR (CEILING MOUNTED UNO)	
D	MAGNETIC DOOR HOLDER	
H	HEAT DETECTOR (CEILING MOUNTED, 135° UNO)	

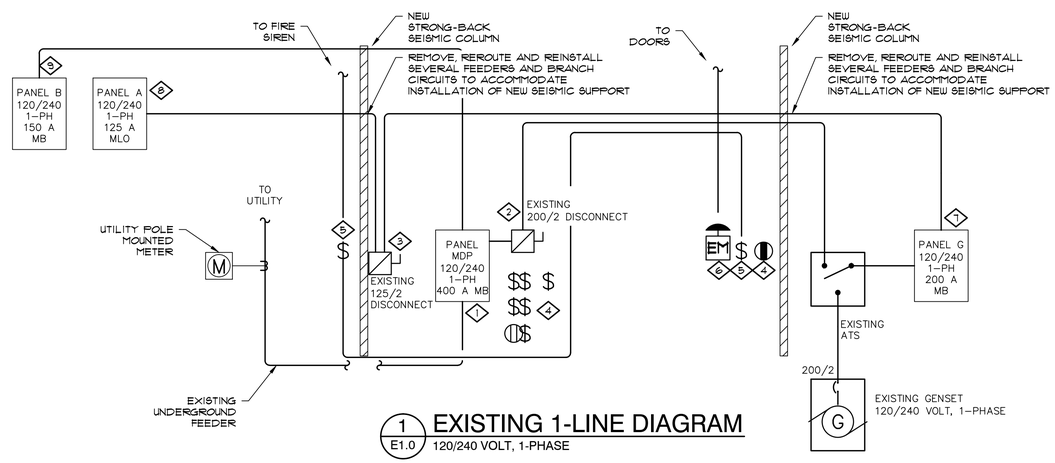
SYMBOLS	ABBREVIATIONS	NOTES
AIC	AMPERE INTERRUPTING CAPACITY	
AMP	AMPERE	
C	CONDUIT	
EC	EMPTY CONDUIT (WITH PULL-IN LINE)	
ELEC	ELECTRICAL	
FAAP	FIRE ALARM ANNUNCIATOR PANEL	
FACP	FIRE ALARM CONTROL PANEL	
G, GND	GROUND	
GEN	GENERATOR	
GFI	GROUND FAULT CIRCUIT INTERRUPTER TYPE	
HP	HORSEPOWER	
IG	ISOLATED GROUND	
MECH	MECHANICAL	
MFGR	MANUFACTURER	
NEC	NATIONAL ELECTRIC CODE	
NL	NIGHT LIGHT	24 HOUR 'ON'
OFCI	OWNER FURNISHED CONTRACTOR INSTALLED	
OFOI	OWNER FURNISHED OWNER INSTALLED	
PB	PULL BOX	
ϕ PH	PHASE	
PNL	PANEL	
PWR	POWER	
SYS	SYSTEM	
T	TELEPHONE	
TTB	TELEPHONE TERMINAL BOARD	
TYP	TYPICAL	
UNO	UNLESS NOTED OTHERWISE	
V	VOLT	
VP	VANDAL PROOF	
W	WATT	
WP	WEATHERPROOF TYPE	



DETAIL NOTES

1. #1/0 KCMIL CU MAIN BONDING JUMPER AND EQUIPMENT BONDING JUMPER PER NEC 250.28(D) AND 250.102(C)
2. #1/0 KCMIL CU GROUNDING ELECTRODE SYSTEM JUMPER PER NEC 250.52(A)(1),3 AND 5).
3. #6 CU TO ROD, PIPE, OR PLATE ELECTRODES PER NEC 250.66(A)
4. #4 CU TO CONCRETE ENCASED ELECTRODE PER NEC 250.52(A)(3) AND 250.66(B)
5. WHERE REQUIRED, BOND PIPING SYSTEMS AND EXPOSED STRUCTURAL STEEL PER NEC 250.104

2 GROUNDING DETAIL
E0.1 TYPICAL



1 EXISTING 1-LINE DIAGRAM
E1.0 120/240 VOLT, 1-PHASE

KEYED NOTES

1. 400 AMP MDP: REPLACE EXISTING 400 AMP MDP WITH NEW 400 AMP PANEL. PROVIDE NEW 200/2 BREAKER TO SUB-FEED ATS/GENERATOR PANEL 'G'. PROVIDE NEW 150/2 BREAKER TO SUB-FEED NEW EAST ADDITION PANEL 'B'. FIELD COORDINATE ALL OTHER BREAKER REQUIREMENTS WITH EXISTING EQUIPMENT TO REMAIN. PROVIDE NEW TYPE WRITTEN PANEL SCHEDULE WITH PLASTIC COVER. PROVIDE NEW ENGRAVED PHYNOLIC PANEL LABEL WITH NAME, VOLTAGE AND AMP RATING.
2. EXISTING 200/2 DISCONNECT: DEMO EXISTING DISCONNECT AND REFEED ATS/GENERATOR FROM NEW BREAKER INSIDE PANEL MDP.
3. EXISTING 125/2 DISCONNECT: DEMO EXISTING DISCONNECT AND REFEED PANEL A (IN OFFICE AREA) DIRECTLY FROM EXISTING BREAKER IN PANEL 'G'
4. VARIOUS RECEPTACLES AND SWITCHES WITH ASSOCIATED CONDUIT AND CONDUCTORS. FIELD COORDINATE REQUIREMENTS TO ACCOMMODATE INSTALLATION OF NEW SEISMIC SUPPORTS.
5. OLD FIRE SIREN CONTROLS: REMOVE EXISTING FIRE SIREN CONTROLS AND POWER COMPLETELY. FIRE SIREN TO BE REMOVED BY OTHERS.
6. EMERGENCY GARAGE DOOR OPENER CONTROL: REMOVE AND REINSTALL COMPLETELY TO ACCOMMODATE SEISMIC SUPPORT INSTALLATION.
7. EXISTING PANEL'G': FIELD VERIFY ALL LOADS IN PANEL 'G'. PROVIDE NEW TYPE WRITTEN PANEL SCHEDULE IN PLASTIC COVER. PROVIDE NEW ENGRAVED HYNOLIC LABEL WITH PANEL NAME, VOLTAGE, AND AMP RATING.
8. EXISTING PANEL'A': FIELD VERIFY ALL LOADS IN PANEL 'A'. PROVIDE NEW TYPE WRITTEN PANEL SCHEDULE IN PLASTIC COVER. PROVIDE NEW ENGRAVED HYNOLIC LABEL WITH PANEL NAME, VOLTAGE, AND AMP RATING.
9. NEW PANEL 'B': PROVIDE NEW 32-POLE 150 AMP MAIN BREAKER PANEL IN NEW ADDITION. SEE E2.2 FOR ADDITIONAL DETAILS AND PANEL SCHEDULE. FAULT CURRENT < 6,000 AIC SEE PANEL SCHEDULE FOR FEEDER SIZE.

PROJECT NOTES

1. WORK SHOWN ON PLAN IS BASED ON AVAILABLE INFORMATION AT THE TIME OF DESIGN. FIELD COORDINATE SEISMIC UPGRADE REQUIREMENTS WITH ARCHITECTURAL AND STRUCTURAL. CONTRACTOR IS TO FIELD VERIFY EXISTING CIRCUITING AND INSTALLATION.
2. CONTRACTOR SHALL COORDINATE AND PERFORM NECESSARY ELECTRICAL DEMOLITION WORK ASSOCIATED WITH ALL ITEMS AND EQUIPMENT TO BE REMOVED.
3. CONFIRM THAT ALL EXISTING DEVICES AND EQUIPMENT PLANNED FOR REUSE ARE IN GOOD OPERATING CONDITION. UNSUITABLE ITEMS SHALL NOT BE REUSED. RETURN ALL OTHER ITEMS SUITABLE FOR REUSE TO OWNER.
4. WIRING WHICH SERVES USABLE EXISTING OUTLETS SHALL BE REROUTED AND RESTORED CLEAR OF CONSTRUCTION. MAINTAIN ELECTRICAL CONTINUITY OF EXISTING SYSTEM. REPAIR AND RECONDITION ASSOCIATED SURFACES TO MATCH ADJACENT SURFACES. VERIFY EXACT LOCATIONS IN THE FIELD.

PANEL 'MDP' 1 — FAULT CURRENT = 9,430

400 AMP MAIN BREAKER — 120 / 240 VOLTS — 1-PHASE, 3-WIRE

EXISTING FEEDER — SURFACE MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	25%
CONNECTED VA	720	10460	5736	0	0	9800	0	14468	12248	26716	121	33395 VA	151
DIVERSITY FACTOR	125%	98%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	900	10230	5736	0	0	9800	0	14517	12149	26666	121	33333 VA	151

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR	PH	BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1	P	PANEL B	14517					150	2 A	200					0	PANEL G	P	2
3	P	PANEL B	12149						B						0	PANEL G	P	4
5			0		12	12	1/2	20	2 A	20	1/2	12	12		0			6
7			0						B						0			8
9			0		12	12	1/2	20	2 A	20	1/2	12	12		0			10
11			0						B						0			12
13			0		12	12	1/2	20	2 A	20	1/2	12	12		0			14
15			0						B						0			16
17			0		12	12	1/2	20	1 A	20	1/2	12	12		0			18
19			0		12	12	1/2	20	1 B	20	1/2	12	12		0			20
21			0		12	12	1/2	20	1 A	20	1/2	12	12		0			22
23			0		12	12	1/2	20	1 B	20	1/2	12	12		0			24
25			0		12	12	1/2	20	1 A	20	1/2	12	12		0			26
27			0		12	12	1/2	20	1 B	20	1/2	12	12		0			28
29									A									30
31									B									32

MDP — 1/10/2024 — FED FROM UTILITY TRANSFORMER

ALL CIRCUIT CONDUCTORS SIZED FOR COPPER — North Bay Schedules

EXISTING OFFICE PANEL — PANEL 'A' 8 — FAULT CURRENT = 4,960

125 AMP MLO — 120 / 240 VOLTS — 1-PHASE, 3-WIRE

EXISTING FEEDER — SURFACE MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	25%
CONNECTED VA	0	0	0	0	0	0	0	0	0	0	0	0 VA	
DIVERSITY FACTOR	125%	100%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	0	0	0	0	0	0	0	0	0	0	0	0 VA	

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR	PH	BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1	H	MEETING RM HEATER	0		12	12	1/2	20	2 A	20	1/2	12	12		0	HALLWAY HEATER	H	2
3	H		0						B						0		H	4
5	H	WATER HEATER	0		12	12	1/2	20	2 A	20	1/2	12	12		0	LTS & PLUGS	L	6
7	H		0						B	20	1/2	12	12		0	LTS & PLUGS	L	8
9	H	RANGE	0		12	12	1/2	20	2 A	20	1/2	12	12		0	TOILET LTS	L	10
11	H		0						B	20	1/2	12	12		0	TOILET LTS	L	12
13	H	OFFICE HEATER	0		12	12	1/2	20	2 A	20	1/2	12	12		0	TOILET LTS	L	14
15	H		0						B	20	1/2	12	12		0	KITCHEN LTS & PLUGS	L	16
17	L	LTS & PLUGS	0		12	12	1/2	20	1 A	20	1/2	12	12		0	KITCHEN LTS & PLUGS	R	18
19	L	LTS & PLUGS	0		12	12	1/2	20	1 B	20	1/2	12	12		0	KITCHEN LTS & PLUGS	R	20
21	L	LTS & PLUGS	0		12	12	1/2	20	1 A	20	1/2	12	12		0	DISPLAY CASE LTS	L	22
23									B									24

A — 1/10/2024 — FED FROM PANEL 'G'

ALL CIRCUIT CONDUCTORS SIZED FOR COPPER — North Bay Schedules

EXISTING GENERATOR PANEL — PANEL 'G' 7 — FAULT CURRENT = 8,272

200 AMP MAIN BREAKER — 120 / 240 VOLTS — 1-PHASE, 3-WIRE

FEEDER SIZE — ALUMINUM: 2" C, 3 #250 PH, #4 GRD — SURFACE MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	25%
CONNECTED VA	0	0	0	0	0	0	0	0	0	0	0	0 VA	
DIVERSITY FACTOR	125%	100%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	0	0	0	0	0	0	0	0	0	0	0	0 VA	

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR	PH	BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1		RANGE OR DRYER	0		12	10	1/2	50	2 A	2 125					0	PANEL A	P	2
3			0						B						0	PANEL A	P	4
5		UNSURE	0		12	8	1/2	70	2 A	1 20	1/2	12	12		0	RECEPTACLES	L	6
7			0						B	1 20	1/2	12	12		0	LTS EAST	L	8
9			0		12	12	1/2	20	1 A	1 20	1/2	12	12		0	LTS OUTSIDE	L	10
11		WATER HEATER	0		12	10	1/2	30	2 B	1 20	1/2	12	12		0	DOOR - SOUTH	L	12
13			0						A	1 20	1/2	12	12		0	DOOR - MID	L	14
15		LTS - CENTER BAY	0		12	12	1/2	20	1 B	1 20	1/2	12	12		0	DOOR - NORTH	L	16
17		LTS - NORTH BAY	0		12	12	1/2	20	1 A	2 30	1/2	10	12		0	AIR COMPRESSOR	R	18
19		LTS - 400W YARD	0		12	12	1/2	20	1 B						0			20
21		GEN BLOCK HEAT	0		12	12	1/2	20	1 A	1 20	1/2	12	12		0	LT - EMERGENCY	L	22
23		LTS - SOUTH BAY	0		12	12	1/2	20	1 B	1 20	1/2	12	12		0	LTS - AIR ROOM	L	24
25		HEATER - CENTRAL	0		12	10	1/2	60	2 A	1 20	1/2	12	12		0	LTS	L	26
27			0						B	1 20	1/2	12	12		0	RECEPTACLES	L	28
29		TWIN	0		12	12	1/2	20	1 A	1 20	1/2	12	12		0	GENERATOR PLUG		30

G — 1/10/2024 — FED FROM PANEL 'ATS'

ALL CIRCUIT CONDUCTORS SIZED FOR COPPER — North Bay Schedules

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REGISTERED PROFESSIONAL ENGINEER
65580
OREGON
JULY 13, 2004
GREGORY L. FRIDE
EXPIRES: 12-31-24

PROJECT NO.: 21.59

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

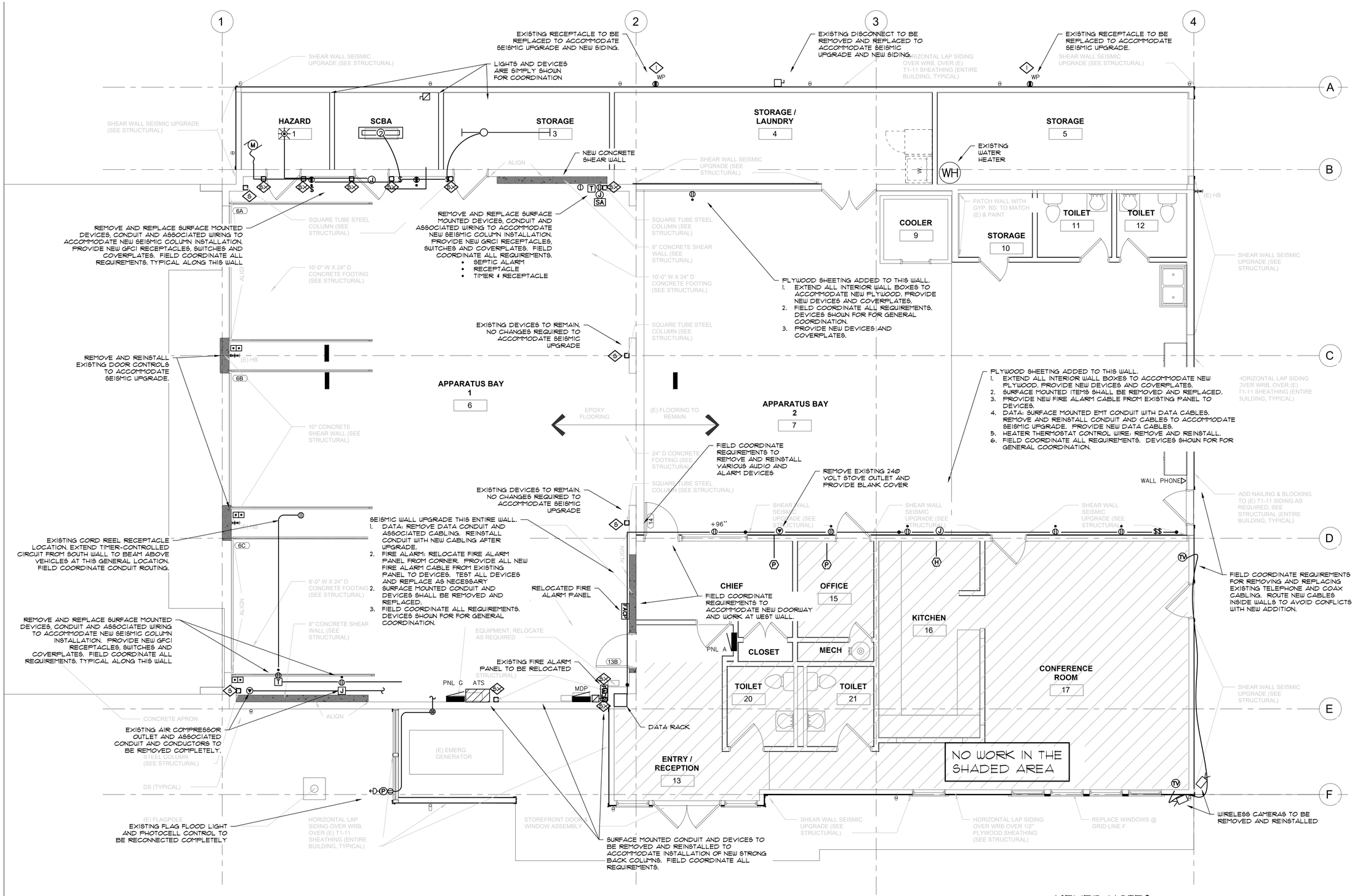
REVISIONS:

DATE	DESCRIPTION

DATE: JANUARY 2024

SHEET TITLE:
ALTERNATE BID
POWER & LIGHTING

E1.0



KEYED NOTES

1. REPLACE EXISTING OUTDOOR RECEPTACLE WITH NEW GFCI RECEPTACLE WITH WEATHERPROOF WHEN IN USE COVER. FIELD COORDINATE EXTENSION OF EXISTING BOX AND CONDUCTORS AS REQUIRED TO ACCOMMODATE SEISMIC UPGRADE.
2. NEW STRONG-BACK COLUMN – NO APPARENT DISRUPTION OF EXISTING ELECTRICAL. FIELD VERIFY ALL CONDITIONS.
3. NEW STRONG-BACK COLUMN – CONFLICTS WITH EXISTING SURFACE MOUNTED CONDUIT. REMOVE, REROUTE AND REINSTALL COMPLETELY CONDUIT AND ASSOCIATED CONDUCTORS. FIELD COORDINATE ALL REQUIREMENTS.

1 E2.1 BASE BID - POWER & LIGHTING

SCALE: 1/4" = 1'-0"

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REGISTERED PROFESSIONAL ENGINEER
65586
OREGON
JULY 13, 2004
GREGORY L. PRIDE
EXPIRES: 12-31-24

PROJECT NO.: 21.59

NORTH BAY FIRE SEISMIC GRANT UPGRADE & ADDITION

NORTH BAY FIRE DISTRICT
67577 EAST BAY RD
NORTH BEND, OR 97459

CONSTRUCTION

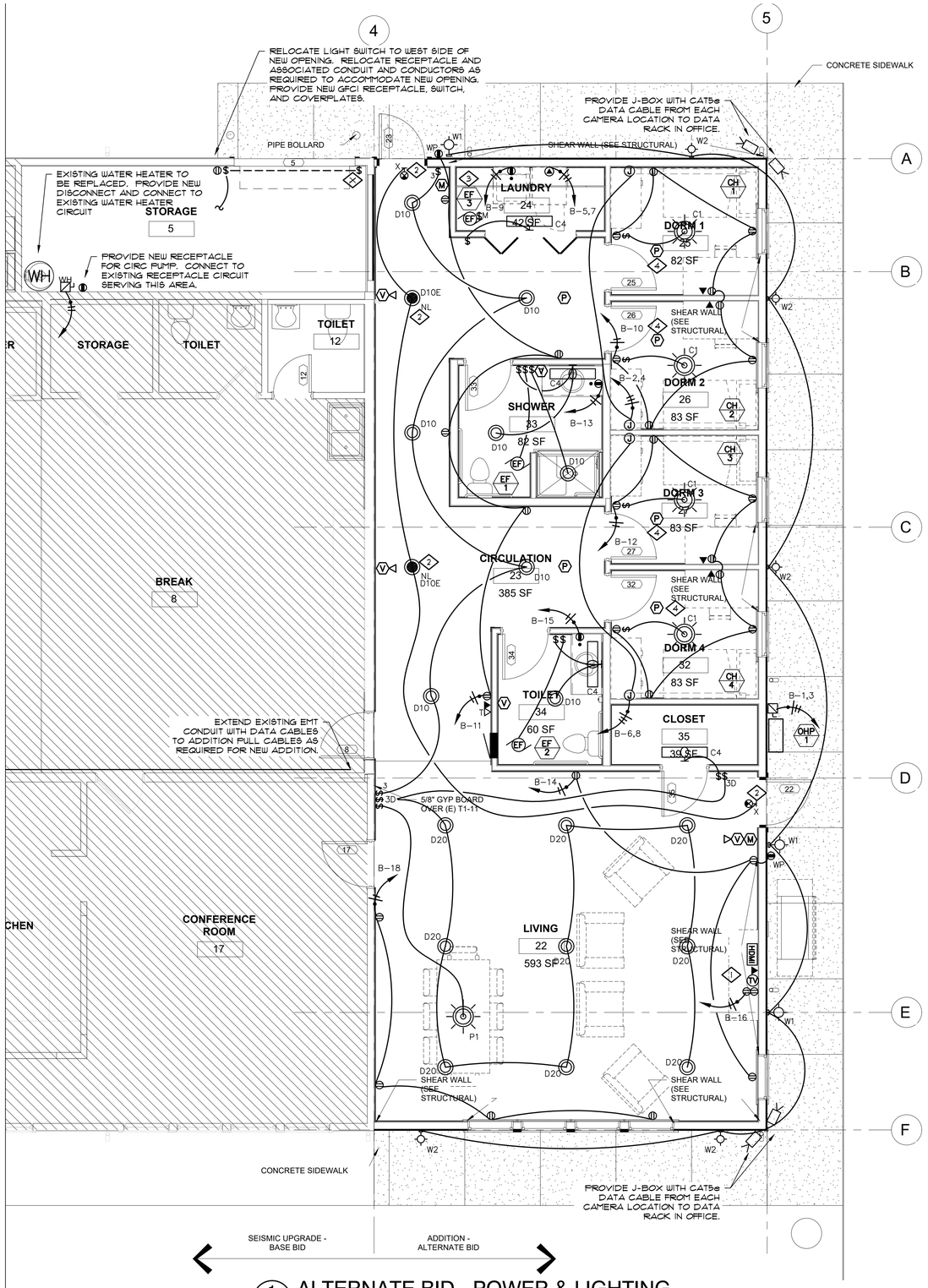
REVISIONS:

DATE	DESCRIPTION

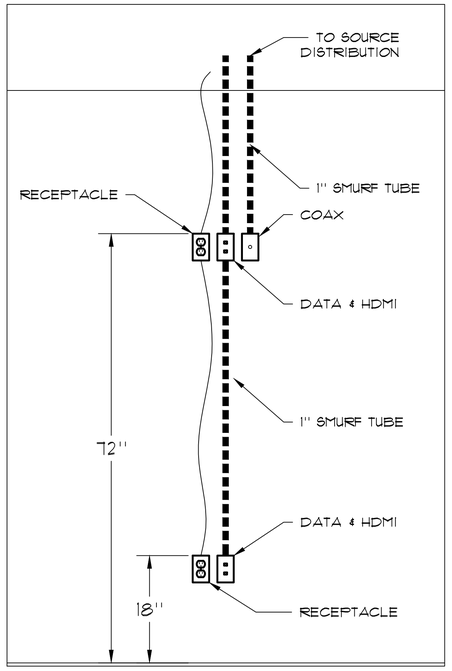
DATE: JANUARY 2024

SHEET TITLE: BASE BID POWER & LIGHTING

E2.1



1 E2.2 ALTERNATE BID - POWER & LIGHTING
SCALE: 1/4" = 1'-0"



3 E3.2 A/V DETAIL - WALL MOUNTED TV
DAYROOM & TRAINING ROOM

LIGHT FIXTURE SCHEDULE

NAME	MANUFACTURER	DESCRIPTION
C1	PROGRESS LIGHTING P350135-009-30	TYPE: 13" CEILING DRUM TRIM: WHITE SHADE - BRUSHED NICKEL MOUNTING: SURFACE - CEILING LAMPS: LED 3000K, 1400 LUMENS, 17 WATTS
C4	BARTCO - WEDGE BSS745-45-35-ND-L-SM-SN-AW	TYPE: CEILING / WALL CORNER LINEAR LED TRIM: WHITE - FROSTED LENS LENGTH: 45" NOMINAL MOUNTING: SURFACE - CEILING / WALL CORNER LAMPS: LED 3500K, 2400 LUMENS, 20 WATTS
D10 / D10E	LITHONIA LDN6 10LM 35K L06 AR LD MVOLT	TYPE: 6" LED RECESSED DOWNLIGHT TRIM: SELF FLANGED, CLEAR DIFFUSE MOUNTING: RECESSED - VERIFY CEILING TYPE D10E: ADD EMERGENCY BATTERY PACK LAMPS: LED 3500K, 1000 LUMENS, 12 WATTS
D20	LITHONIA LDN6 20LM 35K L06 AR LD MVOLT	TYPE: 6" LED RECESSED DOWNLIGHT TRIM: SELF FLANGED, CLEAR DIFFUSE MOUNTING: RECESSED - VERIFY CEILING TYPE D10E: ADD EMERGENCY BATTERY PACK LAMPS: LED 3500K, 2000 LUMENS, 25 WATTS
P1	DAINOLITE PHL-264P	TYPE: 30" DIAMETER PENDANT MOUNTING: CEILING PENDANT - SINGLE-STEM VERIFY WITH ARCHITECT FINISH: MATTE BLACK HARDWARE, WHITE DRUM SHADE LAMPS: (4) 15W LED A19 LAMP - WARM WHITE
W1	LITHONIA OLWP LED SWW2 120 PE DDB	TYPE: LED WALL PACK HOUSING: DIE-CAST ALUMINUM FINISH: DARK BRONZE MOUNTING: SURFACE - WALL LAMPS: LED, 3,000K, 1600 LUMEN, 18.5 WATTS
W2	LITHONIA - WEDGE WDGE1 LED P2 30K 80CRI VW MVOLT PE DDBXD	TYPE: LED WALL PACK HOUSING: DIE-CAST ALUMINUM FINISH: DARK BRONZE MOUNTING: SURFACE - WALL LAMPS: LED, 3,000K, 1900 LUMEN, 15 WATTS
X1	LITHONIA - ECBR LED M6	TYPE: LED EXIT SIGN WITH EMERGENCY LIGHT BAR HOUSING: THERMOPLASTIC OR POLYCARBONATE FINISH: WHITE HOUSING WITH RED LETTERS MOUNTING: FIELD VERIFY MOUNTING LAMPS: LED BATTERY: NI-CAD BATTERY NOTE: DOUBLE FACE AS NECESSARY

NORTH BAY FIRE DEPARTMENT — ALL FIXTURES ARE 120 VOLT UNLESS NOTED OTHERWISE

PANEL 'B' — FAULT CURRENT = 5Ø84

150 AMP — MAIN BREAKER — 120 / 240 VOLTS — 1-PHASE, 3-WIRE

FEEDER SIZE — ALUMINUM: 2" C, 3 #3/0 PH, #4 GRD — SURFACE MOUNTED

LOAD DISTRIBUTION	LTG	REC	MOTOR	DATA	EXTG	HEAT	MISC	PH-A	PH-B	TOTAL	AMPS	WITH SPARE	80%
CONNECTED VA	720	10460	5736	0	0	9800	0	14468	12248	26716	121	32059 VA	145
DIVERSITY FACTOR	125%	98%	100%	100%	65%	100%	100%						
DIVERSIFIED VA	900	10230	5736	0	0	9800	0	14517	12149	26666	121	31999 VA	145

PL	T	LOAD	VA	HP	PHW	GND	CON	BKR	PH		BKR	CON	GND	PHW	HP	VA	LOAD	T	PL
1	M	HEAT PUMP OHP-1	2868		10	10	1/2	50	2	A	2	20	1/2	12	12		1200	COVE HEATER 1 & 2	H	2
3	M		2868							B							1200		H	4
5	H	DRYER	2500		10	10	1/2	30	2	A	2	20	1/2	12	12		1200	COVE HEATER 3 & 4	H	6
7	H		2500							B							1200		H	8
9	R	WASH MACHINE	1500		12	12	1/2	20	1	A	1	20	1/2	12	12		1440	REC: DORM 1 & 2	R	10
11	R	REC: HALLWAY	1080		12	12	1/2	20	1	B	1	20	1/2	12	12		1440	REC: DORM 3 & 4	R	12
13	R	REC: TOILET / SHOWER	1600		12	12	1/2	20	1	A	1	20	1/2	12	12		720	REC: E. LIVING RM	R	14
15	R	REC: TOILET	1600		12	12	1/2	20	1	B	1	20	1/2	12	12		360	REC: TV WALL	R	16
17	L	LIGHTS	720		12	12	1/2	20	1	A	1	20	1/2	12	12		720	REC: SW LIVING RM	R	18
19										B										20
21										A										22
23										B										24
25										A										26
27										B										28
29										A										30
31										B										32

B — 1/10/2024 — FED FROM PANEL 'MDP' North Bay Schedules

ALL CIRCUIT CONDUCTORS SIZED FOR COPPER

MECHANICAL EQUIPMENT SCHEDULE

ID	DESCRIPTION	LOCATION	HP/KVA		VOLT	PH	DISCONNECT SWITCH SIZE	DISCONNECT SWITCH FUSED	NOTE
OHP - 1	HEAT PUMP	OUTSIDE	5.16	KW	240	1	30/2	X	NEMA 3R, MCA 23.9 / MOCP 25
CH - 1	COVE HEATER	DORM ROOM	1200	W	240	1			LOCKABLE BREAKER
CH - 2	COVE HEATER	DORM ROOM	1200	W	240	1			LOCKABLE BREAKER
CH - 3	COVE HEATER	DORM ROOM	1200	W	240	1			LOCKABLE BREAKER
CH - 4	COVE HEATER	DORM ROOM	1200	W	240	1			LOCKABLE BREAKER
EF-1	EXHAUST FAN	W. TOILET	12	W	120	1			WALL SWITCH
EF-2	EXHAUST FAN	W. TOILET	12	W	120	1			WALL SWITCH
EF-3	EXHAUST FAN	E. TOILET	12	W	120	1			1, HUMIDSTAT
WH-1	WATER HEATER	STORAGE	4.5	KW	240	1	30/2		REPLACE EXISTING
RCP-1	RECIRC. PUMP	STORAGE	90	W	120	1			CORD & PLUG

KEYED NOTES

1 MOTOR RATED SWITCH .

1/10/2024

KEYED NOTES

1. VIDEO WALL: SEE A/V DETAIL FOR WALL MOUNTED TV. PROVIDE RECEPTACLES, COAX TELEVISION, DATA AND HDMI AT VIDEO WALL. PROVIDE RECESSED BOX AT UPPER LOCATION TO ACCOMMODATE LOW-PROFILE WALL MOUNTED TELEVISION.
2. EXIT SIGNS AND BATTERY-PACK NIGHT LIGHTS: CONNECT TO UNSWITCHED LIGHTING CIRCUIT SERVING THIS AREA.
3. LAUNDRY FAN: CONNECT TO UNSWITCHED LIGHTING CIRCUIT SERVING THIS AREA.
4. PROVIDE CODE COMPLIANT SMOKE DETECTOR WITH NOTIFICATION FOR SLEEPING AREAS.
X. DEMO COMPLETELY

SHEET NOTES

1. POWER: 24-MONTH PACIFIC POWER STUDY REQUIRED TO VERIFY THAT EXISTING SERVICE HAS CAPACITY TO ADD PANEL 'B' LOADS.
2. FIRE ALARM: EXTEND EXISTING FIRE ALARM SYSTEM INTO ADDITION SPACE. PROVIDE CODE-COMPLIANT DETECTORS FOR SLEEPING AREAS IN ADDITION TO OTHER MINIMUM REQUIREMENTS SHOWN ON PLANS OR OTHERWISE REQUIRED BY CODE.
3. DATA: EXTEND DATA CABLING FROM DATA RACK IN OFFICE TO EACH LOCATION INDICATED BY DATA TRIANGLE.
4. TELEPHONE: EXTEND TELEPHONE TO NEW ADDITION
5. TELEVISION: EXTEND COAX CABLE TO NEW TELEVISION LOCATION IN ADDITION
6. LIGHTING: CONNECT LIGHT FIXTURES IN ADDITION TO CIRCUIT B-17. FIELD COORDINATE ALL CIRCUITING.

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